

1. DESIGN CRITERIA:

A. CODES AND STANDARDS

ALL DESIGN AND CONSTRUCTION WORK FOR THIS PROJECT SHALL CONFORM TO THE 2009 INTERNATIONAL BUILDING CODE.

B. LIVE LOAD DATA

CORRIDORS.....	100 PSF
MECHANICAL.....	150 PSF
ROOF.....	20 PSF
STAGE FLOOR.....	150 PSF
STAIRS.....	100 PSF
STORAGE.....	125 PSF
ALL LIVE LOADS NON-REDUCIBLE	

C. WIND LOAD DATA

BASIC WIND SPEED.....	90 MPH
IMPORTANCE FACTOR.....	1.15
EXPOSURE CLASSIFICATION.....	"C"
C AND C WIND DESIGN PRESSURE:	
OVERHANGS.....	32 PSF
WALLS.....	25 PSF

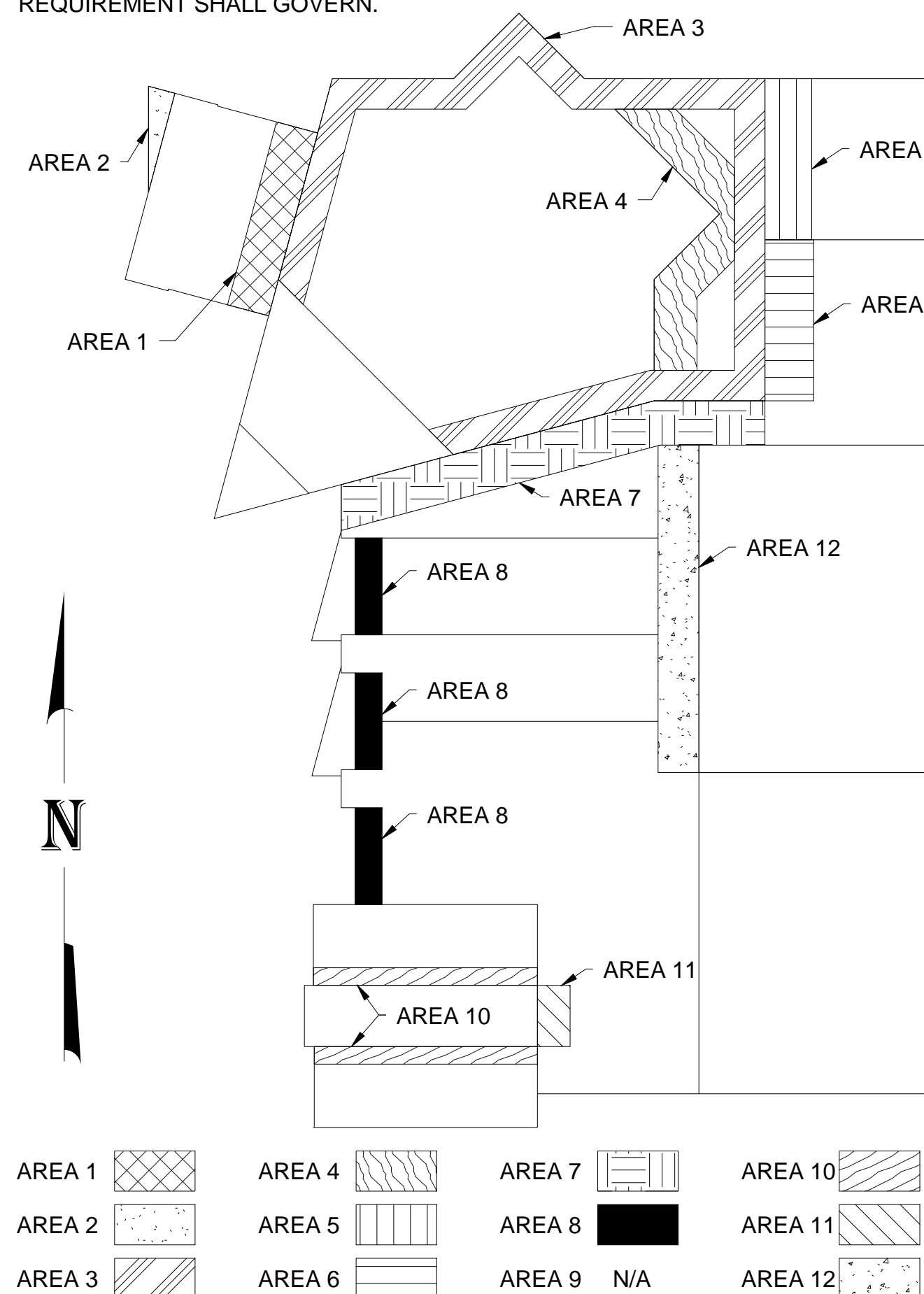
E. SEISMIC LOAD DATA

MAPPED SPECTRAL ACCELERATIONS	
0.2 SECOND PERIOD (S _s).....	1.12
1.0 SECOND PERIOD (S ₁).....	0.31
SPECTRAL RESPONSE COEFFICIENTS	
0.2 SECOND PERIOD (S _s).....	0.67
1.0 SECOND PERIOD (S ₁).....	0.58
OCCUPANCY CATEGORY.....	III
SEISMIC IMPORTANCE FACTOR.....	1.25
SITE CLASS.....	"E"
SEISMIC DESIGN CATEGORY.....	"D"
ANALYSIS PROCEDURE.....	MODAL RESPONSE SPECTRUM
SEISMIC FORCE-RESISTING SYSTEM	
N-S DIRECTION...A.1 SPECIAL REINFORCED CONCRETE SHEAR WALLS	
R = 5, Ω _o = 2 1/2, Cd = 5	
A.7 SPECIAL REINFORCED MASONRY SHEAR WALLS	
R = 5, Ω _o = 2 1/2, Cd = 3 1/2	
E-W DIRECTION...A.1 SPECIAL REINFORCED CONCRETE SHEAR WALLS	
R = 5, Ω _o = 2 1/2, Cd = 5	
A.7 SPECIAL REINFORCED MASONRY SHEAR WALLS	
R = 5, Ω _o = 2 1/2, Cd = 3 1/2	

F. SNOW LOAD DATA

GROUND SNOW LOAD.....	15 PSF
FLAT ROOF SNOW LOAD.....	17 PSF
SNOW EXPOSURE FACTOR.....	1.0
SNOW LOAD IMPORTANCE FACTOR.....	1.1
THERMAL EXPOSURE FACTOR.....	1.0
DESIGN SNOW LOAD.....	17 PSF
SNOW DRIFT.....	SEE SNOW DRIFT MAP ON THIS SHEET

G. IN CASE OF CONFLICT BY GOVERNING CODES, THE MOST STRINGENT REQUIREMENT SHALL GOVERN.



1 Snow Drift Loading
1" = 40'-0"

2. FOUNDATIONS:

A. GENERAL

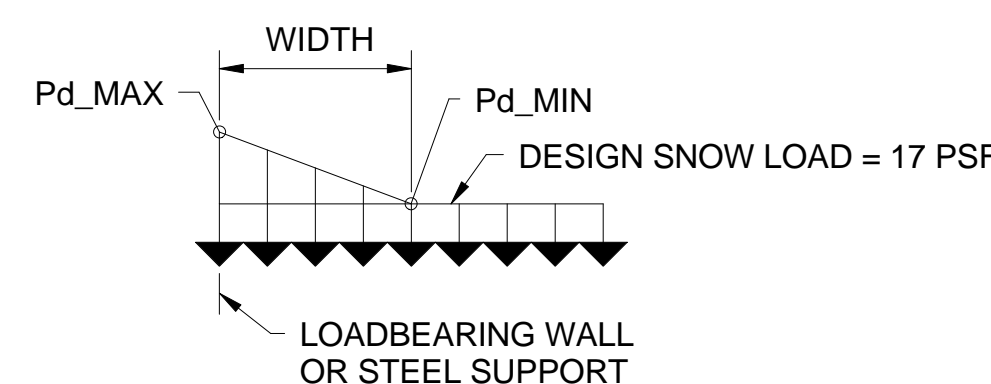
- THE FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE RECOMMENDATIONS MADE IN THE GEOTECHNICAL REPORT BY HURST-ROSCHÉ ENGINEERS, INC. DATED JANUARY 16, 2013. FOLLOW AND PROVIDE FOR ALL RECOMMENDATIONS OF THE GEOTECHNICAL REPORT.
- THE UPPER 6 INCHES OF ALL SLAB SUBGRADES, INCLUDING GRADE BEAMS, SHALL BE COMPACTED TO 95 PERCENT OR GREATER OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY (ASTM D698) AND WITHIN 3 PERCENT OF THE OPTIMUM MOISTURE CONTENT IMMEDIATELY PRIOR TO PLACEMENT. ALL BACKFILL AROUND AND ABOVE ALL FOUNDATION ELEMENTS, CAPS, AND WALLS SHALL BE COMPACTED TO 95 PERCENT OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY (ASTM D698) WITHIN 3 PERCENT OF THE OPTIMUM MOISTURE CONTENT. WELL-GRADED AGGREGATE LOCATED BENEATH FOOTINGS SHOULD BE COMPACTED TO 100 PERCENT OR GREATER OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY (ASTM D698). COHESIVE FILL MATERIAL BENEATH FOOTINGS SHOULD BE COMPACTED TO 95 PERCENT OR GREATER OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY (ASTM D698) AND WITH A MOISTURE CONTENT WITHIN 3 PERCENT OF THE OPTIMUM MOISTURE CONTENT.
- ALL ORGANIC AND/OR OTHER UNSUITABLE MATERIALS SHALL BE REMOVED FROM BELOW GRADE BEAMS, FOOTINGS, SLABS, AND BACKFILL AREAS, AND THEN BACKFILLED WITH ACCEPTABLE GRANULAR FILL COMPACTED TO 100 PERCENT OR GREATER OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY (ASTM D698) OR COHESIVE SOIL MATERIAL COMPACTED TO 95 PERCENT OR GREATER OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY (ASTM D698) AND WITH A MOISTURE CONTENT WITHIN 3 PERCENT OF THE OPTIMUM MOISTURE CONTENT.
- THE CONTRACTOR SHALL PROVIDE ALL NECESSARY MEASURES TO PREVENT ANY WATER, FROST, OR ICE FROM PENETRATING ANY FOOTING, GRADE BEAM, OR STRUCTURAL SLAB SUBGRADE BEFORE AND AFTER PLACING OF CONCRETE AND UNTIL SUCH SUBGRADES ARE FULLY PROTECTED BY THE PERMANENT BUILDING STRUCTURE.
- ALL SLABS-ON-GRADE SHALL BE PLACED OVER A CONTINUOUS VAPOR RETARDER OVER A MINIMUM OF 4 INCHES OF COMPACTED GRANULAR MATERIAL WHICH IS PLACED OVER A COMPACTED SOIL SUBGRADE.
- GRADE BEAMS AND WALLS THAT RETAIN EARTH ON BOTH SIDES SHALL BE BACKFILLED ON BOTH SIDES SIMULTANEOUSLY.

B. FOOTINGS

- SHALLOW FOOTINGS SHALL BEAR ON PREPARED SUBGRADE CAPABLE OF SUSTAINING A NET ALLOWABLE BEARING PRESSURE OF 2.9 KSF FOR CONTINUOUS FOOTING AND 3.45 KSF FOR INDIVIDUAL COLUMN FOOTINGS UNDER FULL SERVICE LIVE AND DEAD LOAD.
- FOOTINGS SHALL BE POURED INTO AN EARTH-FORMED TRENCH U.N.O.
- ALL BEARING MATERIAL SHALL BE INSPECTED BY THE GEOTECHNICAL ENGINEER OR AUTHORIZED REPRESENTATIVE PRIOR TO CONCRETE PLACEMENT. THE GEOTECHNICAL ENGINEER SHALL BE THE SOLE JUDGE AS TO THE SUITABILITY OF THE BEARING MATERIAL. FOOTING ELEVATIONS SHALL BE ADJUSTED AS REQUIRED.
- BOTTOM OF EXTERIOR FOOTINGS SHALL BEAR A MINIMUM OF 2'-0" BELOW FINAL EXTERIOR GRADE FOR FROST PROTECTION U.N.O.

Snow Drift Table			
Area	Pd_MAX	Pd_MIN	Width
1	50.7 PSF	0 PSF	12.7 FT
2	24.2 PSF	3.3 PSF	8.2 FT
3	36.2 PSF	0 PSF	9.1 FT
4	52.8 PSF	1.9 PSF	12.8 FT
5	55.6 PSF	0 PSF	14 FT
6	58 PSF	0 PSF	14.6 FT
7	52.7 PSF	0 PSF	13.2 FT
8	42.8 PSF	3.9 PSF	12 FT
10	20.9 PSF	0 PSF	5.3 FT
11	38.8 PSF	0 PSF	9.7 FT
12	48.7 PSF	0 PSF	12.2 FT

NOTE: AREA 9 INTENTIONALLY LEFT BLANK.



3. CONCRETE:

- NO CONCRETE SHALL BE POURED IN EXCAVATIONS CONTAINING WATER.
- ALL CONSTRUCTION JOINTS SHOWN ON DRAWINGS SHALL BE INCORPORATED INTO THE STRUCTURE, UNLESS THEIR ELIMINATION IS APPROVED BY THE ARCHITECT. ADDITIONAL CONSTRUCTION JOINTS, REQUIRED TO FACILITATE CONSTRUCTION, SHALL BE LOCATED AT POINTS OF MINIMUM SHEAR AND SHALL BE DETAILED ON SHOP DRAWINGS. REINFORCEMENT SHALL PASS CONTINUOUSLY THROUGH THE JOINT.
- UNLESS OTHERWISE SHOWN IN THE ARCHITECTURAL DRAWINGS, PROVIDE 3/4" CHAMFERS AT ALL EDGES THAT ARE EXPOSED TO VIEW IN THE FINISHED STRUCTURE.
- SEE ARCHITECTURAL DRAWINGS FOR DOOR AND WINDOW OPENINGS, SLAB DEPRESSIONS, AND FOR MISCELLANEOUS EMBEDDED PLATES, BOLTS, ANCHORS, ANGLES, ETC.
- REFER TO ARCHITECTURAL DRAWINGS FOR CONCRETE FINISHES. WHERE FINISH IS NOT SPECIFIED, CONFORM TO REQUIREMENTS OF ACI 301.
- MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS SHALL BE REFERRED TO FOR DRAINS, SLEEVES, OUTLET BOXES, CONDUIT, ANCHORS, ETC. THE VARIOUS TRADES ARE RESPONSIBLE FOR PLACING THEIR ITEMS.
- REFER TO MECHANICAL DRAWINGS FOR HOUSEKEEPING PADS AND INERTIA BASES AT MECHANICAL EQUIPMENT.
- REFER TO PLUMBING DRAWINGS FOR UNDERFLOOR AND PERIMETER FOUNDATION DRAINS.
- BASE PLATES, ANCHOR BOLTS, SUPPORT ANGLES, ETC., BELOW GRADE SHALL BE COATED WITH 1/8" OF MASTIC OR COVERED WITH A MINIMUM OF 3" CONCRETE.
- PROVIDE CONTINUOUS WATERSTOPS IN ALL CONSTRUCTION JOINTS IN BUILDING WALLS BELOW GRADE.
- ALL CONCRETE IS REINFORCED CONCRETE UNLESS SPECIFICALLY CALLED OUT AS "UNREINFORCED." REINFORCE ALL CONCRETE NOT OTHERWISE SHOWN WITH SAME REINFORCED STEEL AS IN SIMILAR SECTIONS OR AREAS.
- AT ALL OPENINGS IN CONCRETE WALLS, SLABS, AND REENTRANT CORNERS, ADD 2-#5 BARS (OPENING DIMENSION PLUS 60 BAR DIAMETERS LONG) AT EACH OF THE FOUR SIDES. ALSO ADD 2-#5 BARS 4'-0" LONG DIAGONAL AT EACH OF FOUR CORNERS. BAR LENGTHS SHALL EXTEND 2'-0" PAST OPENINGS EACH SIDE.
- BARS MARKED CONTINUOUS AND ALL VERTICAL STEEL SHALL BE LAPPED 70 BAR DIAMETERS (2'-0" MIN) AT SPLICES (UNLESS NOTED OTHERWISE ON DRAWINGS).
- NO ALUMINUM ITEMS SHALL BE EMBEDDED IN CONCRETE
- ALL REINFORCEMENT SPACING ARE SHOWN TO CENTERLINE OF BARS UNLESS NOTED OTHERWISE.
- PLACE REINFORCEMENTS AS SHOWN. REFER TO NOTES BELOW FOR REINFORCEMENT NOT SHOWN:
 - CENTER ALL VERTICAL REINFORCEMENT IN WALL PANELS, FOUNDATIONS, AND FROST WALLS.
 - CENTER ALL WWR SLAB REINFORCEMENT AND DOWELS.
- INTERIOR SLAB-ON-GRADE SHALL BE CONSTRUCTED AS A 4" NORMALWEIGHT CONCRETE SLAB WITH 6X6 - W2.9XW2.9 W.W.R.
- FIRST FLOOR SLAB SHALL BE CONSTRUCTED AS A 2 1/2" NORMALWEIGHT CONCRETE SLAB WITH 6X6 - W1.4XW1.4 W.W.R. OVER 1 1/2" COMPOSITE METAL DECK (TOTAL SLAB THICKNESS = 4").
- SECOND FLOOR SLAB SHALL BE CONSTRUCTED AS A 2 1/2" LIGHTWEIGHT CONCRETE SLAB WITH 6X6 - W1.4XW1.4 W.W.R. OVER 1 1/2" COMPOSITE METAL DECK (TOTAL SLAB THICKNESS = 4").
- INTERMEDIATE LANDING SHALL BE CONSTRUCTED AS A 2 1/2" LIGHTWEIGHT CONCRETE SLAB WITH 6X6 - W1.4XW1.4 W.W.R. OVER 1 1/2" COMPOSITE METAL DECK (TOTAL SLAB THICKNESS = 4").
- ROOF SLAB SHALL BE CONSTRUCTED AS A 2 1/2" LIGHTWEIGHT CONCRETE SLAB WITH 6X6 - W1.4XW1.4 W.W.R. OVER 1 1/2" COMPOSITE METAL DECK (TOTAL SLAB THICKNESS = 4").
- THE SURFACE OF FOUNDATIONS AND SLABS THAT RECEIVE MASONRY CONSTRUCTION SHALL BE SLIGHTLY ROUGHENED. THE ROUGH FINISH SHALL BE A MINIMUM OF 1/8" UPSET.

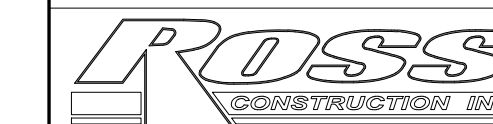
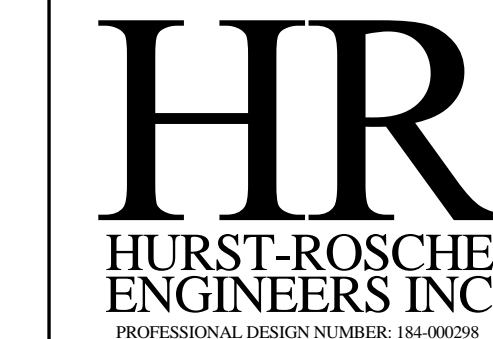
4. STEEL JOISTS:

- BRIDGING:
 - K SERIES JOISTS:** UNLESS NOTED OTHERWISE, BRIDGING SHALL BE DESIGNED & SPACED IN ACCORDANCE WITH S.J.I. SPECIFICATIONS.
 - LH SERIES JOISTS:** UNLESS NOTED OTHERWISE, BRIDGING SHALL BE DESIGNED & SPACED IN ACCORDANCE WITH S.J.I. SPECIFICATIONS.
- UNLESS NOTED OTHERWISE, JOISTS SHALL BE ATTACHED TO SUPPORTING STEEL WORK AS FOLLOWS:
 - K SERIES:** TWO 1/8" FILLET WELDS (ONE EACH SIDE) 1 1/2" LONG OR EQUIVALENT.
 - LH SERIES:** TWO 1/4" FILLET WELDS (ONE EACH SIDE) 2 1/2" LONG OR EQUIVALENT.
- BRIDGING THAT TERMINATES AT, OR IS INTERRUPTED BY, STRUCTURAL STEEL BEAMS, SHALL BE ATTACHED TO TOP FLANGE BY FIELD WELDING OR BOLTING.
- JOISTS SHALL BE STOCKPILED AT THE JOBSITE IN A VERTICAL POSITION, RESTING ON THEIR TOP OR BOTTOM CHORDS, AND SHALL BE ADEQUATELY SUPPORTED WITH WOOD BLOCKING. KEEP JOISTS FREE OF MUD AND DIRT.
- IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO SEE THAT JOISTS WHICH ARE DAMAGED, KINKED, BENT, OR WITH BROKEN WELDS, ARE NOT PLACED IN THE STRUCTURE.
- IF A CONCENTRATED LOAD OCCURS BETWEEN PANEL POINTS, THE CONTRACTOR SHALL PROVIDE AN ADDITIONAL WEB MEMBER FROM THE LOAD POINT TO THE NEAREST PANEL POINT ON THE OPPOSITE CHORD. SEE 9/S-308.
- SUBMIT SHOP DRAWINGS FOR JOISTS INDICATING SIZE, SPACING, AND BRIDGING AS REQUIRED BY THE SPECIFICATIONS.
- DESIGN OF ROOF JOISTS AND BRIDGING SHALL CONSIDER A NET UPLIFT FROM WIND OF 24 PSF (UNFACTORED).
- CONTRACTOR TO HAVE METAL SHIMS AVAILABLE FOR ANY NECESSARY JOIST BEARING ELEVATION ADJUSTMENT.

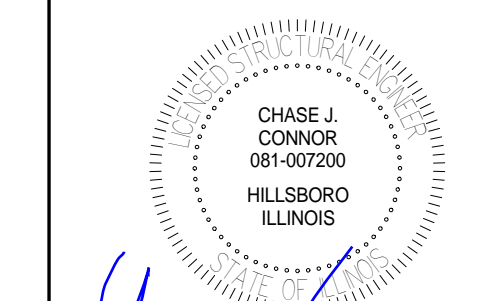


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SIGNATURE
Chase Connor

DATE 03/19/15

11-30-2016

LICENSE EXPIRES

PHASE 6 - FINE ARTS BUILDING ADDITION
 NEW MARION HIGH SCHOOL
 MARION C.U.S.D. #2
 MARION, WILLIAMSON COUNTY, ILLINOIS

Mark	Date	Description

DATE: 03/19/15

PROJECT NO: 360-2632

DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

GENERAL NOTES

S-001

5. STRUCTURAL STEEL:

- A. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED, AND ERECTED IN ACCORDANCE WITH THE AISC CODE OF STANDARD PRACTICE, EXCEPT AS MODIFIED IN THESE NOTES AND THE PROJECT SPECIFICATIONS.
- B. CONNECTIONS MAY BE BOLTED OR WELDED. THE FABRICATOR IS RESPONSIBLE FOR THE DESIGN OF CONNECTIONS NOT DESIGNED ON THE DRAWINGS. GENERALLY, CONNECTIONS SHOWN ON THE STRUCTURAL DRAWINGS ARE SCHEMATIC AND ARE ONLY INTENDED TO SHOW THE RELATIONSHIP OF MEMBERS CONNECTED. ANY CONNECTION THAT IS NOT SHOWN OR IS NOT COMPLETELY DETAILED ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED BY A STRUCTURAL ENGINEER, REGISTERED IN THE STATE OF ILLINOIS, RETAINED BY THE FABRICATOR. IT IS THE RESPONSIBILITY OF THE FABRICATOR TO PROVIDE ALL STIFFENER PLATES, ETC., THAT MAY BE REQUIRED IN ADDITION TO THOSE SHOWN ON THE STRUCTURAL SCHEMATIC DETAILS TO ENSURE THAT THE MEMBERS CONNECTED TOGETHER HAVE ADEQUATE STRENGTH AT THE CONNECTION. COMPLETELY DETAILED MEANS THE FOLLOWING INFORMATION IS SHOWN ON THE DETAIL.

- ALL PLATE DIMENSIONS.
- ALL WELD SIZES, LENGTHS, PITCHES, AND RETURNS.
- ALL HOLE SIZES AND SPACINGS.
- NUMBER AND TYPE OF BOLTS: WHERE BOLTS ARE SHOWN BUT NO NUMBER IS GIVEN, THE CONNECTION HAS NOT BEEN COMPLETELY DETAILED.
- WHERE PARTIAL INFORMATION IS GIVEN, IT SHALL BE THE MINIMUM REQUIREMENT FOR THE CONNECTION.

THE BEAM END (FACTORED) REACTIONS ARE NOTED BELOW U.N.O. IN PLANS:

W10: 20 KIPS	W16: 40 KIPS	W40: 170 KIPS
W12: 25 KIPS	W18: 50 KIPS	C: 15 KIPS
W14: 30 KIPS	W21: 55 KIPS	HSS: 15 KIPS

THE REACTIONS ABOVE ARE LRFD (FACTORED) REACTIONS. AISC "MANUAL OF STEEL CONSTRUCTION - 13TH EDITION" SHALL BE USED FOR DETAILING CONNECTIONS, UNLESS OTHER METHODS ARE APPROVED BY THE ENGINEER. REACTIONS IN EXCESS OF THE VALUES SHOWN WILL APPEAR ON THE PLAN AT THE END OF BEAMS ENCASED GRAPHICALLY IN A BOX.

DESIGN CALCULATIONS, SEALED BY FABRICATOR'S REGISTERED ENGINEER, FOR ALL BEAM AND GIRDER CALCULATIONS AND ALL PRIMARY BRACING AND HANGER CONNECTIONS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL PRIOR TO FABRICATION.

- C. MOMENT CONNECTIONS ARE SHOWN THUS ► ON THE DRAWINGS.

- D. U.N.O. THE MINIMUM PLATE THICKNESS SHALL BE 1/4", THE MINIMUM BOLT DIAMETER SHALL BE 5/8", THE MINIMUM WELD SHALL BE 3/16" AND THE MINIMUM DESIGN LOAD ON ANY CONNECTION SHALL BE 10 KIPS. BOLT HOLES SHALL BE STANDARD 1/16" DIAMETER LARGER THAN BOLT.

- E. PROVIDE WELDED STIFFENER PLATES ON BOTH SIDES OF THE WEB OF BEAMS AT POINT OF CONCENTRATED LOADS, INCLUDING BEAMS SUPPORTING COLUMNS OR RUNNING OVER THE TOPS OF COLUMNS, OR GIRDERS, AND AT THE LOCATION OF CHANGE OF SLOPE (KINKS) AT ANY MEMBER. MINIMUM STIFFENER PLATE THICKNESS SHALL BE 1/2" OR FLANGE THICKNESS OF THE COLUMN ABOVE OR BELOW, WHICHEVER IS GREATER.

- F. ALL STRUCTURAL STEEL EXPOSED TO VIEW ON INTERIOR OR EXTERIOR OF THE BUILDING SHALL CONFORM TO THE REQUIREMENTS OF ARCHITECTURALLY EXPOSED STEEL OF AISC CODE OF STANDARD PRACTICE SECTION 10.

- G. ALL EXTERIOR EXPOSED STEEL SHALL BE HOT-DIPPED GALVANIZED PER ASTM A123, U.N.O.

- H. BOLTED CONNECTIONS:

- ALL EXTERIOR EXPOSED BOLTS, NUTS, AND WASHERS SHALL BE TYPE 3 U.N.O.
- SHOP DRAWINGS SHALL INDICATE THE TYPE OF BOLT USED IN EACH CONNECTION AND THE DESIGN VALUES USED FOR THE VARIOUS BOLT TYPES.

- I. ALL WELDS EXPOSED TO VIEW SHALL BE GROUND SMOOTH.

- J. SPLICING OF STEEL MEMBERS, UNLESS SHOWN ON THE DRAWINGS, IS PROHIBITED WITHOUT WRITTEN APPROVAL OF THE ARCHITECT/ENGINEER.

- K. NO CHANGE IN SIZE OR POSITION OF STRUCTURAL ELEMENTS SHALL BE MADE AND HOLES, SLOTS, CUTS, ETC. ARE NOT PERMITTED THROUGH ANY MEMBER UNLESS THEY ARE APPROVED BY ARCHITECT/ENGINEER.

- L. NO FINAL BOLTING OR WELDING SHALL BE MADE UNTIL AS MUCH OF THE STRUCTURE AS WILL BE STIFFENED THEREBY HAS BEEN PROPERLY ALIGNED.

- M. FABRICATE ALL BEAMS WITH THE MILL CAMBER UP.

- N. ALL CONNECTIONS TO BE SHOP WELDED AND FIELD BOLTED WHERE PRACTICALLY POSSIBLE EXCEPT AS INDICATED ON DRAWINGS.

- O. ALL STRUCTURAL STEEL SHALL HAVE ONE SHOP COAT OF RUST INHIBITING PRIMER PAINT CONFORMING TO PROJECT SPECIFICATIONS. FIELD TOUCH UP ALL UNPAINTED AREAS.

- P. ALL DETAILS, SECTIONS, AND NOTES SHOWN ON DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR SITUATIONS ELSEWHERE.

- Q. ALL ADDITIONAL STEEL OR OTHER MATERIALS REQUIRED BY THE CONTRACTOR FOR ERECTION PURPOSES AND SITE ACCESS OF STOCKPILED MATERIALS SHALL BE PROVIDED AT NO COST TO THE OWNER. ALL SUCH ADDITIONAL MATERIALS SHALL BE REMOVED BY THE CONTRACTOR UNLESS APPROVED BY THE OWNER IN WRITING.

6. ANCHORS:

- A. FASTNERS SHOWN IN THE TABLE BELOW ARE MANUFACTURED BY HILTI. CONTRACTOR MAY SUBSTITUTE EQUIVALENT ALTERNATES WITH WRITTEN APPROVAL FROM A/E.

Fastner Schedule		
TYPE	SPECIFICATION	MINIMUM EMBEDMENT
1/2" Adhesive	HAS-E THREADED ROD W/ HILTI HY 70 ADHESIVE	4 1/2"
1/2" Dia. Mechanical	KWIK BOLT 3 EXPANSION ANCHOR	3 1/2"
1/4" Screw Anchor	KWIK CON II+	1 3/4"
Drilled Rebar	#6 HOOKED GALVANIZED STEEL REBAR WITH HILTI HY 200 SAFE SET SYSTEM SPACED @ 2'-0" O.C.	1' - 0"

- B. IF ANCHORED IN MASONRY, CELLS SHALL BE GROUTED FULL U.N.O.

- C. INSTALL PER MANUFACTURE'S RECOMMENDATIONS.

- D. USE STAINLESS STEEL ANCHORS AND HARDWARE WHEN IN CONTACT WITH STAINLESS STEEL FRAMING.

7. REINFORCED MASONRY:

- A. SEE SHEET S-401 AND S-402.

8. METAL ROOF DECK:

- A. METAL ROOF DECK SHALL COMPLY WITH THE REQUIREMENTS OF THE STEEL DECK INSTITUTE.

- B. METAL ROOF DECK 31 SHALL BE S.D.I. 18 GAUGE STEEL ROOF DECKING 1.5" DEEP AND ROLLED OF STEEL SHEETS CONFORMING TO ASTM A653 GRADE 50 AND SHALL BE GALVANIZED TO G90 REQUIREMENTS. NO METAL ROOF DECK SHALL HAVE SECTION PROPERTIES PER FOOT OF WIDTH LESS THAN THE FOLLOWING:

I (+)= 0.373 INCH(4)
I (-)= 0.373 INCH(4)
S (+)= 0.408 INCH(3)
S (-)= 0.411 INCH(3)

- C. METAL ROOF DECK 31 HAS BEEN DESIGNED TO FUNCTION AS A DIAPHRAGM FOR THE TRANSMISSION OF LATERAL LOADS. CONNECT DECK UNITS TO EACH OTHER WITH S-SLC 01 M HWH SIDELAP CONNECTOR OR EQUIVALENT @ 2'-6" SPACING FOR THE 1.5" DEEP METAL ROOF DECK. CONNECT DECK UNITS TO EXTERIOR SUPPORTS AND ALL OTHER DECK BOUNDARIES WITH HILTI X-ENP - 19-L15 OR EQUIVALENT IN A 36/3 PATTERN.

- D. DO NOT SUSPEND PIPES OR DUCTS FROM ROOF DECK.

- E. FABRICATE DECK UNITS IN LENGTHS TO SPAN THREE OR MORE SUPPORT SPACINGS. SUPPORTS SPACED CLOSER THAN 1' SHALL BE CONSIDERED AS ONE SUPPORT.

- F. DECKING MANUFACTURER SHALL COORDINATE SIZE AND LOCATION OF ROOF OPENINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.

9. COMPOSITE METAL DECK:

- A. COMPOSITE METAL DECK SHALL COMPLY WITH THE REQUIREMENTS OF THE STEEL DECK INSTITUTE.

- B. COMPOSITE METAL DECK 30 SHALL BE S.D.I. 20 GAUGE STEEL DECKING 1 1/2" DEEP AND ROLLED STEEL SHEETS CONFORMING TO ASTM A653 GRADE 40 AND SHALL BE GALVANIZED TO G30 REQUIREMENTS. NO METAL DECKING SHALL HAVE SECTION PROPERTIES PER FOOT OF WIDTH LESS THAN THE FOLLOWING:

I (+)= 0.222 INCH(4)
I (-)= 0.186 INCH(4)
S (+)= 0.231 INCH(3)
S (-)= 0.224 INCH(3)

- C. COMPOSITE METAL DECK 30 HAS BEEN DESIGNED TO FUNCTION AS A DIAPHRAGM FOR THE TRANSMISSION OF LATERAL LOADS. CONNECT DECK UNITS TO EACH OTHER WITH S-SLC 01 M HWH SIDELAP CONNECTOR OR EQUIVALENT @ 9" SPACING FOR COMPOSITE METAL DECK. CONNECT DECK UNITS TO EXTERIOR SUPPORTS AND ALL OTHER DECK BOUNDARIES WITH HILTI X-HSN 24 OR EQUIVALENT IN A 36/7 PATTERN.

- D. COMPOSITE METAL DECK 32 SHALL BE S.D.I. 18 GAUGE STEEL DECKING 1 1/2" DEEP AND ROLLED STEEL SHEETS CONFORMING TO ASTM A653 GRADE 40 AND SHALL BE GALVANIZED TO G30 REQUIREMENTS. NO METAL DECKING SHALL HAVE SECTION PROPERTIES PER FOOT OF WIDTH LESS THAN THE FOLLOWING:

I (+)= 0.295 INCH(4)
I (-)= 0.272 INCH(4)
S (+)= 0.324 INCH(3)
S (-)= 0.311 INCH(3)

- E. COMPOSITE METAL DECK 32 HAS BEEN DESIGNED TO FUNCTION AS A DIAPHRAGM FOR THE TRANSMISSION OF LATERAL LOADS. CONNECT DECK UNITS TO EACH OTHER WITH S-SLC 01 M HWH SIDELAP CONNECTOR OR EQUIVALENT @ 3'-0" SPACING FOR COMPOSITE METAL DECK. CONNECT DECK UNITS TO EXTERIOR SUPPORTS AND ALL OTHER DECK BOUNDARIES WITH HILTI X-HSN 24 OR EQUIVALENT IN A 36/4 PATTERN.

- F. FABRICATE DECK UNITS IN LENGTHS TO SPAN THREE OR MORE SUPPORT SPACINGS. SUPPORTS SPACED CLOSER THAN 1' SHALL BE CONSIDERED AS ONE SUPPORT.

- G. CONTRACTOR SHALL FURNISH ADDITIONAL CONCRETE DUE TO WET CONCRETE DEFLECTION OF THE COMPOSITE DECK.

- H. DECKING MANUFACTURER SHALL COORDINATE SIZE AND LOCATION OF OPENINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.

10. MISCELLANEOUS:

- A. STRUCTURAL DRAWINGS ARE INTENDED TO BE USED WITH ARCHITECTURAL AND MECHANICAL DRAWINGS. CONTRACTOR IS RESPONSIBLE FOR COORDINATING SUCH REQUIREMENTS, SHOP DRAWINGS AND WORK.

- B. NO OPENING SHALL BE MADE IN ANY STRUCTURAL MEMBER WITHOUT THE WRITTEN APPROVAL OF THE ARCHITECT/ENGINEER.

- C. NO CHANGE IN SIZE OR DIMENSION OF STRUCTURAL MEMBERS SHALL BE MADE WITHOUT THE WRITTEN APPROVAL OF THE ARCHITECT/ENGINEER.

- D. THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED UPON STRUCTURAL FRAMING. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN CAPACITY OF THE FRAMING AT THE TIME LOADS ARE IMPOSED.

- E. THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES.

- F. UNLESS OTHERWISE NOTED, FIREPROOFING METHODS AND MATERIALS FOR STRUCTURAL MEMBERS ARE NOT SHOWN ON STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR FIRE RATING REQUIREMENTS, FIRE PROOFING METHODS AND MATERIALS.

- G. DO NOT SCALE THESE DRAWINGS, USE DIMENSIONS.

- H. CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD. EXPANSION JOINTS SHOWN ON THE DRAWINGS HAVE BEEN DESIGNED TO ACCOMMODATE ANTICIPATED THERMAL MOVEMENT AFTER THE BUILDING IS COMPLETE.

- I. THE CONTRACTOR SHALL INFORM THE ARCHITECT/ENGINEER IN WRITING OF ANY DEVIATION FROM THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL NOT BE RELIEVED OF THE RESPONSIBILITY FOR SUCH DEVIATION BY THE ARCHITECT'S/ENGINEER'S APPROVAL OF SHOP DRAWINGS, PRODUCT DATA, ETC., UNLESS HE/SHE HAS SPECIFICALLY INFORMED THE ARCHITECT/ENGINEER OF SUCH DEVIATION AT THE TIME OF SUBMISSION, AND THE ARCHITECT/ENGINEER HAS GIVEN WRITTEN APPROVAL TO THE SPECIFIC DEVIATION.

- J. ALL THINGS WHICH, IN THE OPINION OF THE CONTRACTOR, APPEAR TO BE DEFICIENCIES, OMISSIONS, CONTRADICTIONS, OR AMBIGUITIES, IN THE PLANS AND SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER. PLANS AND/OR SPECIFICATIONS WILL BE CORRECTED, OR WRITTEN INTERPRETATION OF THE ALLEGED DEFICIENCY, OMISSION, CONTRADICTION OR AMBIGUITY WILL BE MADE BY THE A/E BEFORE THE AFFECTED WORK PROCEEDS.

- K. FURNISH ALL LABOR, MATERIALS, AND EQUIPMENT NECESSARY TO COMPLETE THE WORK SHOWN OR INFERRED BY THESE DRAWINGS.

- L. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN.

- M. REFERENCE THE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

- N. DETAILS LABELED "TYPICAL DETAILS" ON THE DRAWINGS SHALL APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED. THE APPLICABILITY OF THE DETAIL TO ITS LOCATION ON THE PLANS CAN BE DETERMINED BY THE TITLE OF THE DETAIL. SUCH DETAILS SHALL APPLY WHETHER OR NOT THEY ARE KEYED IN AT EACH LOCATION. DECISIONS REGARDING APPLICABILITY OF TYPICAL DETAILS SHALL BE DETERMINED BY THE A/E.

11. EXISTING CONDITIONS:

- A. BECOME FAMILIAR WITH EXISTING CONDITIONS PRIOR TO BIDDING. PROVIDE NECESSARY MEANS AND METHODS TO ACCOMPLISH THE SPECIFIED WORK.

- B. FIELD VERIFY ALL CONDITIONS, SIZES, SPACINGS, AND DIMENSIONS OF EXISTING STRUCTURE PRIOR TO FABRICATION AND NEW CONSTRUCTION. CONTACT THE STRUCTURAL ENGINEER IF EXISTING CONDITIONS, ETC. VARY FROM THOSE SHOWN ON THESE DRAWINGS.

- C. CONTRACTOR IS RESPONSIBLE FOR TEMPORARY REMOVAL AND REPLACEMENT/RELOCATION OF ANY NON-STRUCTURAL ELEMENTS NECESSARY TO COMPLETE THE STRUCTURAL WORK. FOLLOW ALL APPLICABLE CODES AND REQUIREMENTS OF AFFECTED TRADES. PROVIDE FOR THIS REQUIREMENT IN THE BID.

12. TILT-UP CONCRETE SHEAR WALLS:

- A. SEE SHEET S-701 FOR LATERAL LOADING RESISTED BY SHEAR WALLS.

- B. TILT-UP CONCRETE WORK SHALL COMFORM TO THE LATEST "AMERICAN CONCRETE INSTITUTE" STANDARDS IN ADDITION TO THE OTHER REQUIREMENTS OF THE PROJECT.

- C. SUBMIT ERECTION DRAWINGS/PRODUCT DRAWINGS SIGNED AND SEALED BY A STRUCTURAL ENGINEER LICENSED IN THE STATE OF ILLINOIS SHOWING:

- PLANS LOCATING AND DEFINING ALL TILT-UP WALL PANELS FURNISHED BY THE MANUFACTURER.
- SECTIONS AND DETAILS SHOWING CONNECTIONS, EDGE CONDITIONS, AND SUPPORT CONDITIONS.
- ALL DEAD, LIVE, SNOW, WIND, AND SEMISC LOADS USED IN THE DESIGN.
- LOCATION AND TYPE OF REINFORCING STEEL.
- CONCRETE STRENGTH

- D. DESIGN CRITERIA:
- DESIGN WALL PANELS TO WITHSTAND THE FOLLOWING LOADINGS: INITIAL HANDLING AND ERECTION STRESSES; ALL DEAD AND LIVE LOADS; SPECIFIED WIND AND SEISMIC LOADS; OTHER APPLICABLE LOADS.
 - DESIGN CALCULATIONS SHALL BE PERFORMED BY A STATE OF ILLINOIS LICENSED STRUCTURAL ENGINEER AND SUBMITTED FOR RECORD.
 - DESIGN SHALL CONFORM TO ACI 318-08 AND/OR APPLICABLE CODES.

- E. CONNECTIONS AND CONNECTION DETAILS TO BE DESIGNED BY THE TILT-UP MANUFACTURER.

- F. ERECTION BRACING AND SUPPORT SHALL BE PROVIDED AS REQUIRED TO GIVE ADEQUATE SUPPORT TO STRUCTURE UNTIL FINAL CONNECTIONS HAVE BEEN MADE.

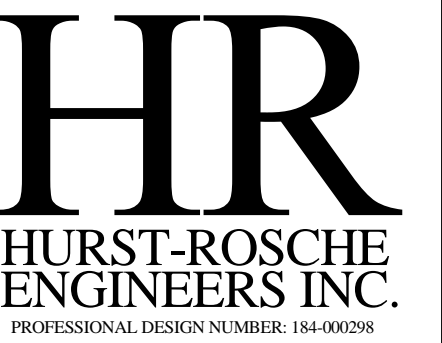
- G. FURNISH AND INSTALL INSERTS, ANCHORS, REGLETS, WELD PLATES, ETC. REQUIRED TO BE CAST INTO UNITS AND SHOW SAME ON SHOP DRAWINGS.

- H. REVIEW DRAWINGS AND SPECIFICATIONS FOR FURTHER REQUIREMENTS.



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504 W. Jackson Marion, IL 62959
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Chase Connor
SIGNATURE

03/19/15

DATE 11-30-2016

LICENSE EXPIRES

PHASE 6 - FINE ARTS BUILDING ADDITION
NEW MARION HIGH SCHOOL
MARION C.U.S.D. #2
MARION, WILLIAMSON COUNTY, ILLINOIS

Mark	Date	Description

DATE: 03/19/15

PROJECT NO: 360-2632

DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

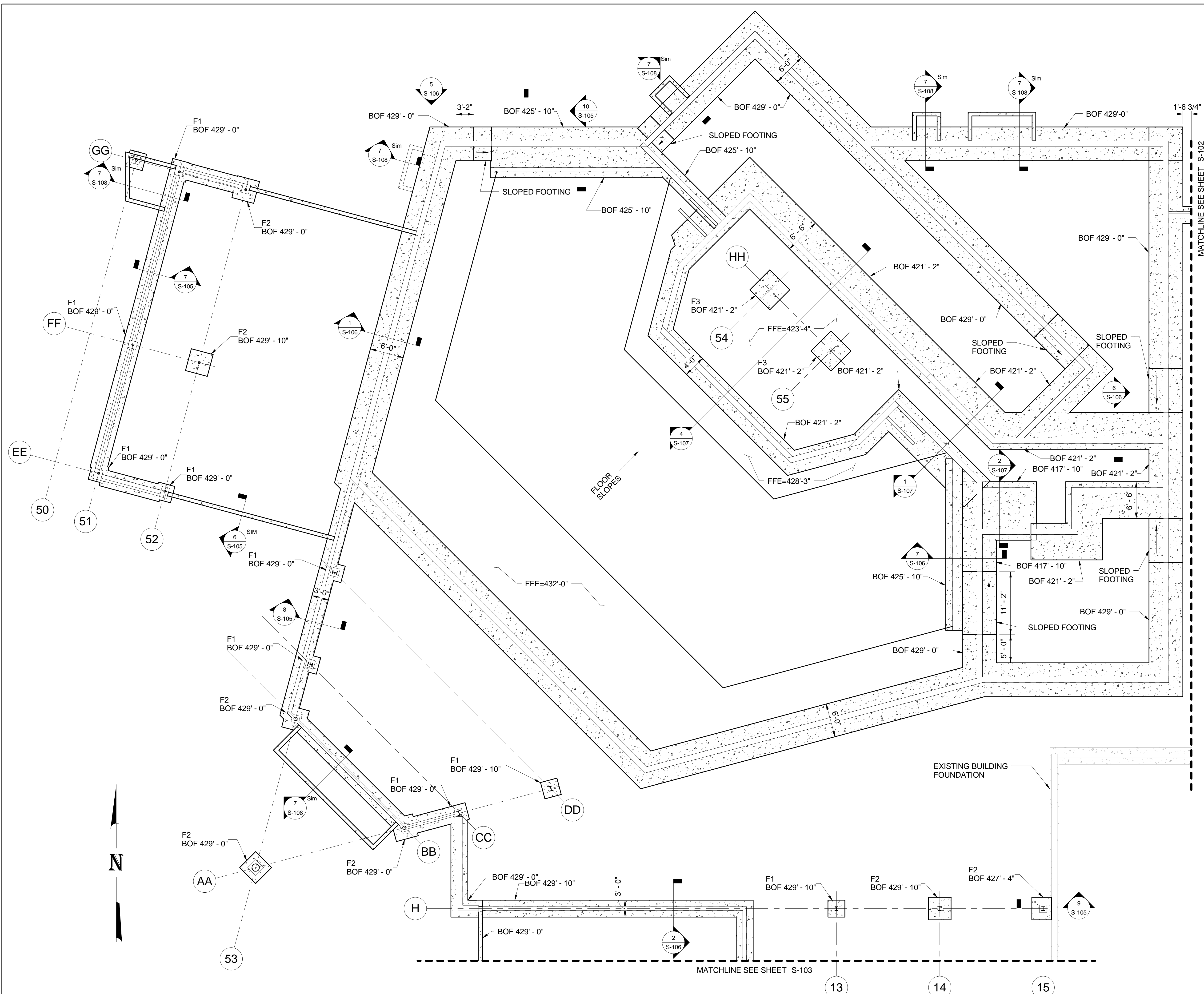
GENERAL NOTES

- F. ERECTION BRACING AND SUPPORT SHALL BE PROVIDED AS REQUIRED TO GIVE ADEQUATE SUPPORT TO STRUCTURE UNTIL FINAL CONNECTIONS HAVE BEEN MADE.

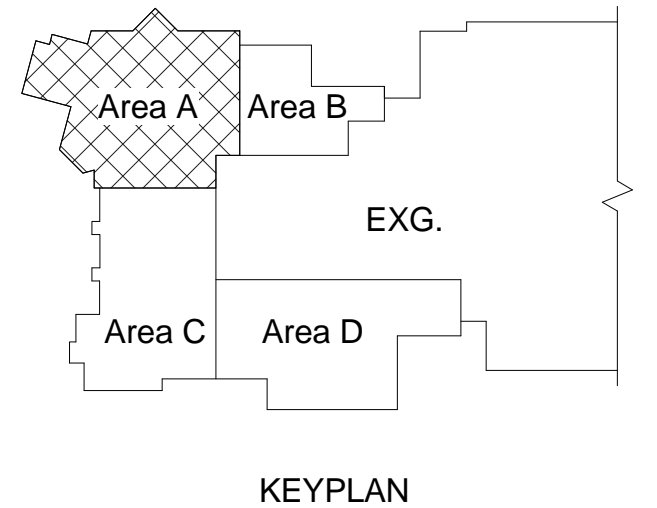
- G. FURNISH AND INSTALL INSERTS, ANCHORS, REGLETS, WELD PLATES, ETC. REQUIRED TO BE CAST INTO UNITS AND SHOW SAME ON SHOP DRAWINGS.

- H. REVIEW DRAWINGS AND SPECIFICATIONS FOR FURTHER REQUIREMENTS.

S-002



- GENERAL NOTES:**
- ALL INTERIOR NON-LOAD BEARING CMU WALLS SHALL HAVE A THICKENED SLAB AS SHOWN ON DETAIL 3/S-108 U.N.O. THICKENED SLAB NOT SHOWN ON SHEET FOR CLARITY.
 - SEE DETAIL 1/S-108 FOR STAIR FOUNDATION.
 - FINISH FLOOR SLAB ELEVATION (F.F.E.) = 432'-0" U.N.O.
 - TOP OF FOUNDATION WALL ELEVATION (TFW) = 432'-0" U.N.O.
 - BRICK LEDGE ELEVATION (BLE) ON EXTERIOR FOUNDATION WALL SHALL BE 431'-4" U.N.O.
 - REFER TO SHEET S-105 FOR STRUCTURAL FOUNDATION SCHEDULE.
 - EXISTING FOUNDATIONS TO BE FIELD VERIFIED. BOF ELEVATION OF PROPOSED FOOTINGS ATTACHED TO EXISTING FOOTINGS ARE APPROXIMATE ONLY AND SHALL BE ADJUSTED UPON FIELD VERIFICATION.
 - REFER TO SHEETS S-502 FOR COMPLETE COLUMN GRID LOCATIONS AND DIMENSIONS.
 - SEE ARCH SHEETS FOR EXTENTS AND LOCATION OF THE INTERIOR SLAB REPLACEMENT.



DAI
 DESIGN ARCHITECTS, INC.
 PROFESSIONAL DESIGN NUMBER: 184-00049

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 EAST ST. LOUIS, IL
 SPRINGFIELD, IL
 ARNOLD, MO
 NEOSHO, MO

HR
 HURST-ROSCHKE
 ENGINEERS INC.
 PROFESSIONAL DESIGN NUMBER: 184-00028

ROSS
 CONSTRUCTION INC.

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 p. 618.993.5904
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CHASE J
 CONNOR
 081-007200
 HILLSBORO
 ILLINOIS

Chase Connor
 SIGNATURE
 DATE 03/19/15
 11-30-2016
 LICENSE EXPIRES

PHASE 6 - FINE ARTS BUILDING ADDITION
 NEW MARION HIGH SCHOOL
 MARION C.U.S.D. #2
 MARION, WILLIAMSON COUNTY, ILLINOIS

Mark	Date	Description

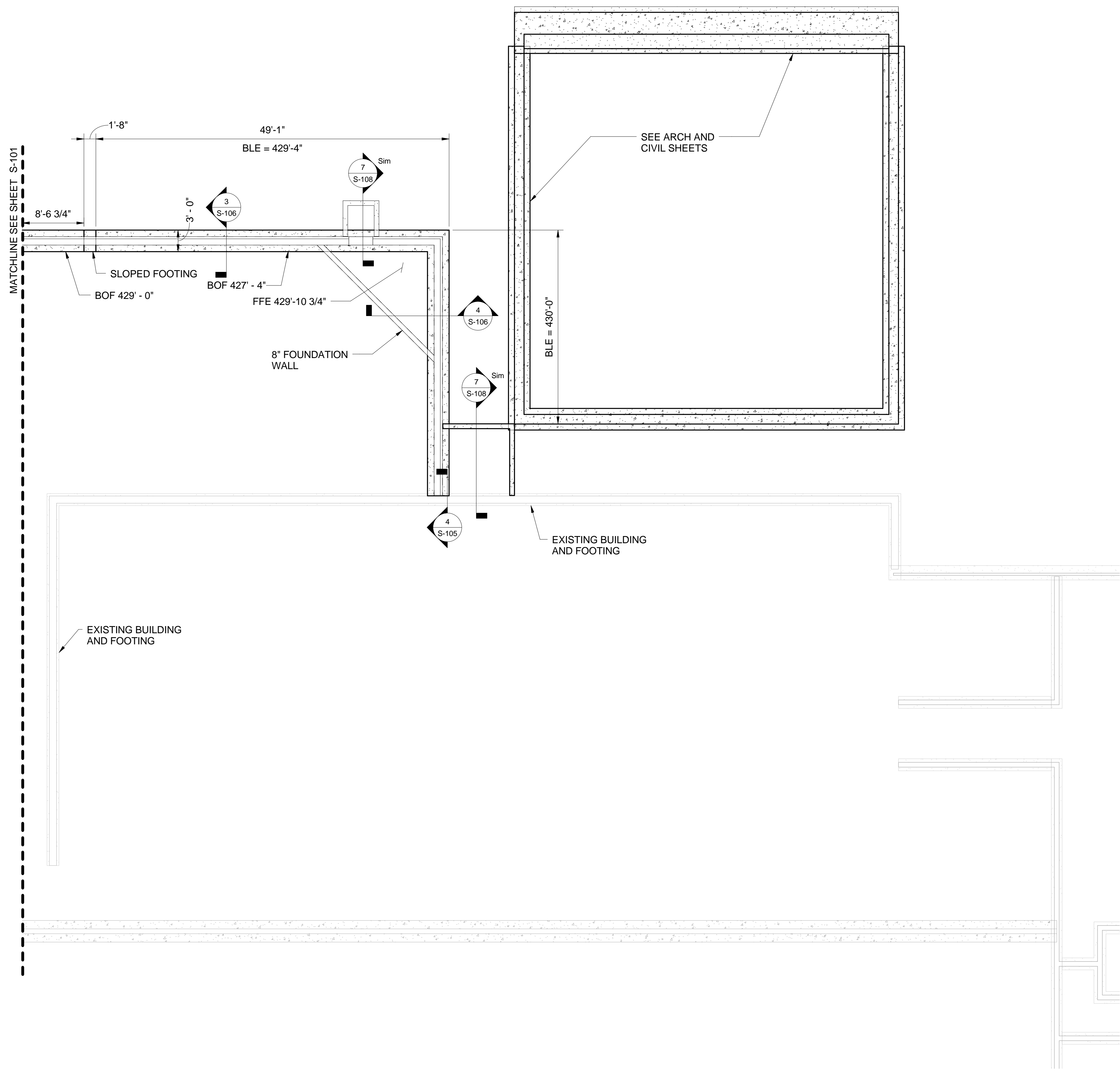
DATE: 03/19/15
 PROJECT NO: 360-2632
 DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

AREA 'A' -
 FOUNDATION PLAN

S-101

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1 Foundation Plan - Area A
 1/8" = 1'-0"



1 Foundation Plan - Area B
1/8" = 1'-0"

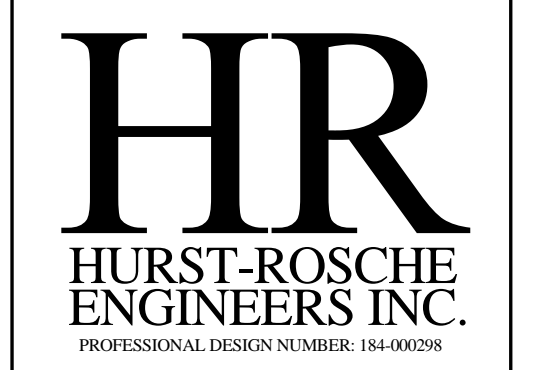
GENERAL NOTES:

1. ALL INTERIOR NON-LOAD BEARING CMU WALLS SHALL HAVE A THICKENED SLAB AS SHOWN ON DETAIL 3/S-108 U.N.O. THICKENED SLAB NOT SHOWN ON SHEET FOR CLARITY.
2. SEE DETAIL 1/S-108 FOR STAIR FOUNDATION.
3. FINISH FLOOR SLAB ELEVATION (F.F.E.) = 432'-0" U.N.O.
4. TOP OF FOUNDATION WALL ELEVATION (TFW) = 432'-0" U.N.O.
5. BRICK LEDGE ELEVATION (BLE) ON EXTERIOR FOUNDATION WALL SHALL BE 431'-4" U.N.O.
6. REFER TO SHEET S-105 FOR STRUCTURAL FOUNDATION SCHEDULE.
7. EXISTING FOUNDATIONS TO BE FIELD VERIFIED. BOF ELEVATION OF PROPOSED FOOTINGS ATTACHED TO EXISTING FOOTINGS ARE APPROXIMATE ONLY AND SHALL BE ADJUSTED UPON FIELD VERIFICATION.
8. REFER TO SHEETS S-502 FOR COMPLETE COLUMN GRID LOCATIONS AND DIMENSIONS.

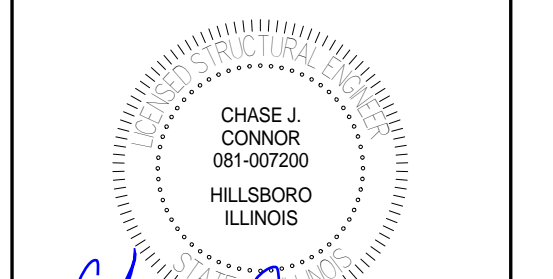


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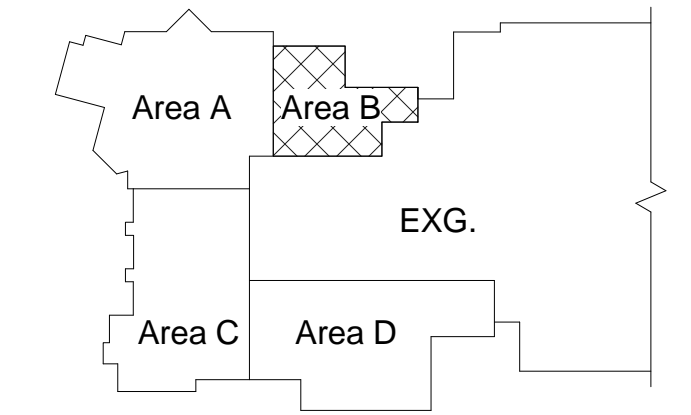


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Chase Connor
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PHASE 6 - FINE ARTS BUILDING ADDITION
NEW MARION HIGH SCHOOL
MARION C.U.S.D. #2
MARION, WILLIAMSON COUNTY, ILLINOIS



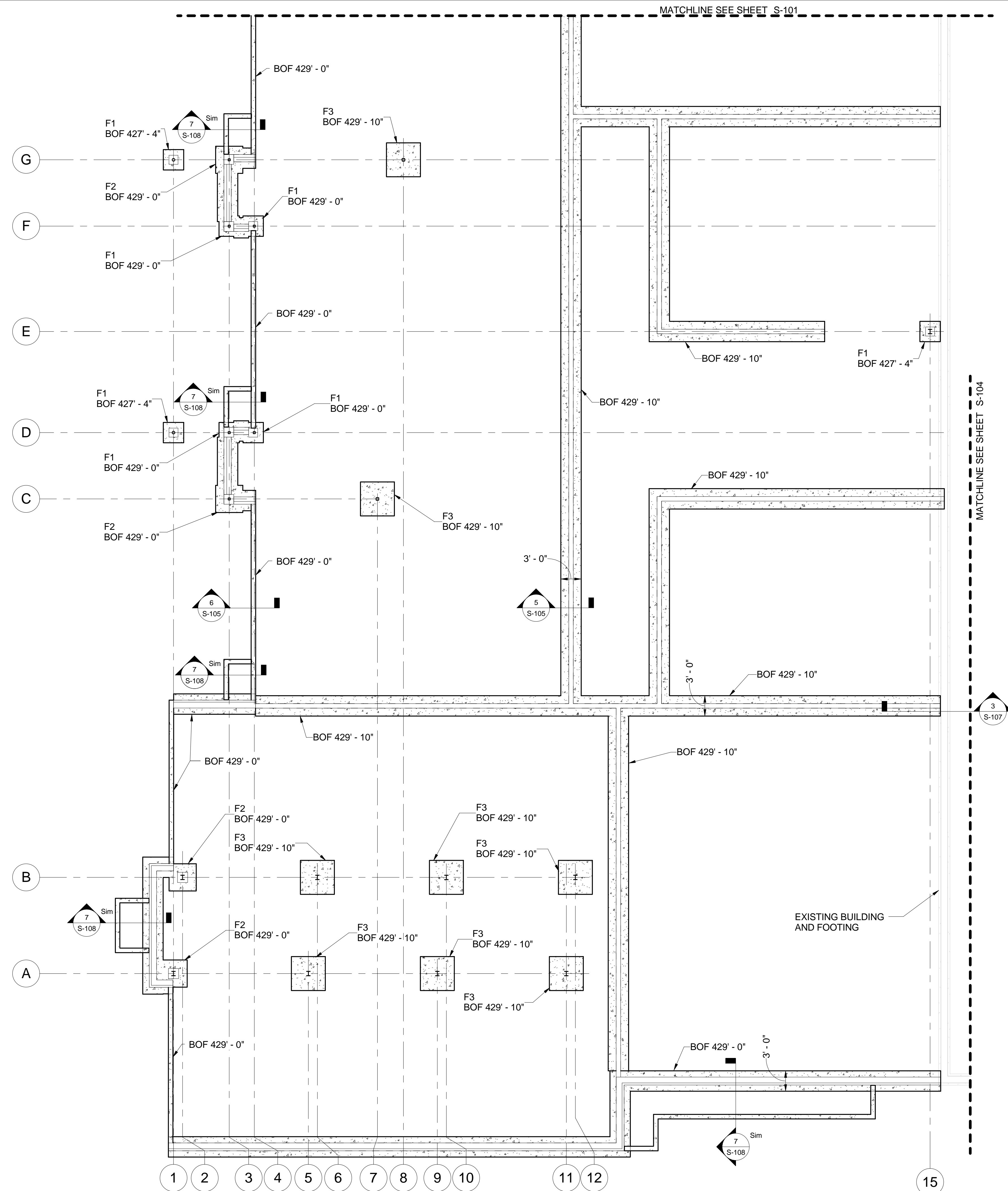
KEYPLAN

Mark	Date	Description

DATE: 03/19/15
PROJECT NO: 360-2632
DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

AREA 'B' -
FOUNDATION PLAN

S-102



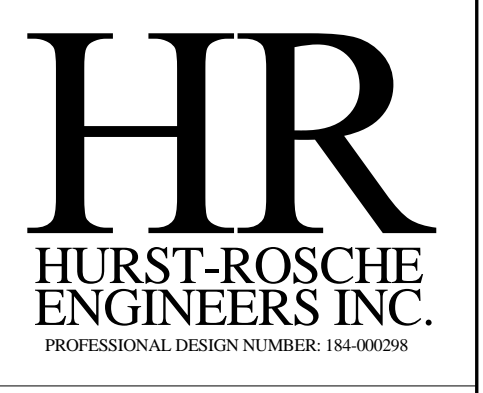
GENERAL NOTES:

- ALL INTERIOR NON-LOAD BEARING CMU WALLS SHALL HAVE A THICKENED SLAB AS SHOWN ON DETAIL 3/S-108 U.N.O. THICKENED SLAB NOT SHOWN ON SHEET FOR CLARITY.
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- REFER TO SHEET S-105 FOR STRUCTURAL FOUNDATION SCHEDULE.
- EXISTING FOUNDATIONS TO BE FIELD VERIFIED. BOF ELEVATION OF PROPOSED FOOTINGS ATTACHED TO EXISTING FOOTINGS ARE APPROXIMATE ONLY AND SHALL BE ADJUSTED UPON FIELD VERIFICATION.
- REFER TO SHEETS S-502 FOR COMPLETE COLUMN GRID LOCATIONS AND DIMENSIONS.

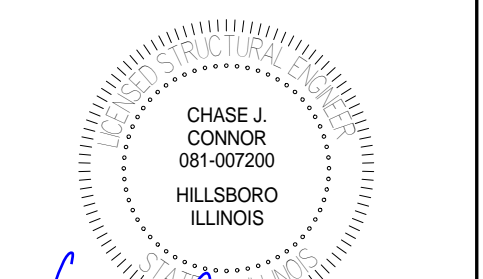


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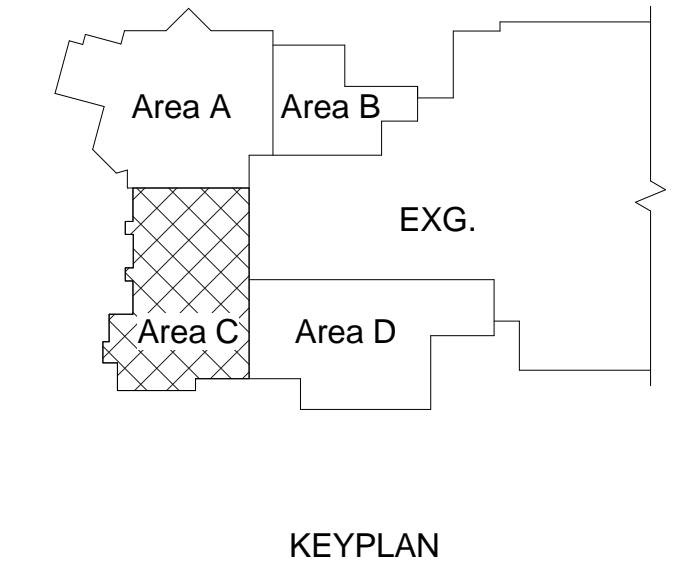


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SIGNATURE: *Chase Connor*
 DATE: 03/19/15
 LICENSE EXPIRES: 11-30-2016

PHASE 6 - FINE ARTS BUILDING ADDITION
 NEW MARION HIGH SCHOOL
 MARION C.U.S.D. #2
 MARION, WILLIAMSON COUNTY, ILLINOIS

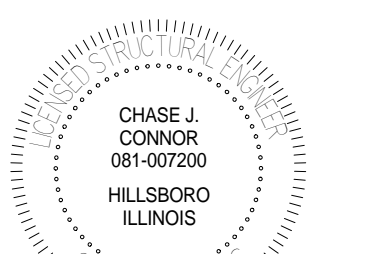


Mark	Date	Description

DATE: 03/19/15
 PROJECT NO: 360-2632
 DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.
 AREA 'C' - FOUNDATION PLAN

S-103

1 Foundation Plan - Area C
 1/8" = 1'-0"



Chase Connor
 SIGNATURE
 03/19/15
 DATE
 11-30-2016
 LICENSE EXPIRES

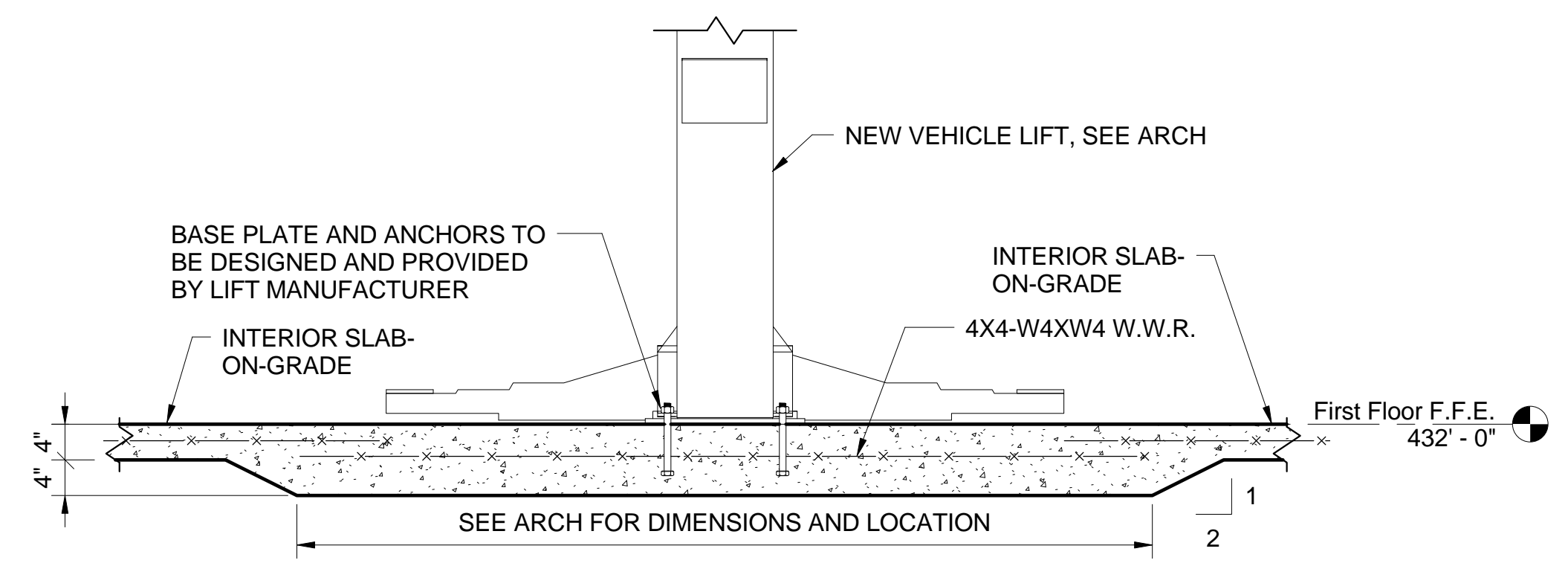
PHASE 6 - FINE ARTS BUILDING ADDITION
NEW MARION HIGH SCHOOL
MARION C.U.S.D. #2
MARION, WILLIAMSON COUNTY, ILLINOIS

Mark	Date	Description

DATE: 03/19/15
 PROJECT NO: 360-2632
 DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

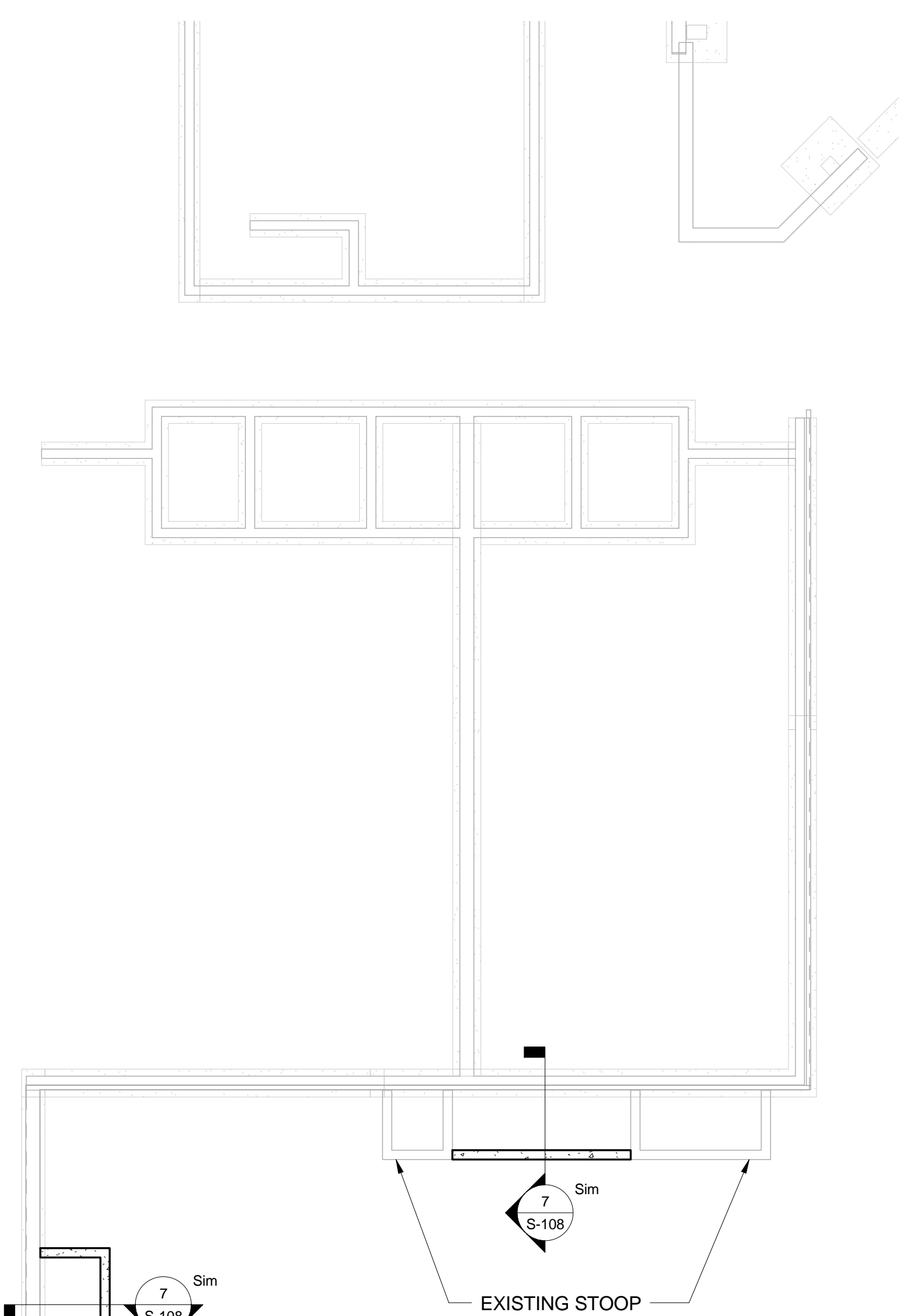
AREA 'D' -
 FOUNDATION PLAN

S-104



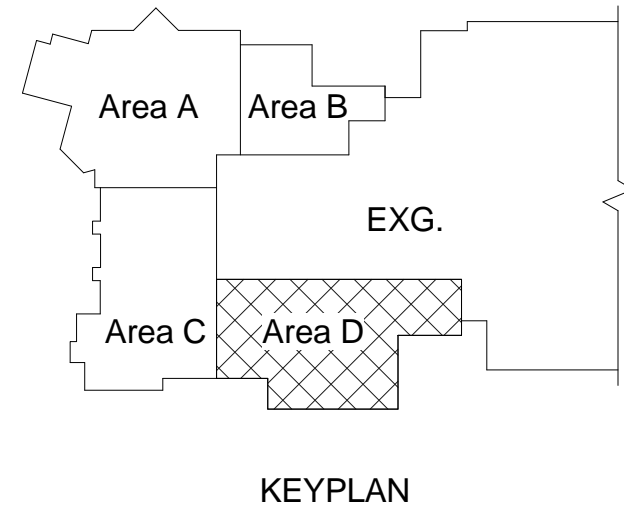
② Foundation Section
 3/4" = 1'-0"

MATCHLINE SEE SHEET S-103



GENERAL NOTES:

- EXISTING FOUNDATIONS TO BE FIELD VERIFIED. BOF ELEVATION OF PROPOSED FOOTINGS ATTACHED TO EXISTING FOOTINGS ARE APPROXIMATE ONLY AND SHALL BE ADJUSTED UPON FIELD VERIFICATION.
- SEE ARCH SHEETS FOR EXTENTS AND LOCATION OF THE INTERIOR SLAB REPLACEMENT.

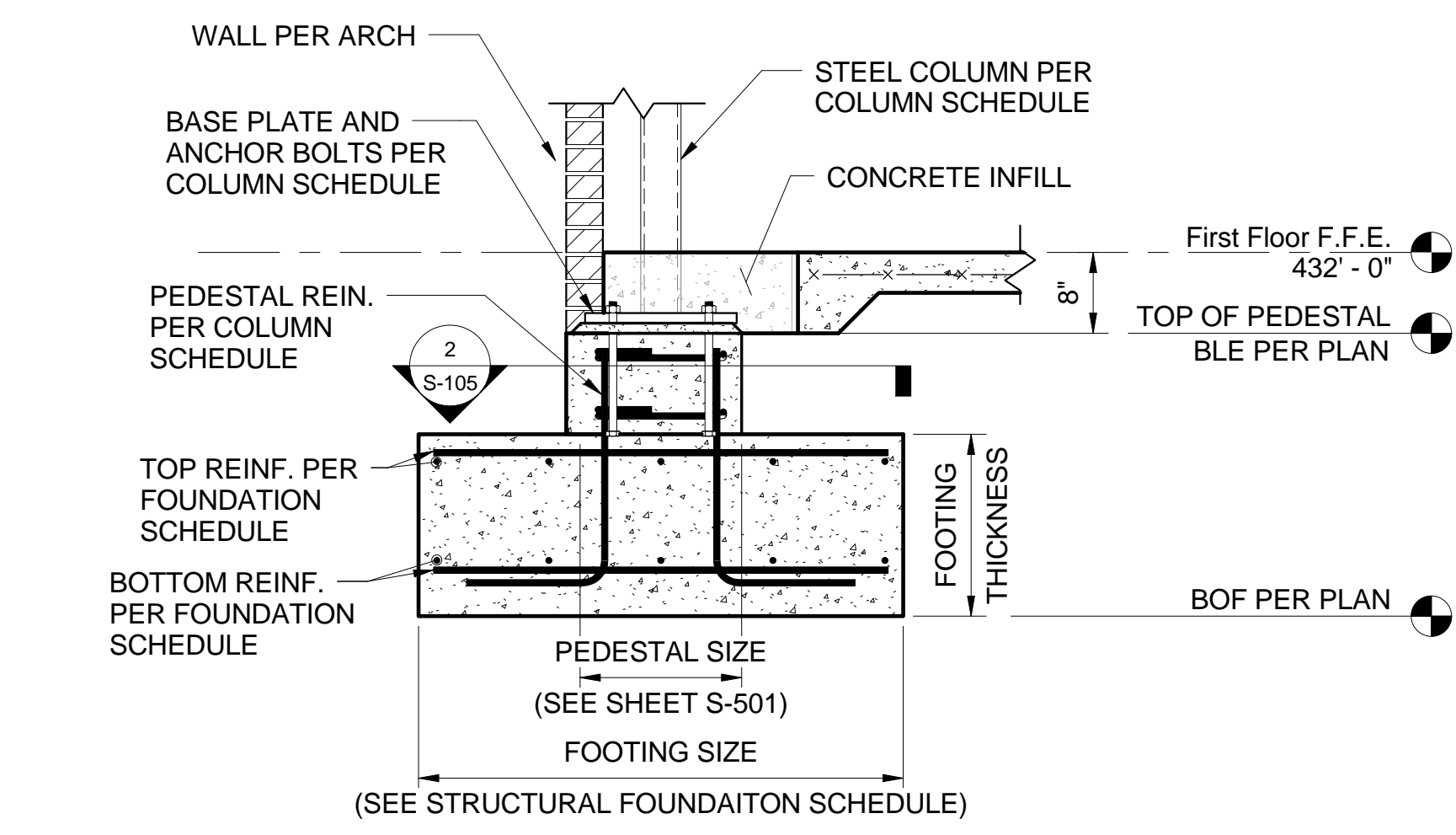


① Foundation Plan - Area D
 1/8" = 1'-0"

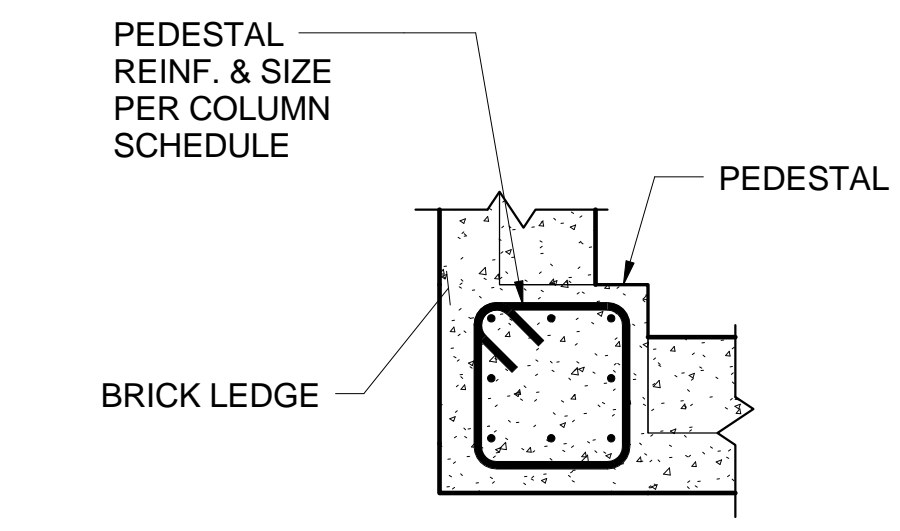
GENERAL NOTES:

- 2" PERIMETER INSULATION SHALL BE PLACED ON THE INSIDE OF ALL EXTERIOR FOUNDATIONS. SEE ARCH SHEETS.
- REFER TO SHEET S-001 & S-002 FOR ADDITIONAL INFORMATION.
- WALL DETAILS SHOWN ARE FOR STRUCTURAL REPRESENTATION ONLY, SEE ARCH SHEETS FOR SIZE, TEXTURE, COLOR, ETC.
- SEE SHEET S-501 FOR COLUMN SCHEDULE.
- SEE ARCH & CIVIL FOR WEEP HOLE LOCATIONS AND DETAILS.
- FOUNDATION REINFORCEMENT HOOKED BAR DEVELOPMENT LENGTH AS FOLLOWS U.I.O.:
#5 BAR.....1'-0"
#7 BAR.....1'-6"

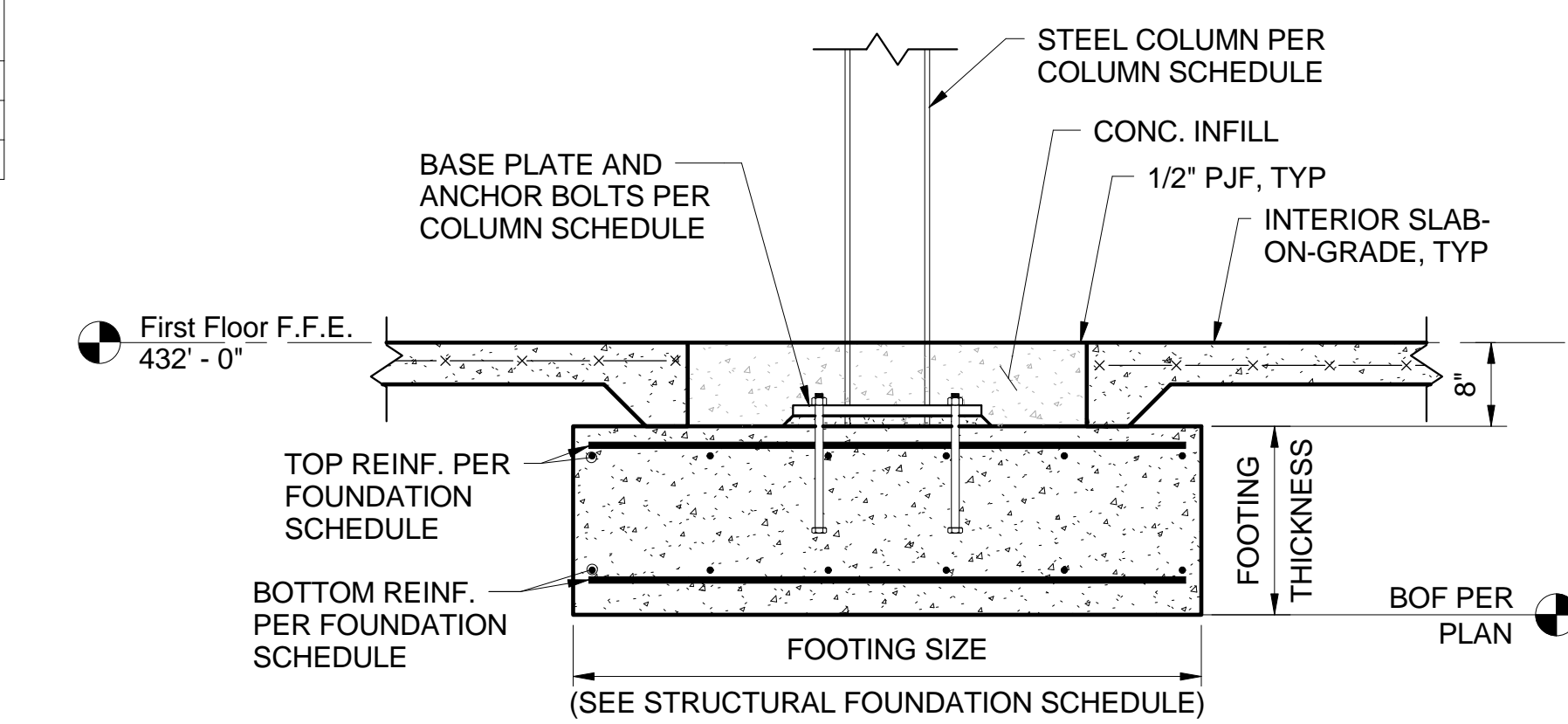
STRUCTURAL FOUNDATION SCHEDULE					
Type Mark	Width	Length	Foundation Thickness	Top Reinf.	Bottom Reinf.
F1	3'-0"	3'-0"	1'-6"	#5 @ 1'-0"	#5 @ 1'-0"
F2	4'-0"	4'-0"	1'-6"	#5 @ 1'-0"	#5 @ 1'-0"
F3	5'-0"	5'-0"	1'-6"	#5 @ 1'-0"	#5 @ 1'-0"



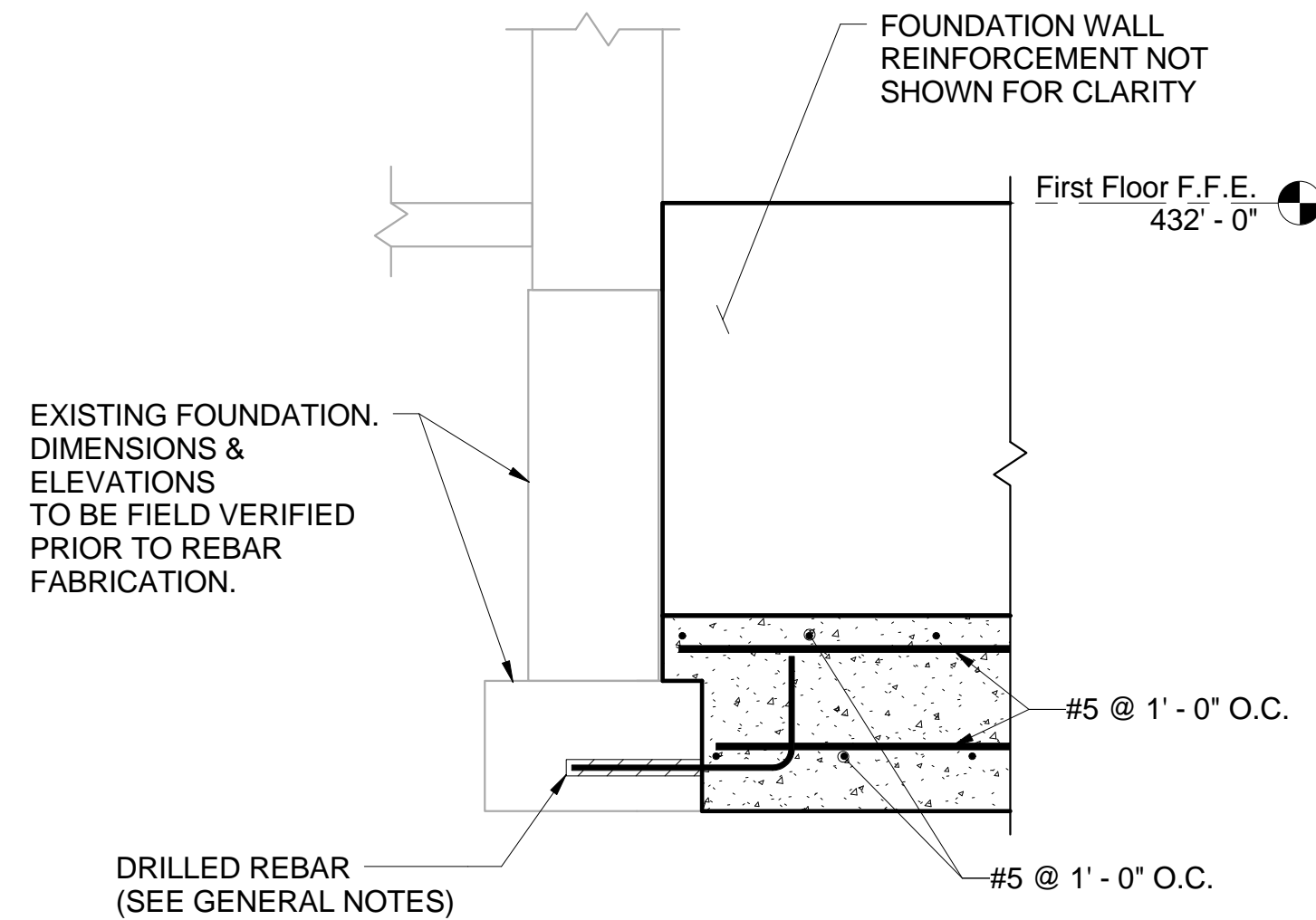
1 Typical Exterior Column Footing
3/4" = 1'-0"



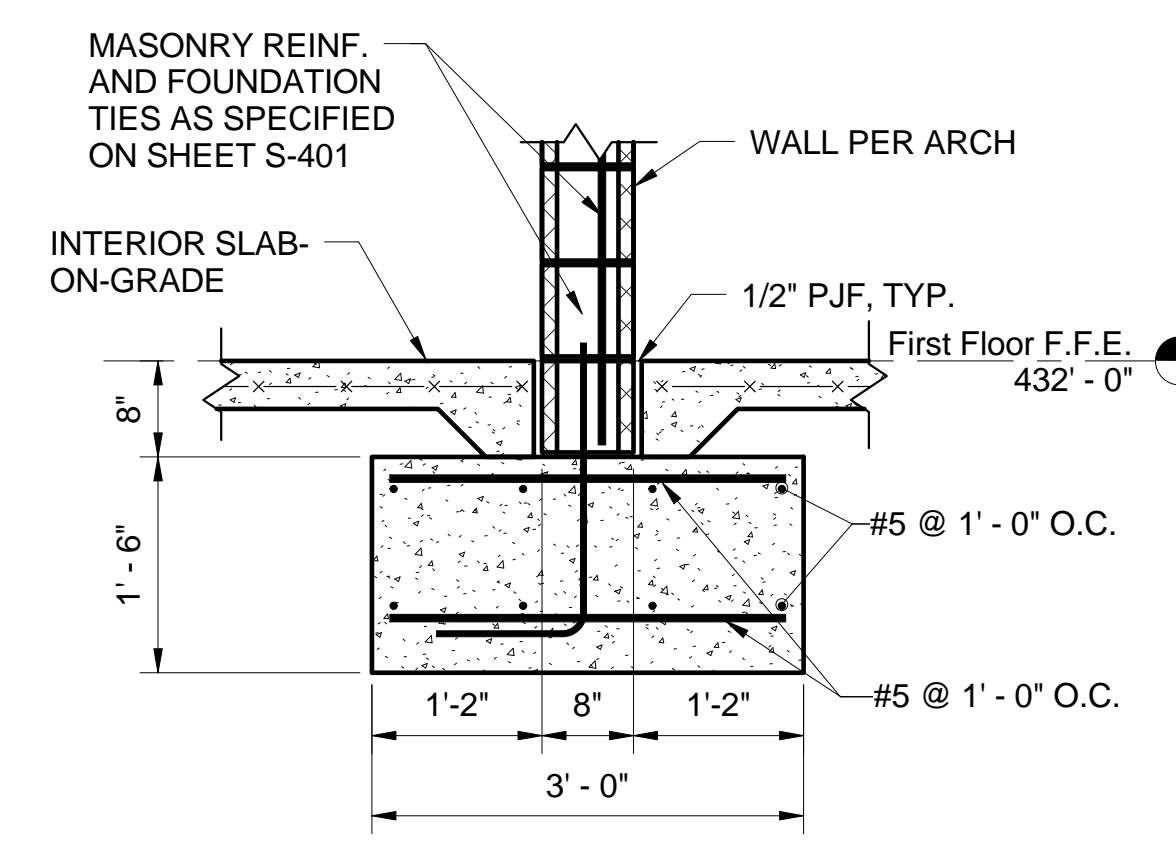
2 Typical Pedestal Section
3/4" = 1'-0"



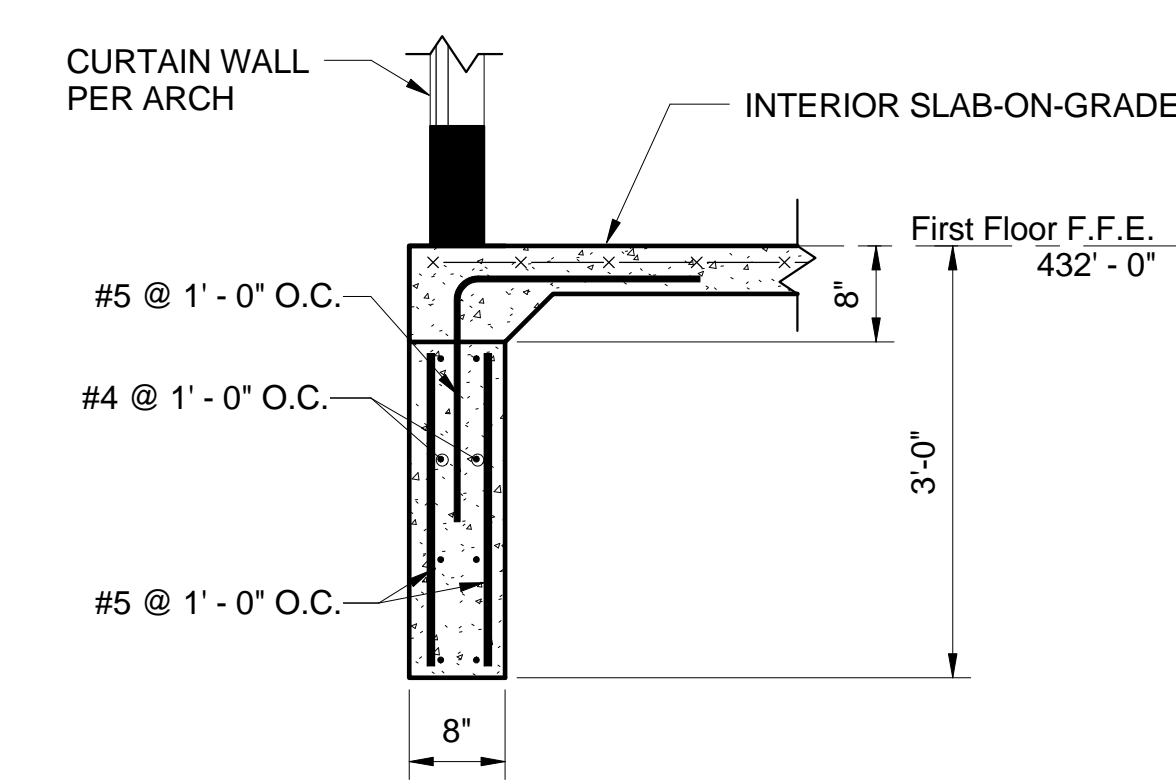
3 Typical Interior Column Footing
3/4" = 1'-0"



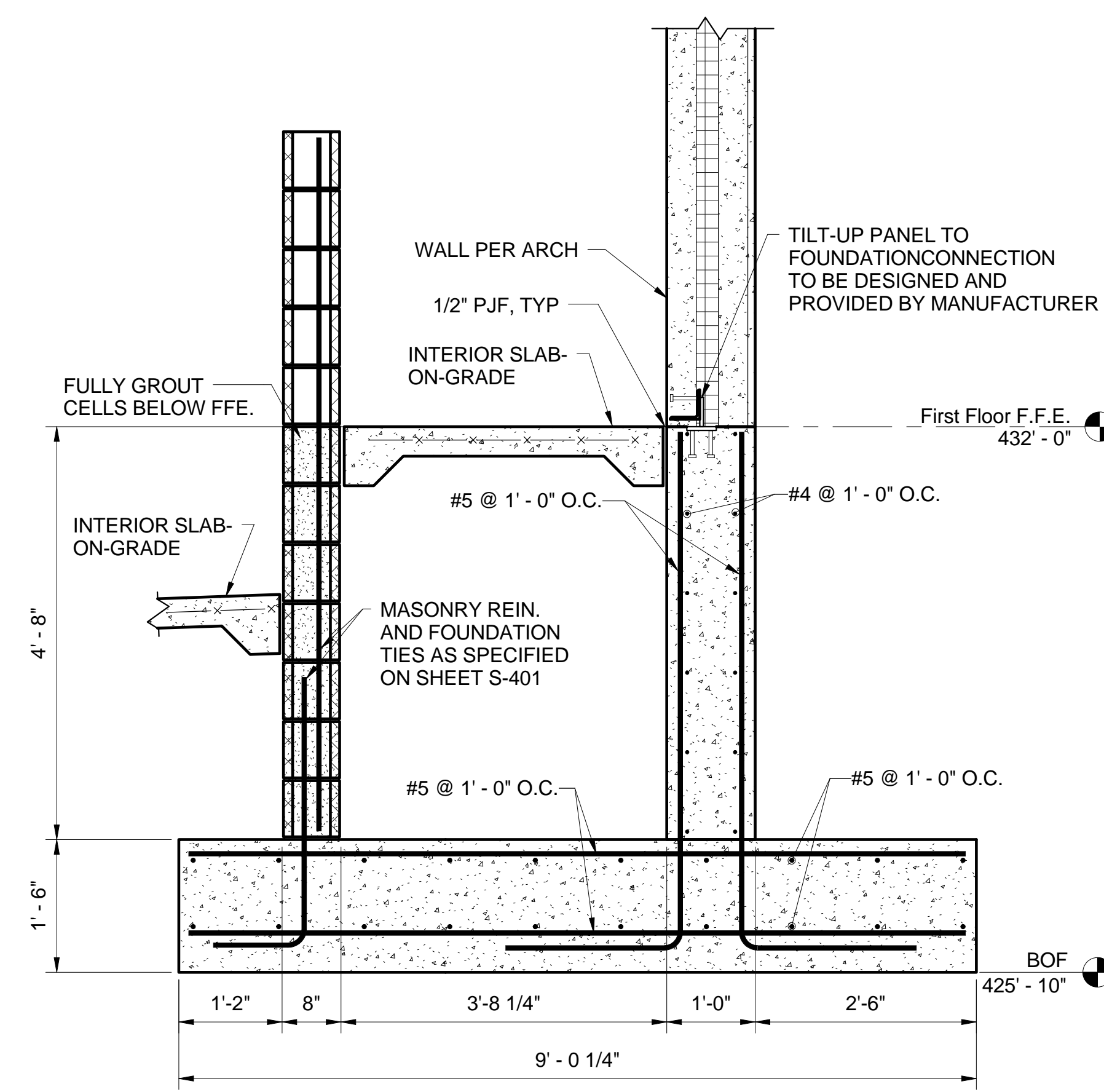
4 Foundation Section
3/4" = 1'-0"



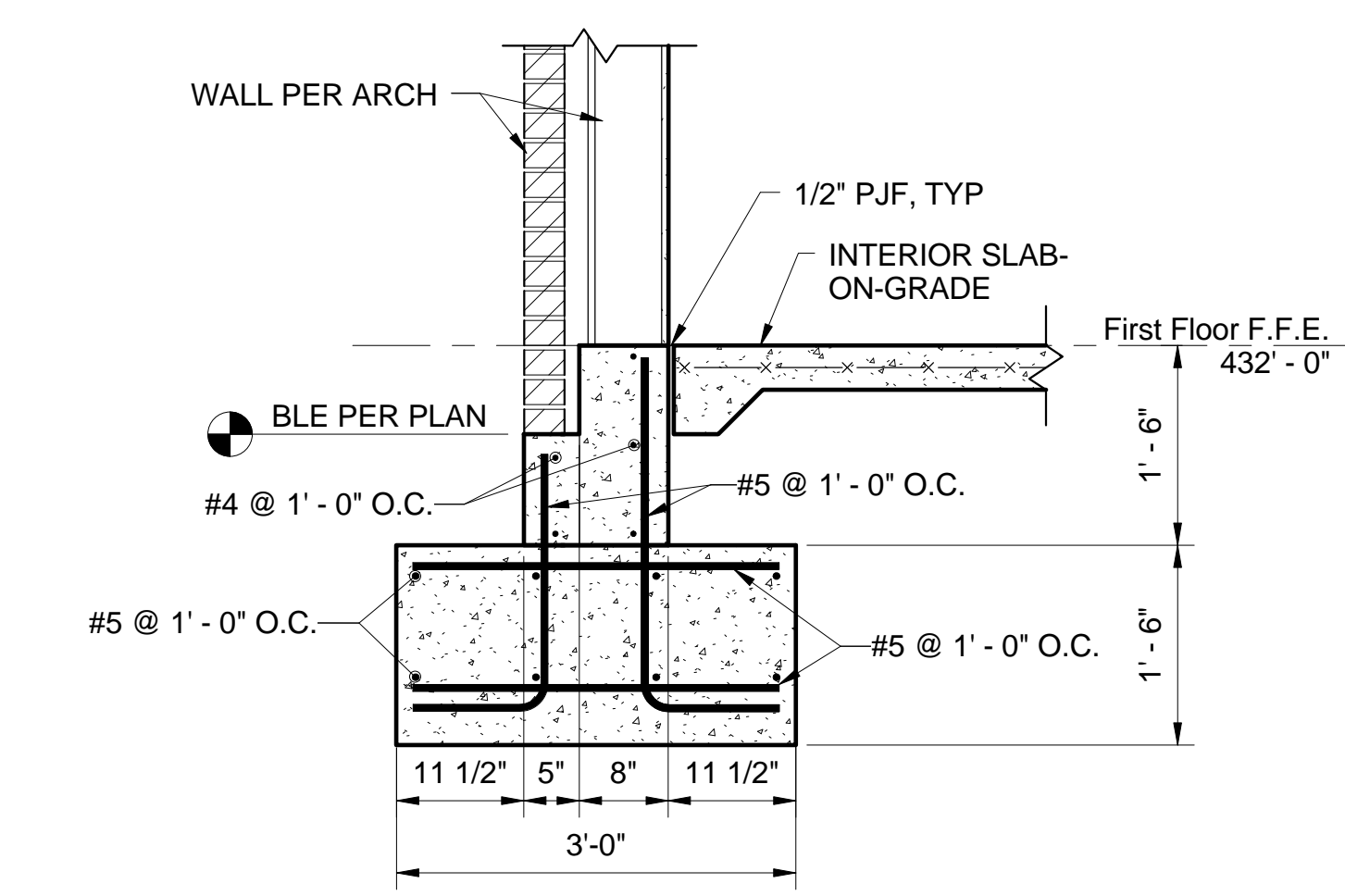
5 Foundation Section
3/4" = 1'-0"



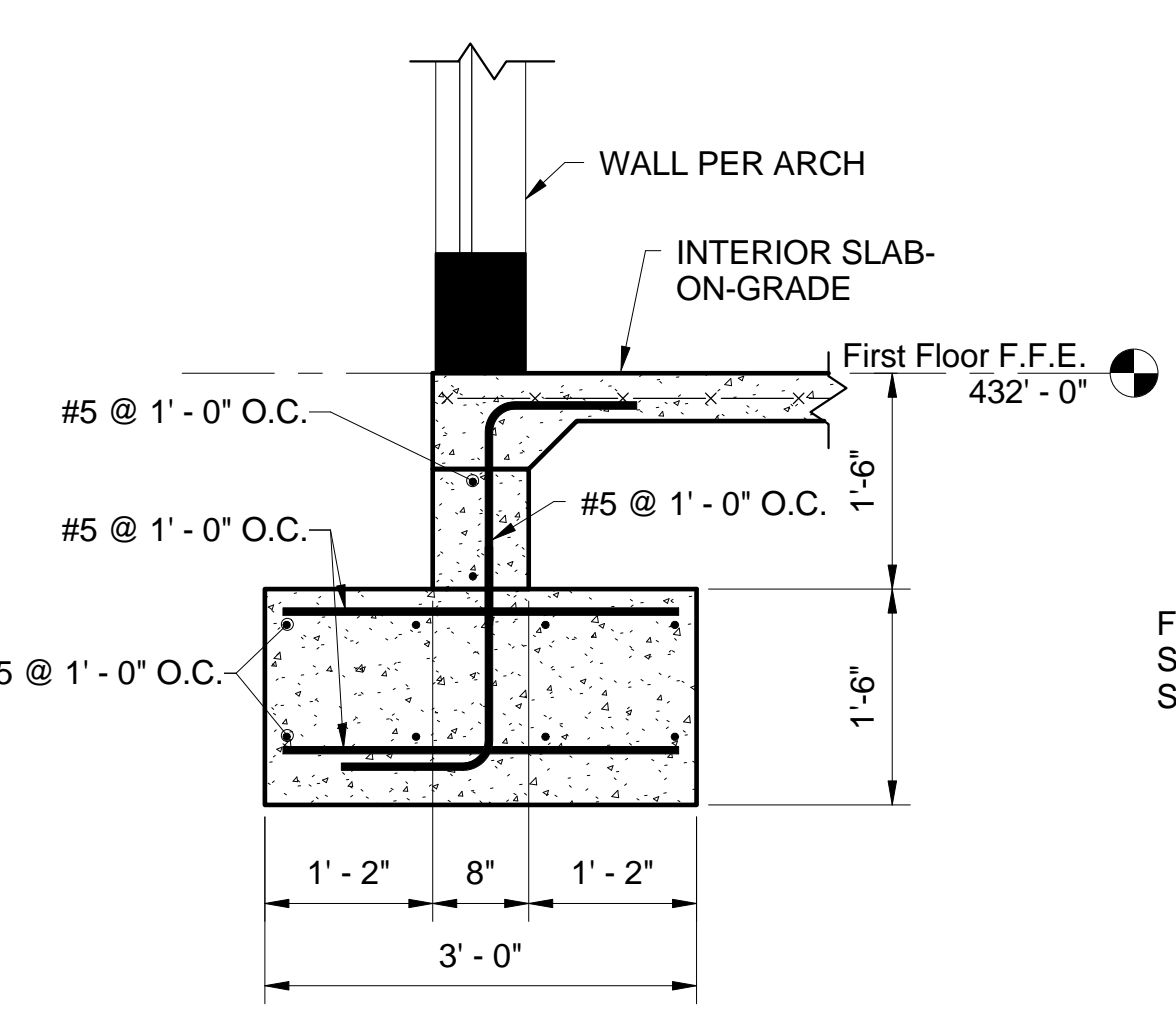
6 Foundation Section
3/4" = 1'-0"



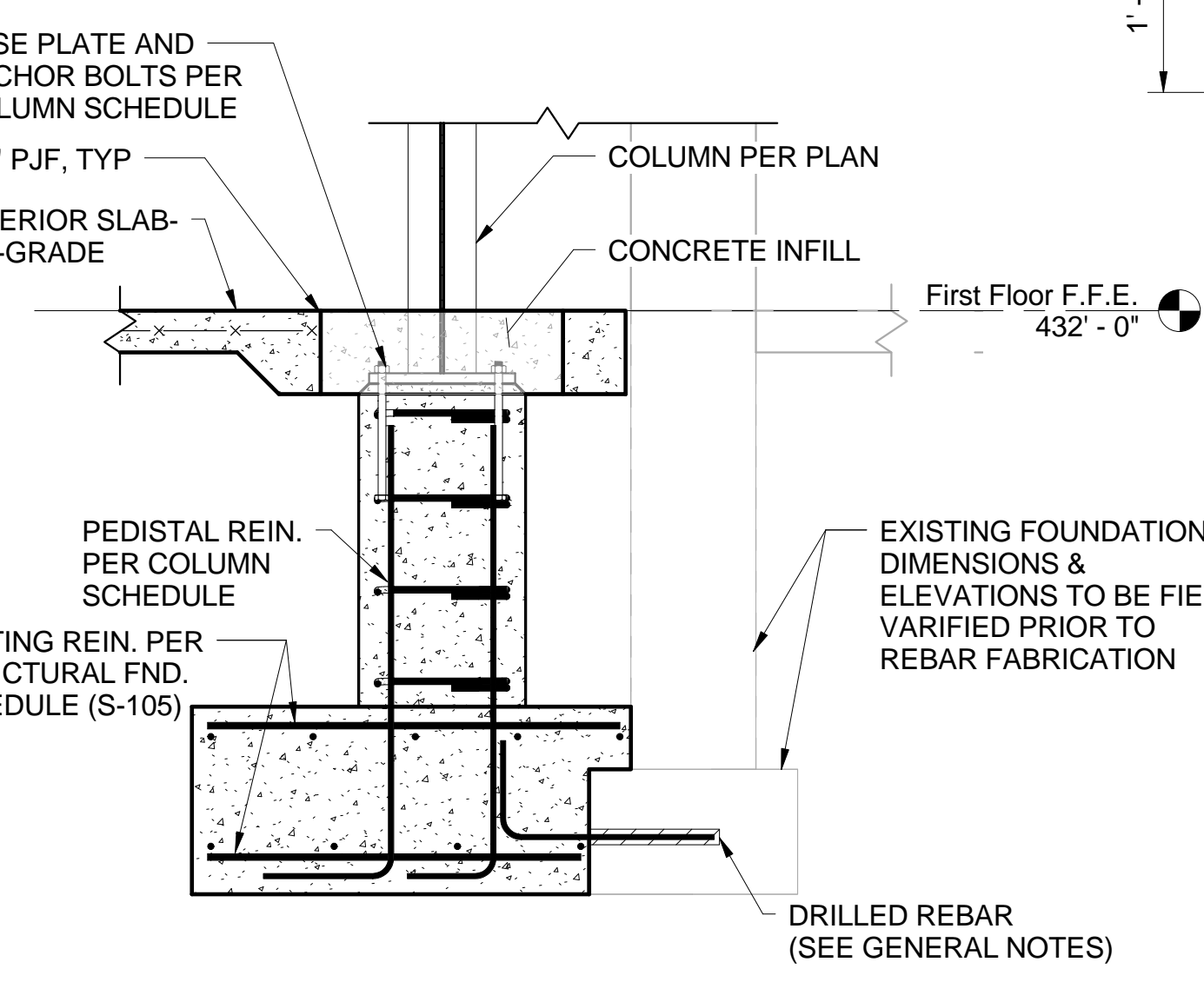
10 Foundation Section
3/4" = 1'-0"



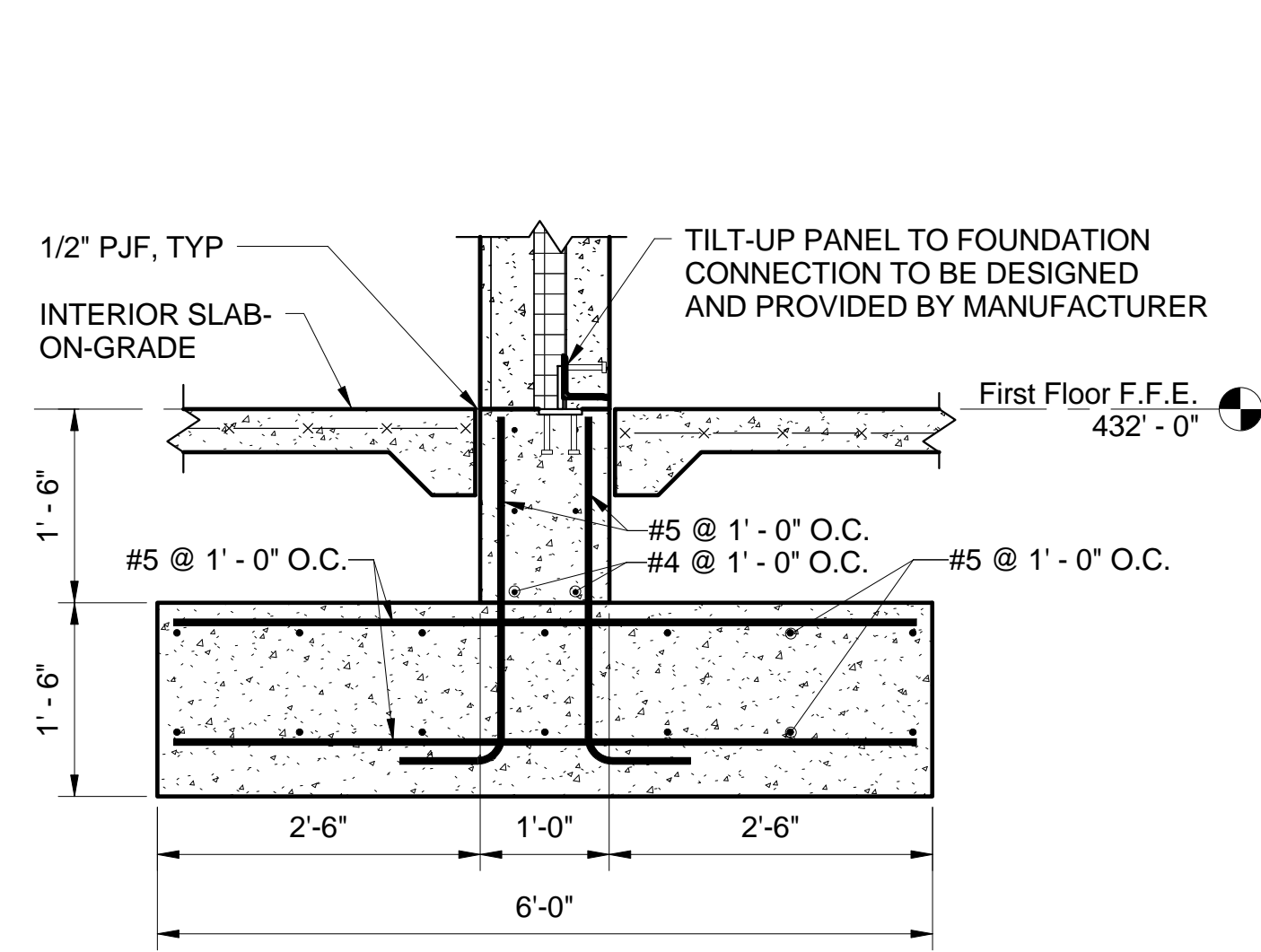
7 Foundation Section
3/4" = 1'-0"



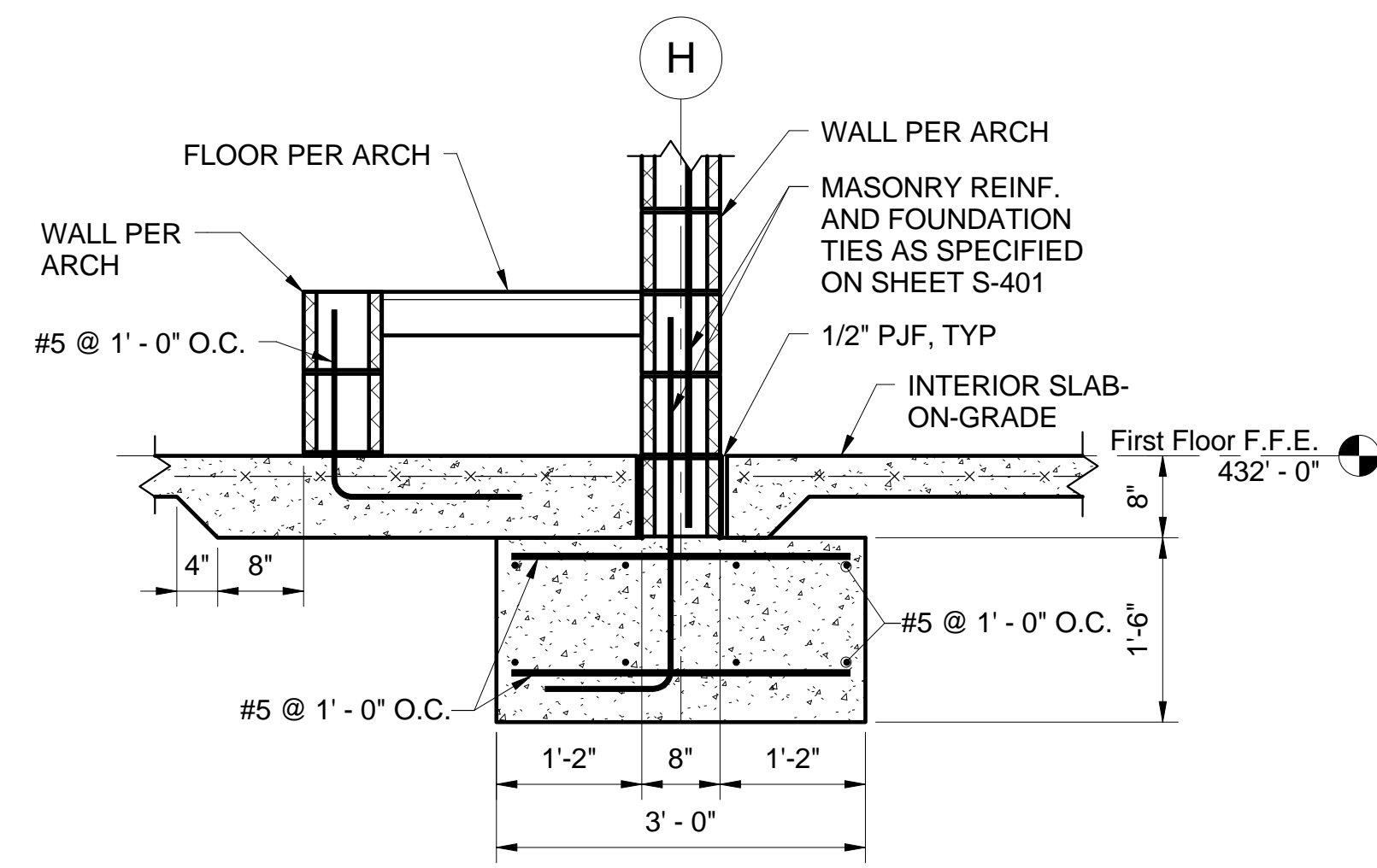
8 Foundation Section
3/4" = 1'-0"



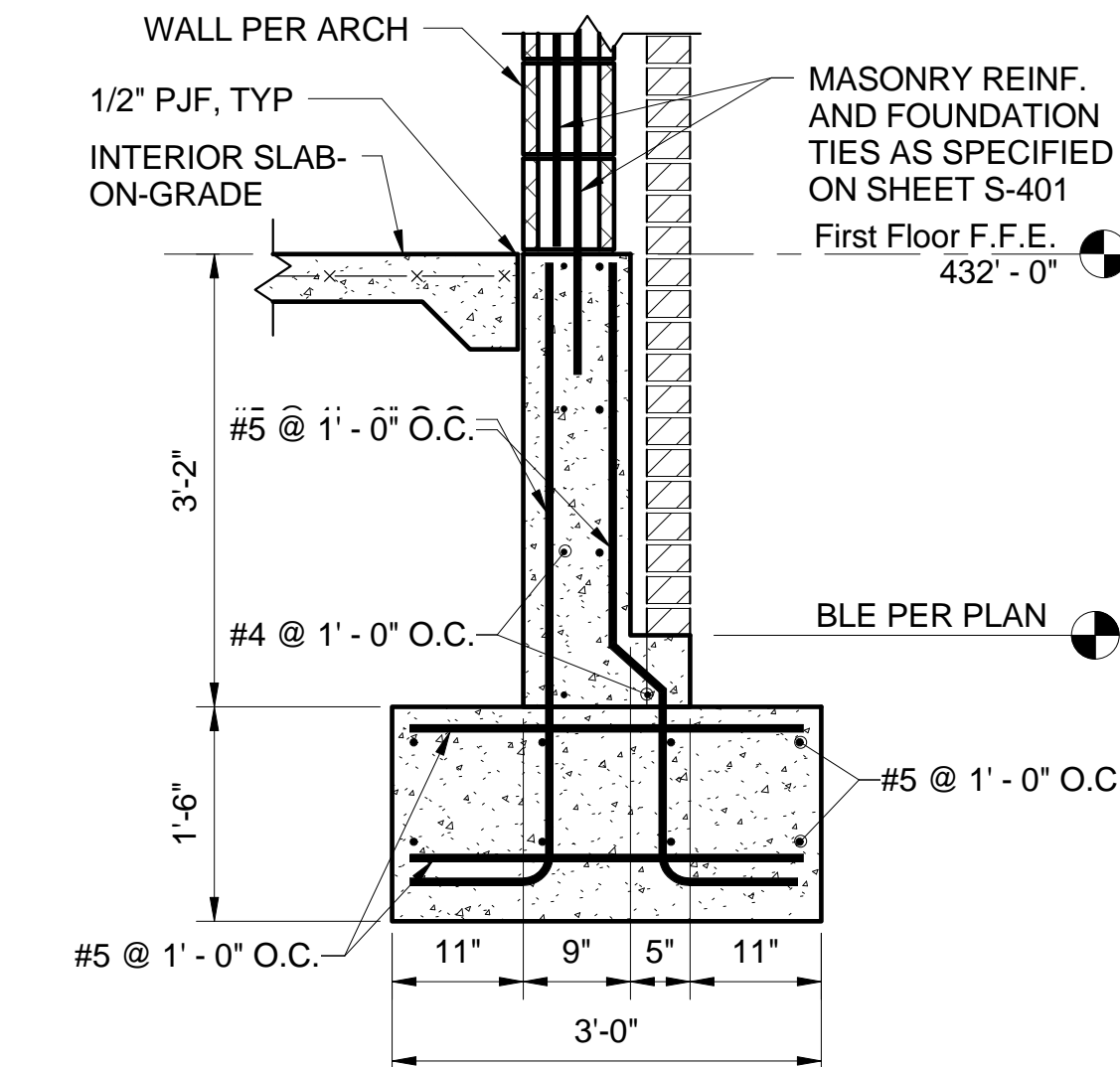
9 Foundation Section
3/4" = 1'-0"



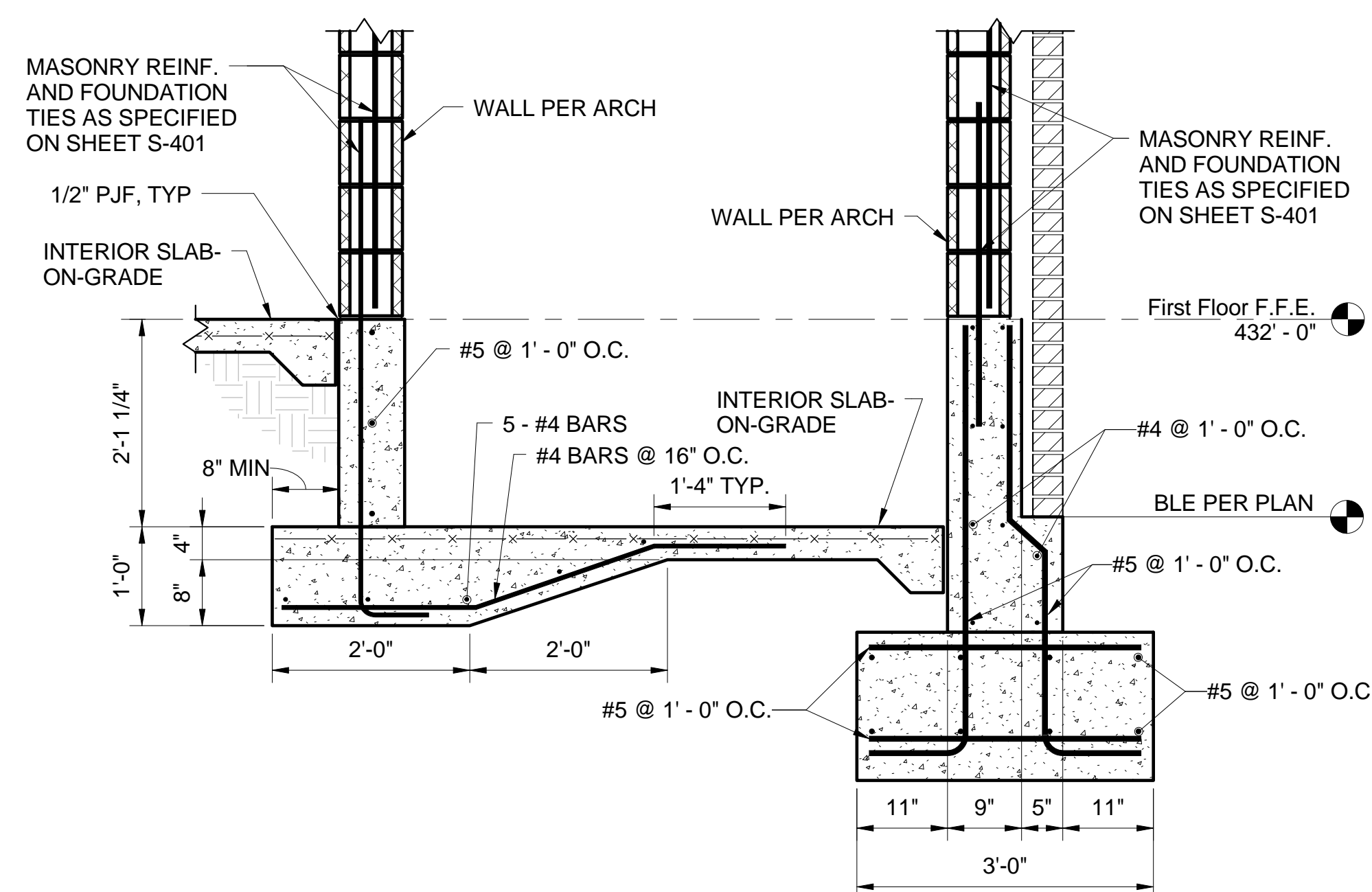
① Foundation Section
3/4" = 1'-0"



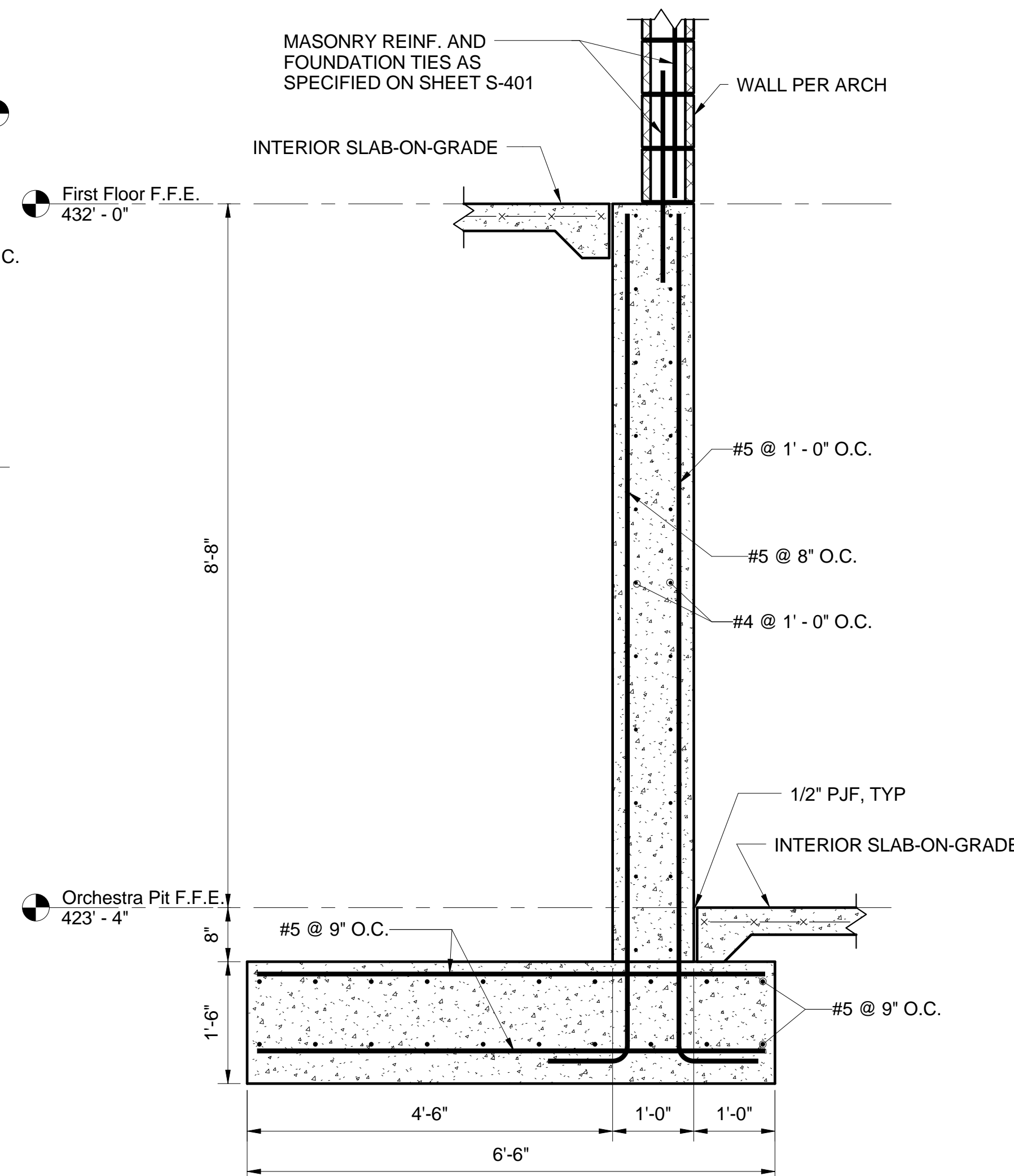
② Foundation Section
3/4" = 1'-0"



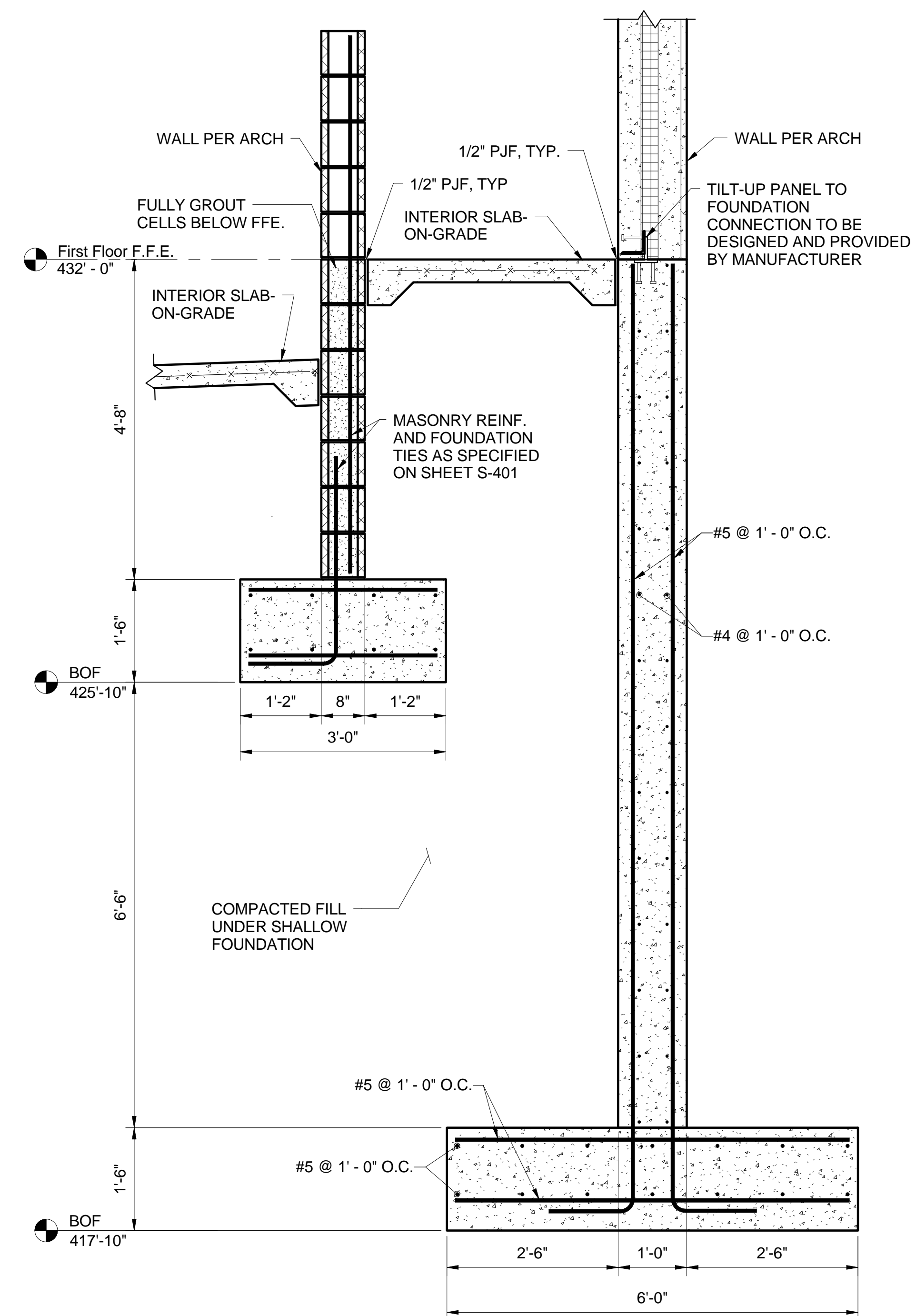
③ Foundation Section
3/4" = 1'-0"



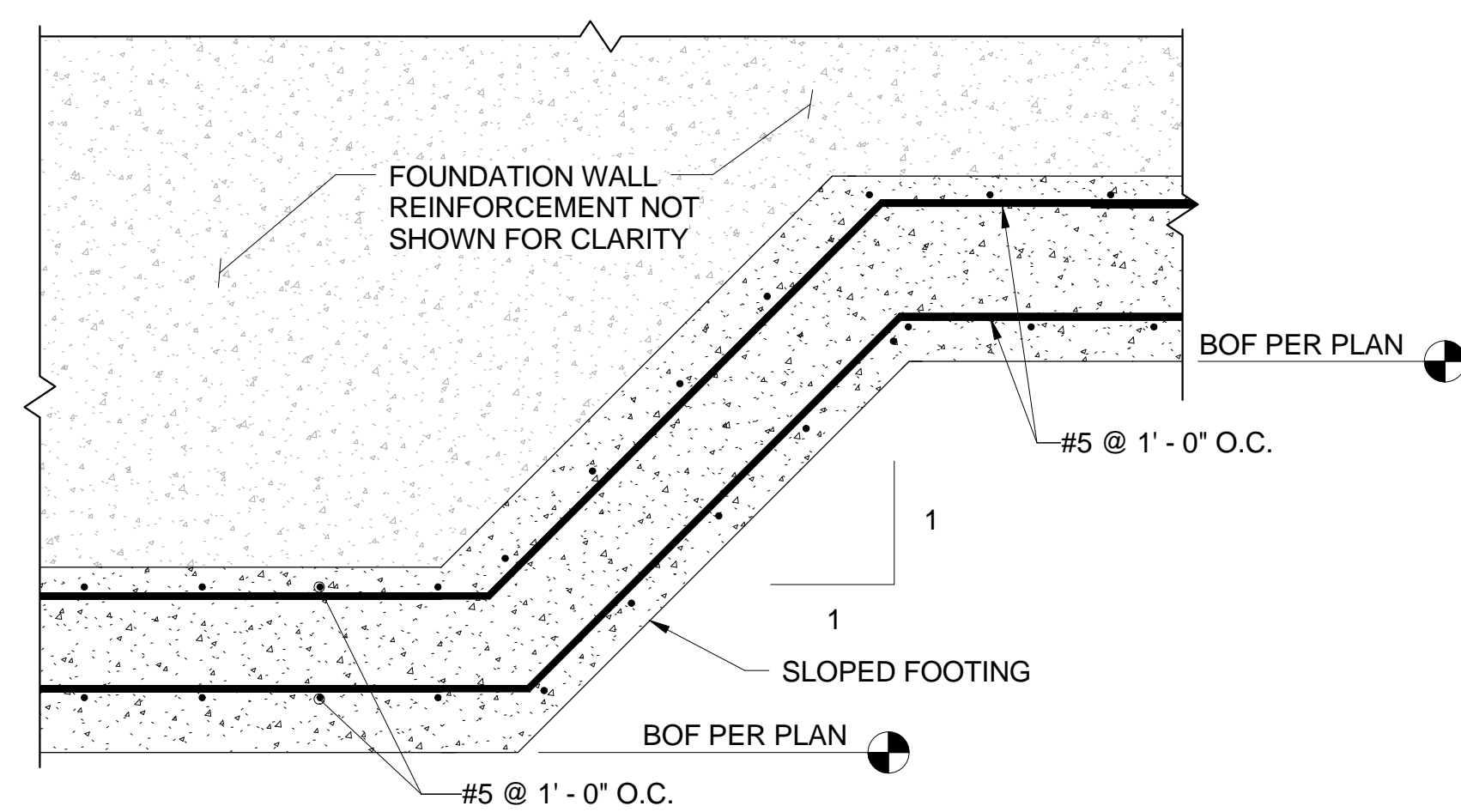
④ Foundation Section
3/4" = 1'-0"



⑥ Foundation Section
3/4" = 1'-0"



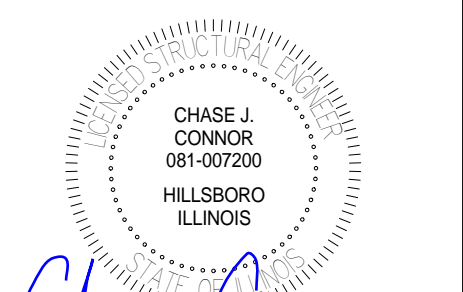
⑦ Foundation Section
3/4" = 1'-0"



⑤ Foundation Section
3/4" = 1'-0"

GENERAL NOTES:

- 2" PERIMETER INSULATION SHALL BE PLACED ON THE INSIDE OF ALL EXTERIOR FOUNDATIONS. SEE ARCH SHEETS.
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- SEE ARCH & CIVIL FOR WEEP HOLE LOCATIONS AND DETAILS.
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#5 BAR.....1'-0"
#7 BAR.....1'-6"



Signature: *Chase Connor*

DATE: 03/19/15

DATE: 11-30-2016

LICENSE EXPIRES

PHASE 6 - FINE ARTS BUILDING ADDITION
NEW MARION HIGH SCHOOL
MARION C.U.S.D. #2
MARION, WILLIAMSON COUNTY, ILLINOIS

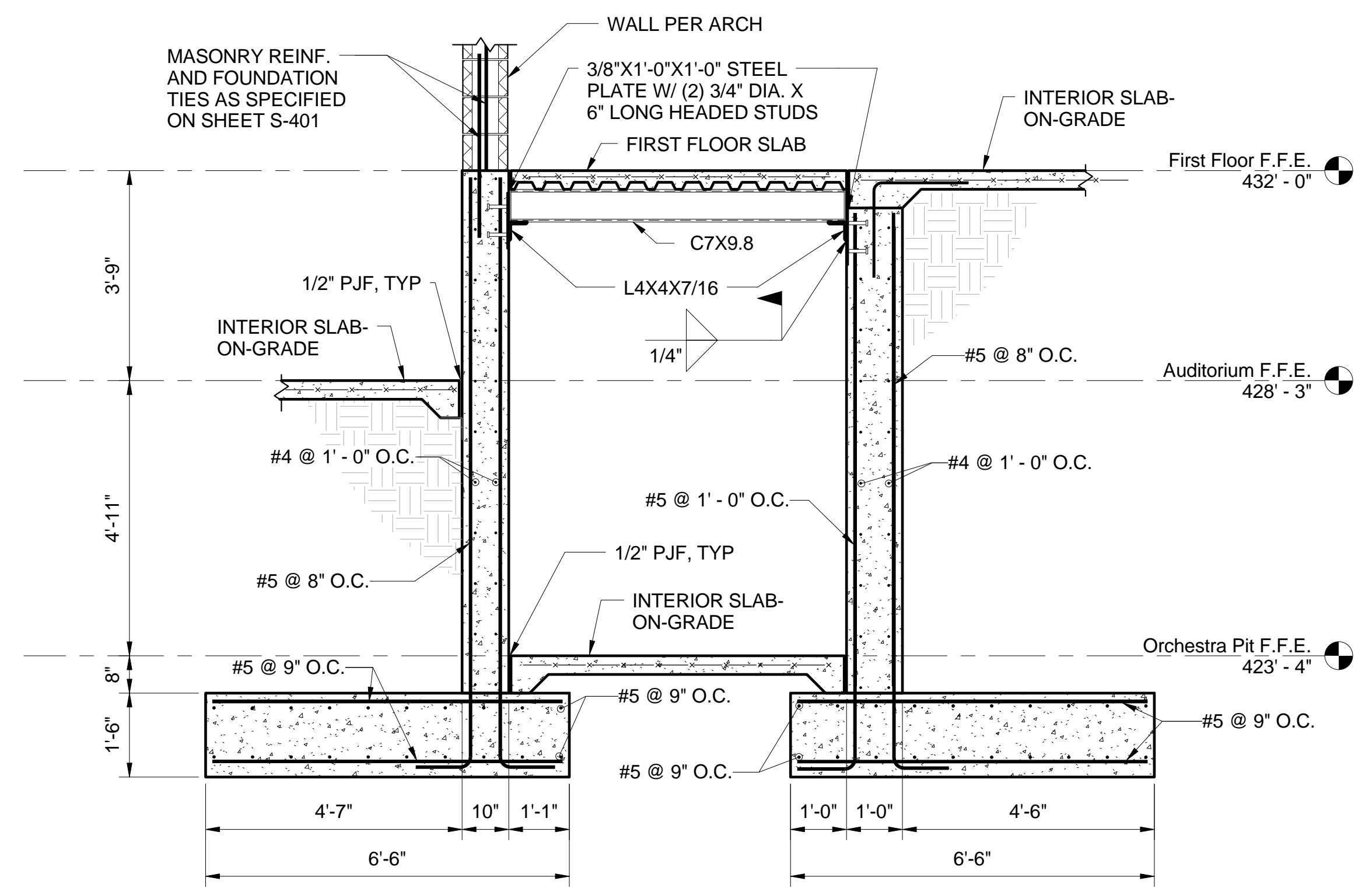
Mark	Date	Description

DATE: 03/19/15

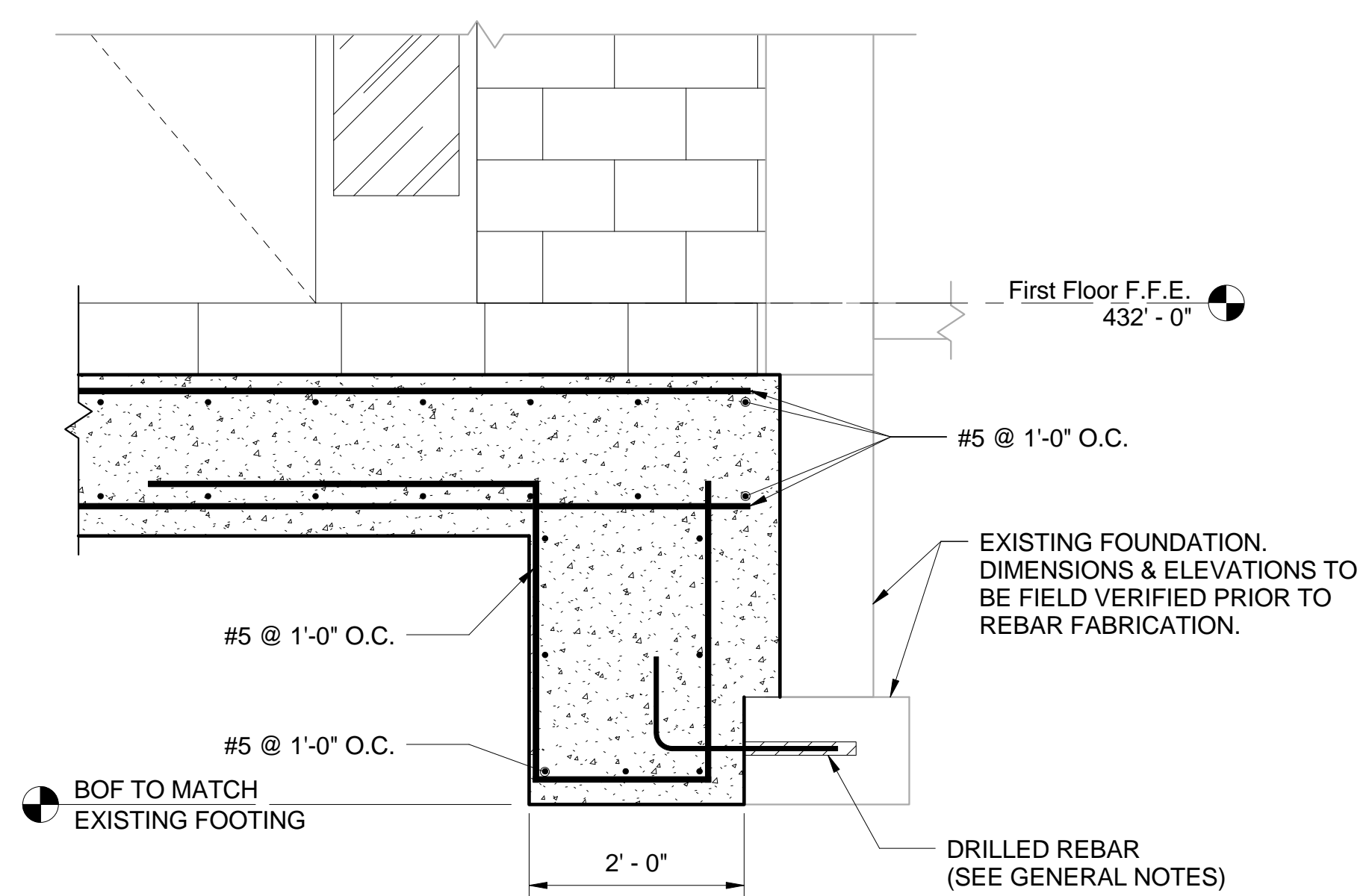
PROJECT NO: 360-2632

DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

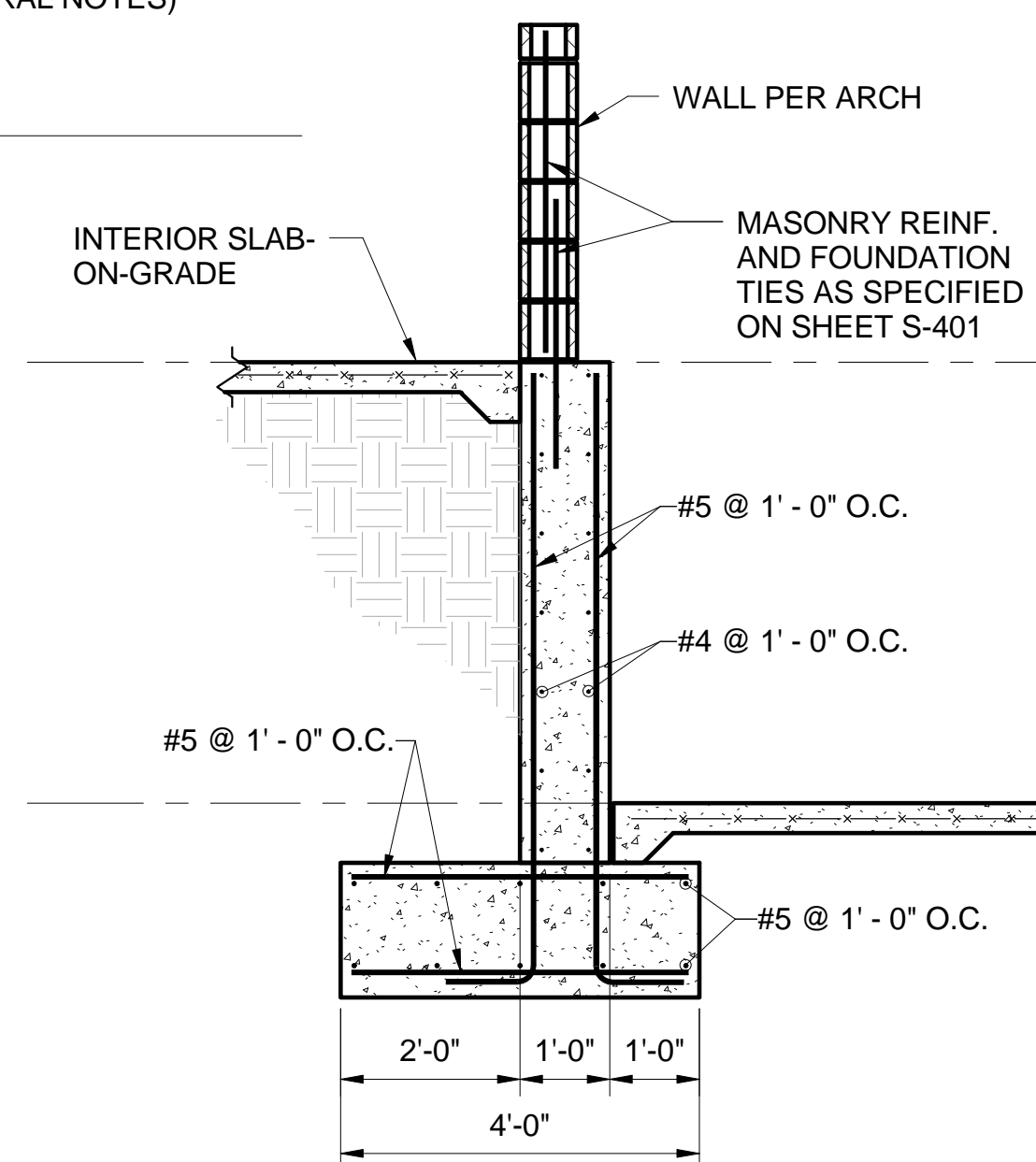
FOUNDATION DETAILS



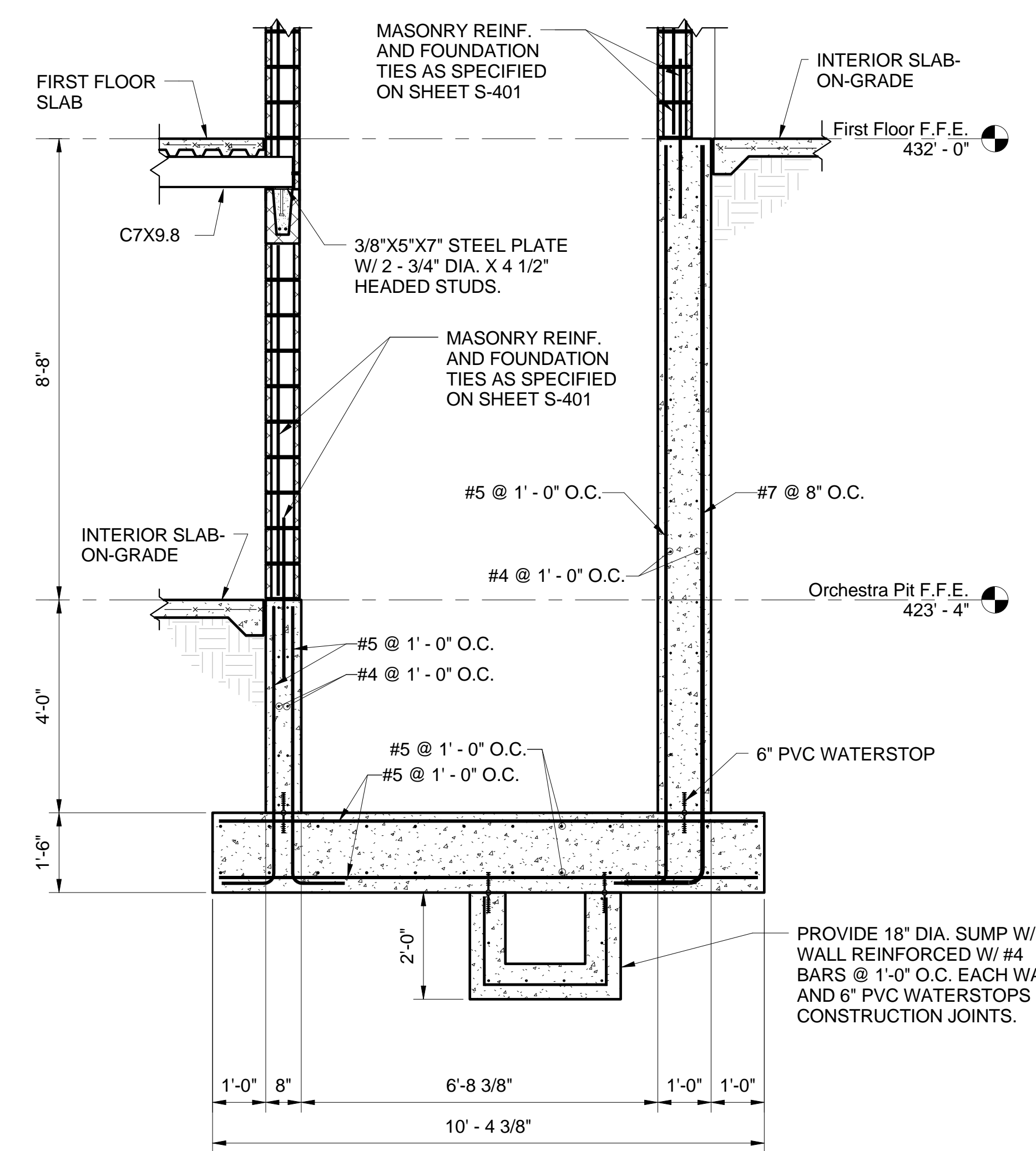
1 Foundation Section
1/2" = 1'-0"



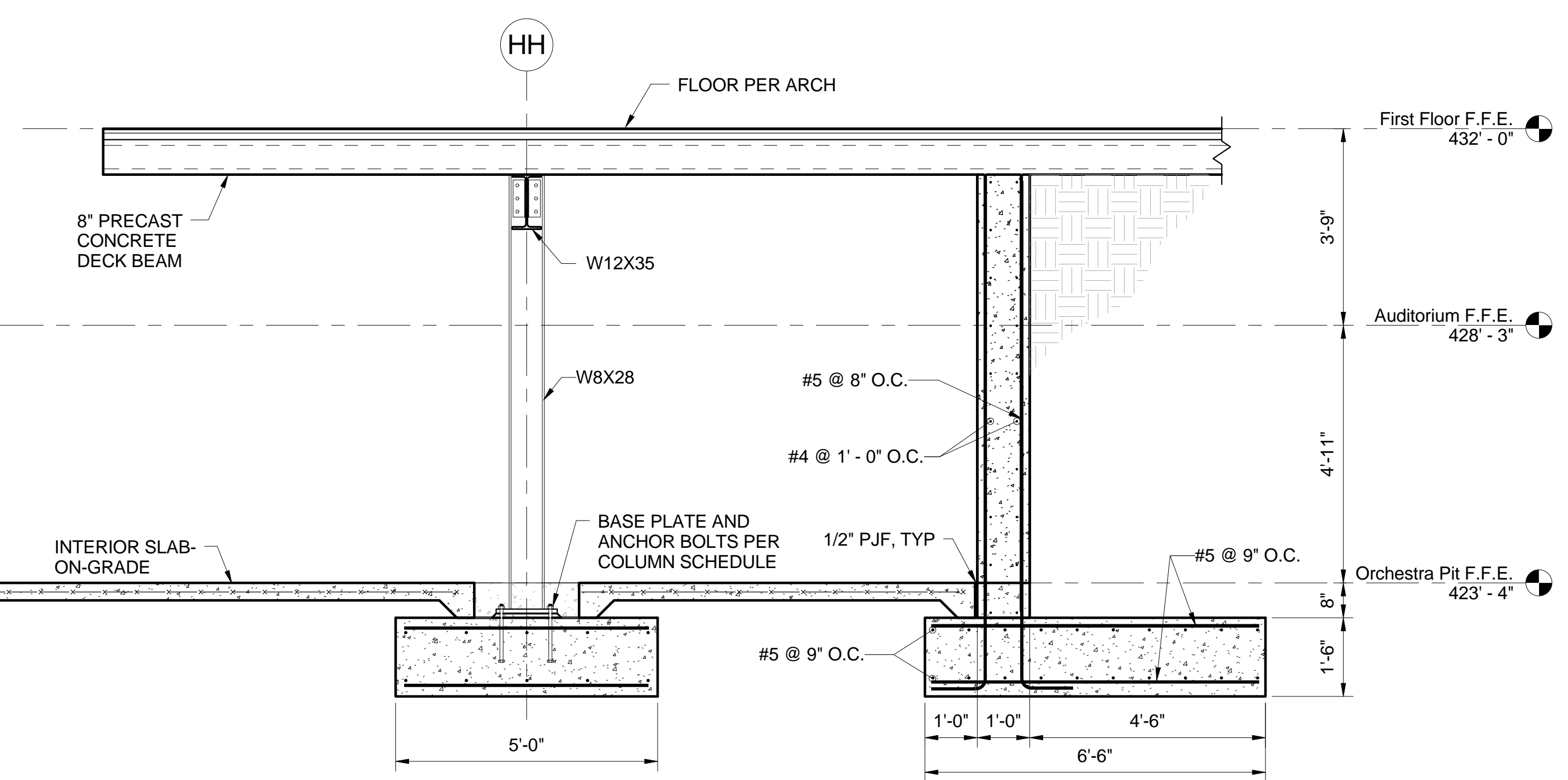
3 Foundation Section
3/4" = 1'-0"



4 Foundation Section
1/2" = 1'-0"



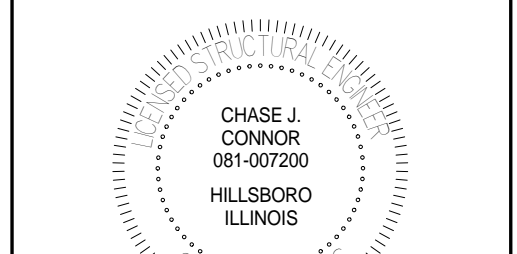
2 Elevator Pit Section
1/2" = 1'-0"



4 Foundation Section
1/2" = 1'-0"

GENERAL NOTES:

- 2" PERIMETER INSULATION SHALL BE PLACED ON THE INSIDE OF ALL EXTERIOR FOUNDATIONS. SEE ARCH SHEETS.
- REFER TO SHEET S-001 & S-002 FOR ADDITIONAL INFORMATION.
- WALL DETAILS SHOWN ARE FOR STRUCTURAL REPRESENTATION ONLY, SEE ARCH SHEETS FOR SIZE, TEXTURE, COLOR, ETC.
- SEE SHEET S-501 FOR COLUMN SCHEDULE.
- SEE ARCH & CIVIL FOR WEEP HOLE LOCATIONS AND DETAILS.
- FOUNDATION REINFORCEMENT HOOKED BAR DEVELOPMENT LENGTH AS FOLLOWS U.N.O.:
#5 BAR.....1'-0"
#7 BAR.....1'-6"



Signature: *Chase Connor*
DATE: 03/19/15
DATE: 11-30-2016
LICENSE EXPIRES

PHASE 6 - FINE ARTS BUILDING ADDITION
NEW MARION HIGH SCHOOL
MARION C.U.S.D. #2
MARION, WILLIAMSON COUNTY, ILLINOIS

Mark	Date	Description

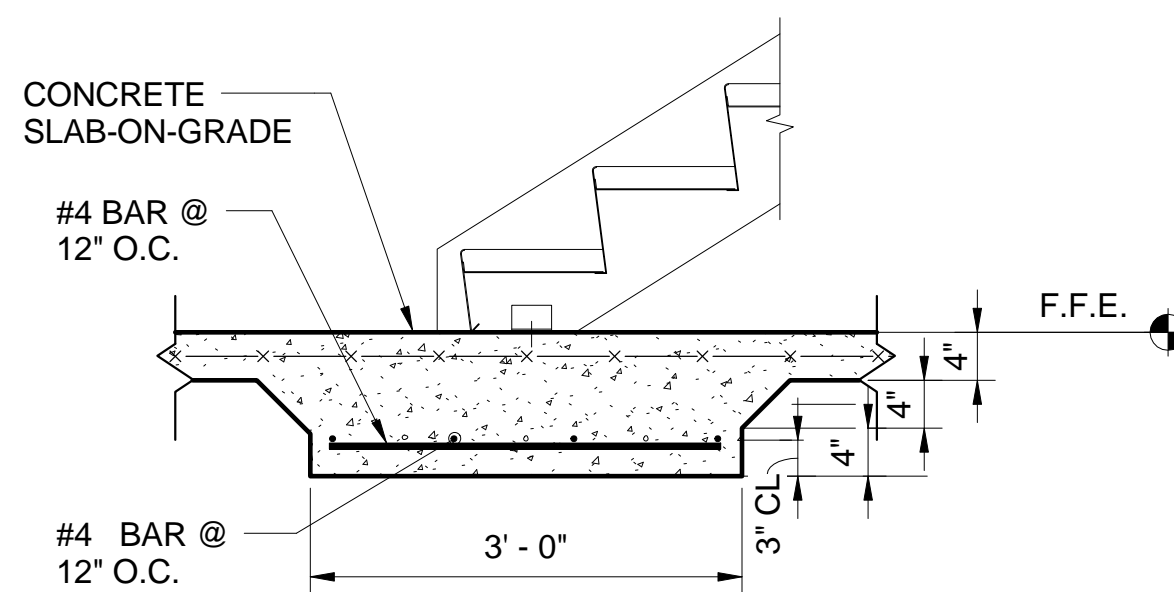
DATE: 03/19/15
PROJECT NO: 360-2632
DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

FOUNDATION DETAILS

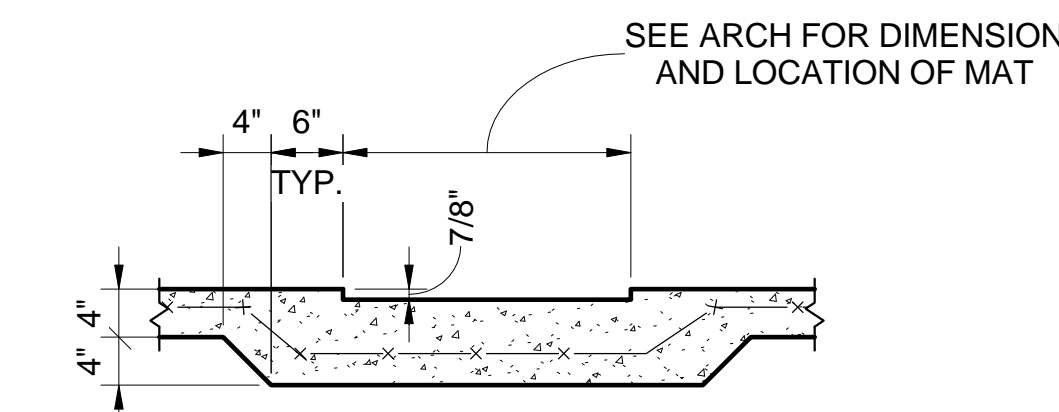
Mark	Date	Description

DATE: 03/19/15
PROJECT NO: 360-2632
DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

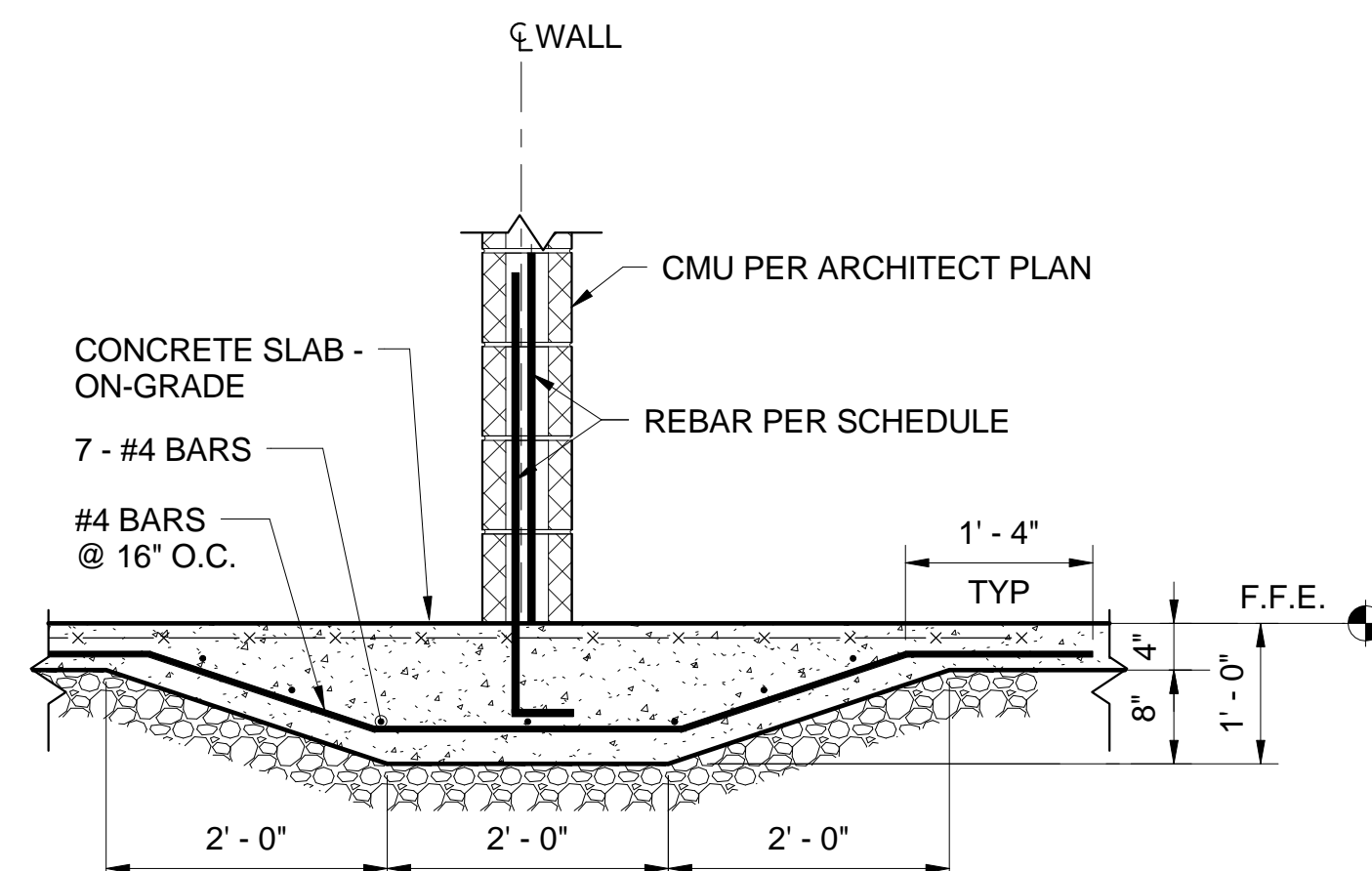
FOUNDATION DETAILS



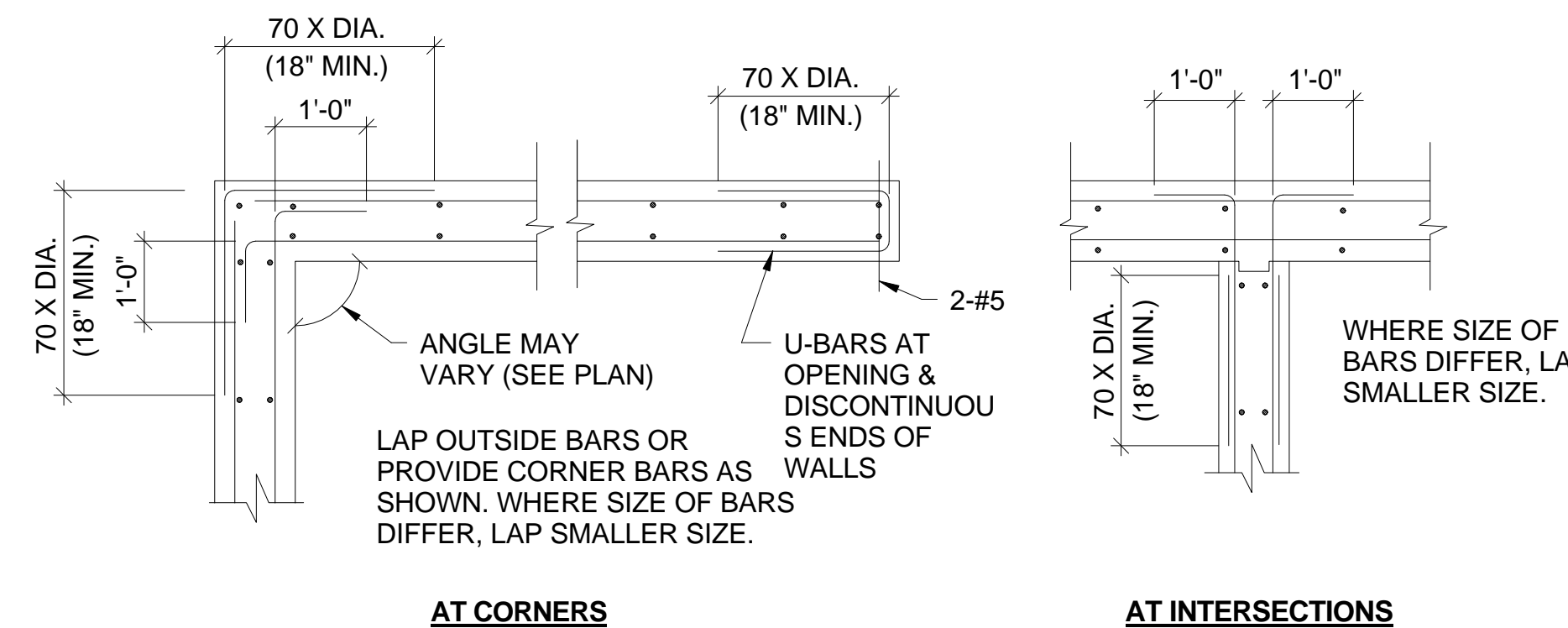
1 Stair Foundation
3/4" = 1'-0"



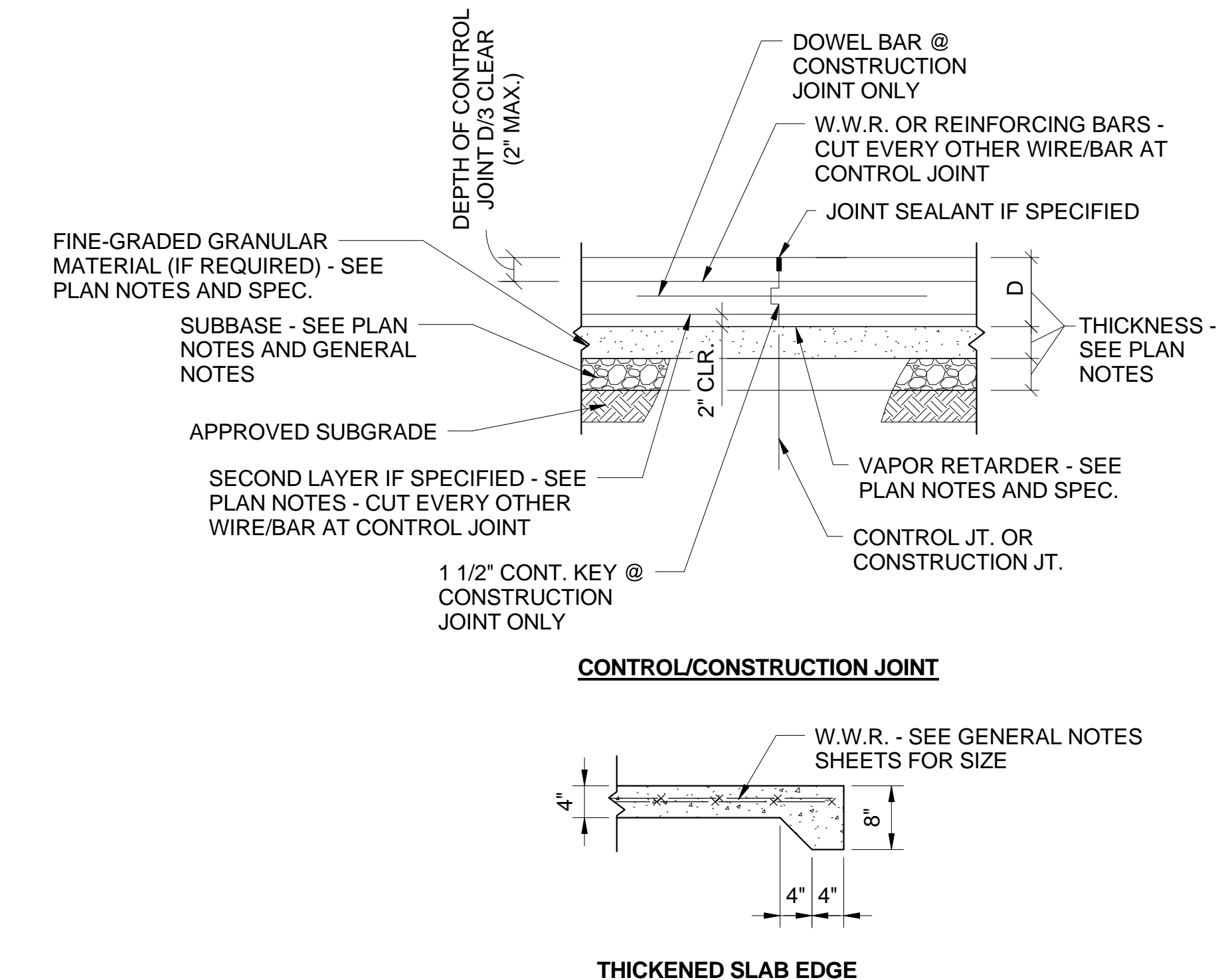
2 Typical Recessed Mat Detail
3/4" = 1'-0"



3 Typical Thickend Slab for CMU Wall
3/4" = 1'-0"

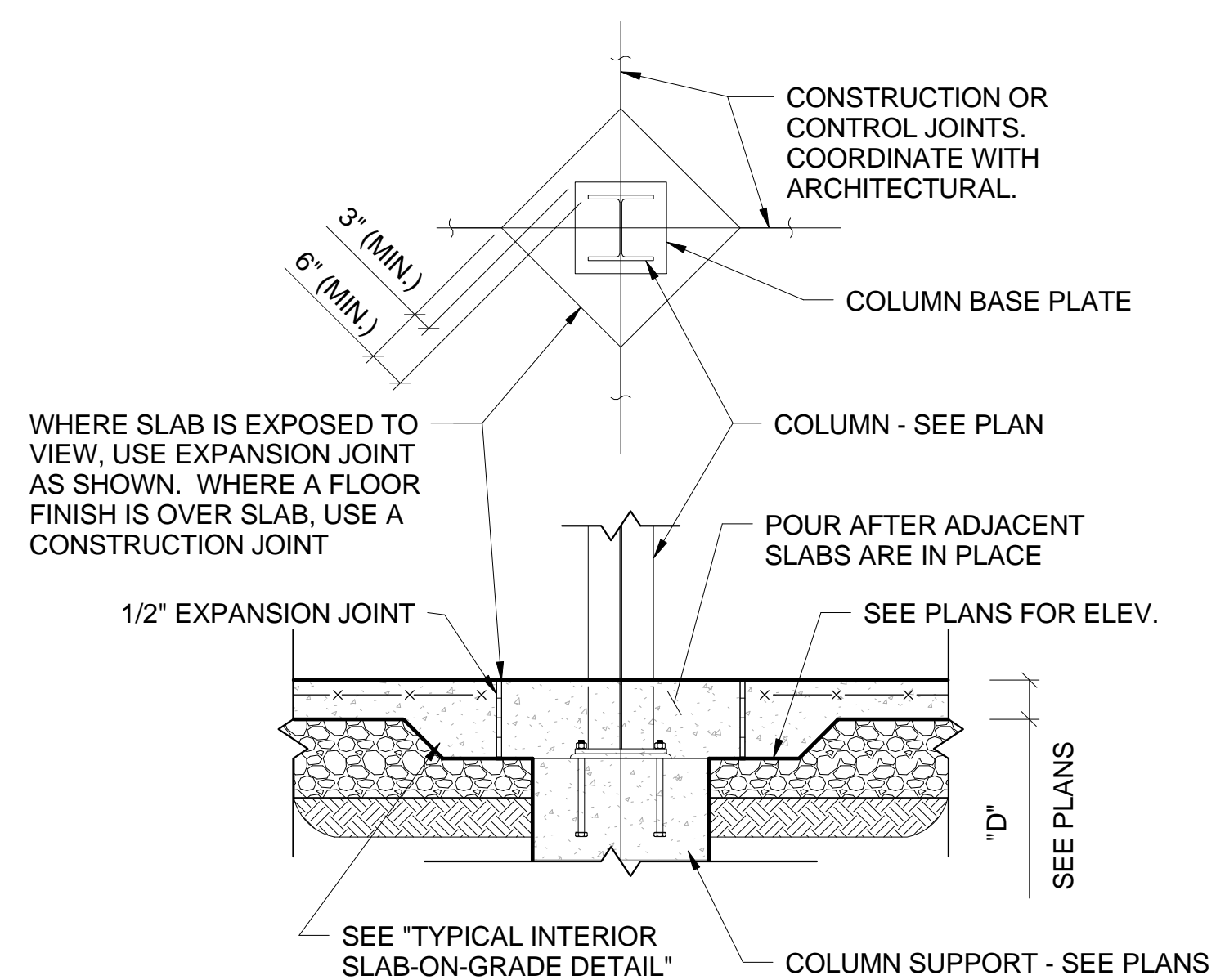


4 Plan of Horizontal Reinforcing at Corners and Intersections of Concrete Walls/Footings
N.T.S.

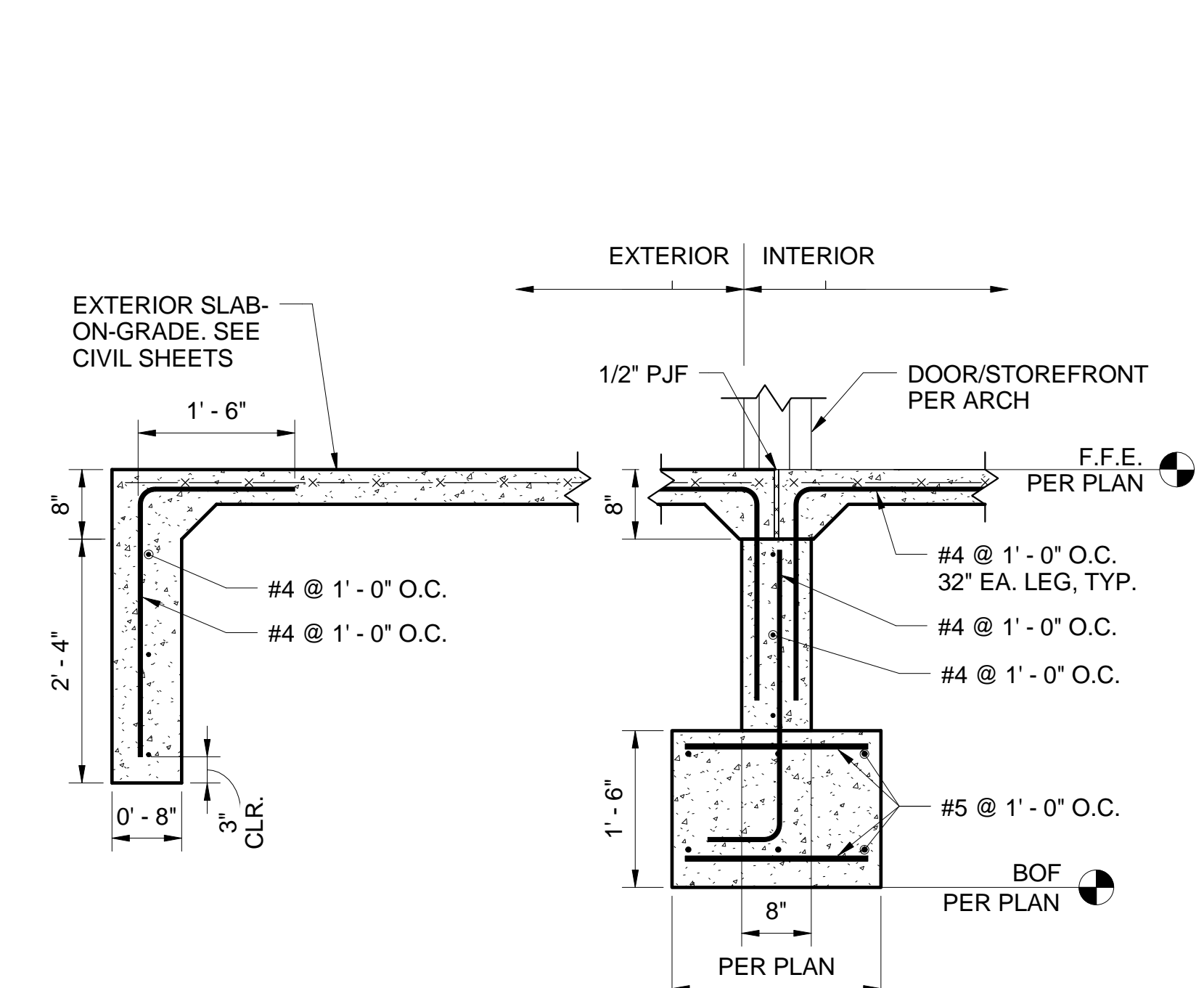


5 Typical Interior Slab-On-Grade Detail
N.T.S.

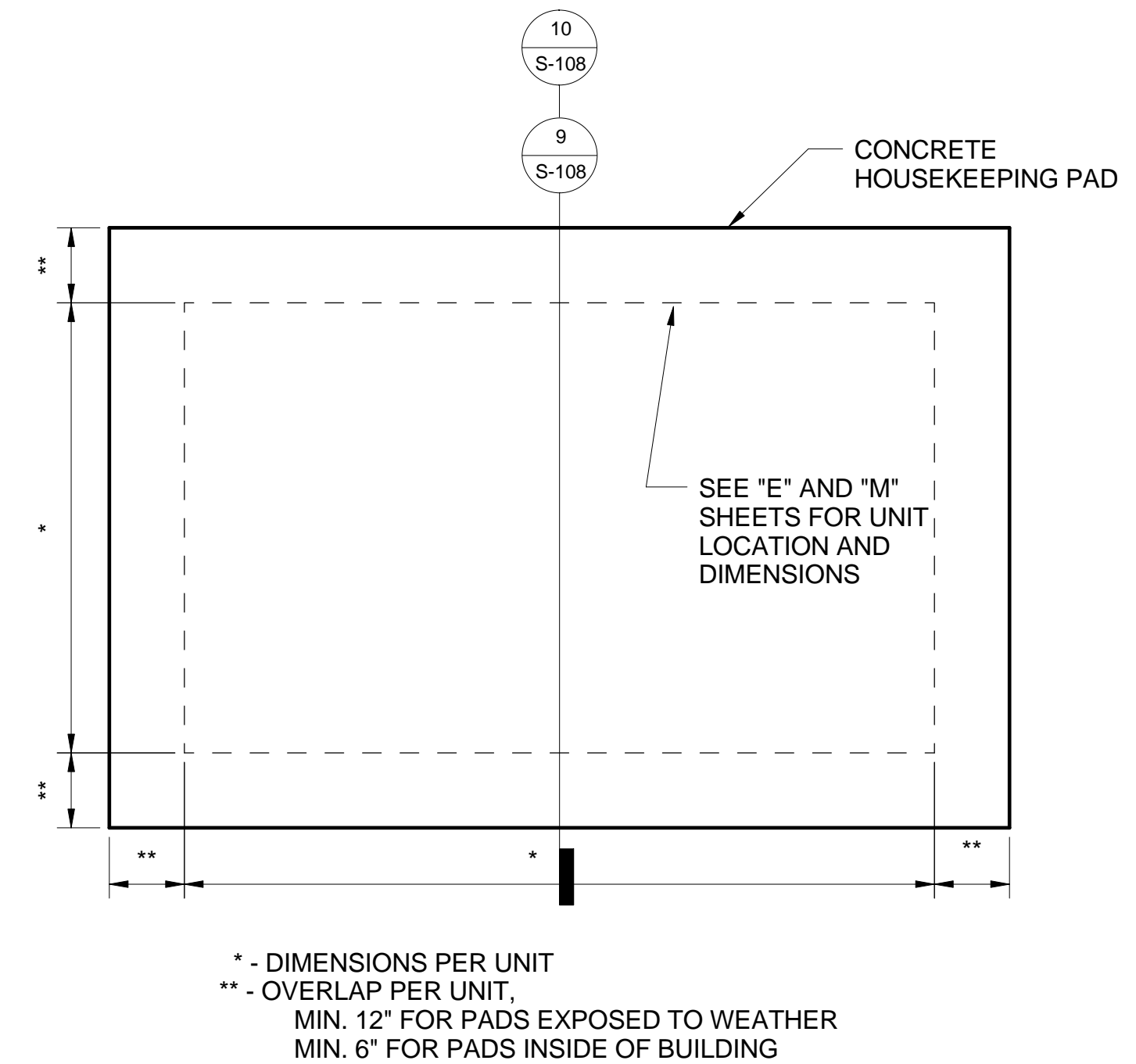
- SLAB-ON-GRADE NOTES:**
1. PROVIDE CONTROL JOINTS IN ALL SLABS-ON-GRADE IN ACCORDANCE WITH THESE NOTES AND DETAILS.
 2. SUBMIT A DETAILED LAYOUT OF THE CONTROL JOINTS FOR ALL EXPOSED TO VIEW CONCRETE SLABS-ON-GRADE FOR REVIEW AND APPROVAL BY ARCHITECT/ENGINEER PRIOR TO CONSTRUCTION.
 3. LOCATE CONTROL JOINTS SO THAT PANELS BETWEEN JOINTS ARE NEARLY SQUARE BUT WITH THE LENGTH NEVER EXCEEDING TWICE THE WIDTH. JOINT SPACING SHALL NOT EXCEED 15'-0". TYPICALLY, 10'-0" AT VINYL TILE FLOORING-SEE ARCH. DRAWINGS.
 4. LOCATE CONTROL JOINTS TO COINCIDE WITH JOINTS IN ARCHITECTURAL FINISHES.
 5. PROVIDE STRIPABLE CONTROL JOINTS WHERE SLABS ARE EXPOSED TO VIEW. PROVIDE SAW CUT OR STRIPABLE CONTROL JOINTS IN OTHER AREAS. INSTALL SAW CUTS AS SOON AS PRACTICAL AND BEFORE THE CUTTING WOULD RESULT IN CHIPPING, SPALLING, OR TEARING BUT IN NO CASE MORE THAN 24 HOURS AFTER PLACEMENT.
 6. DO NOT LOCATE CONTROL/CONSTRUCTION JOINTS BELOW MASONRY WALLS.
 7. PROVIDE DOWELS AT CONSTRUCTION JOINTS. DOWELS SHALL BE 5/8" Φ X 24" LONG A36 SMOOTH BARS WITH ONE END GREASED. SPACING EQUALS 12" O.C.



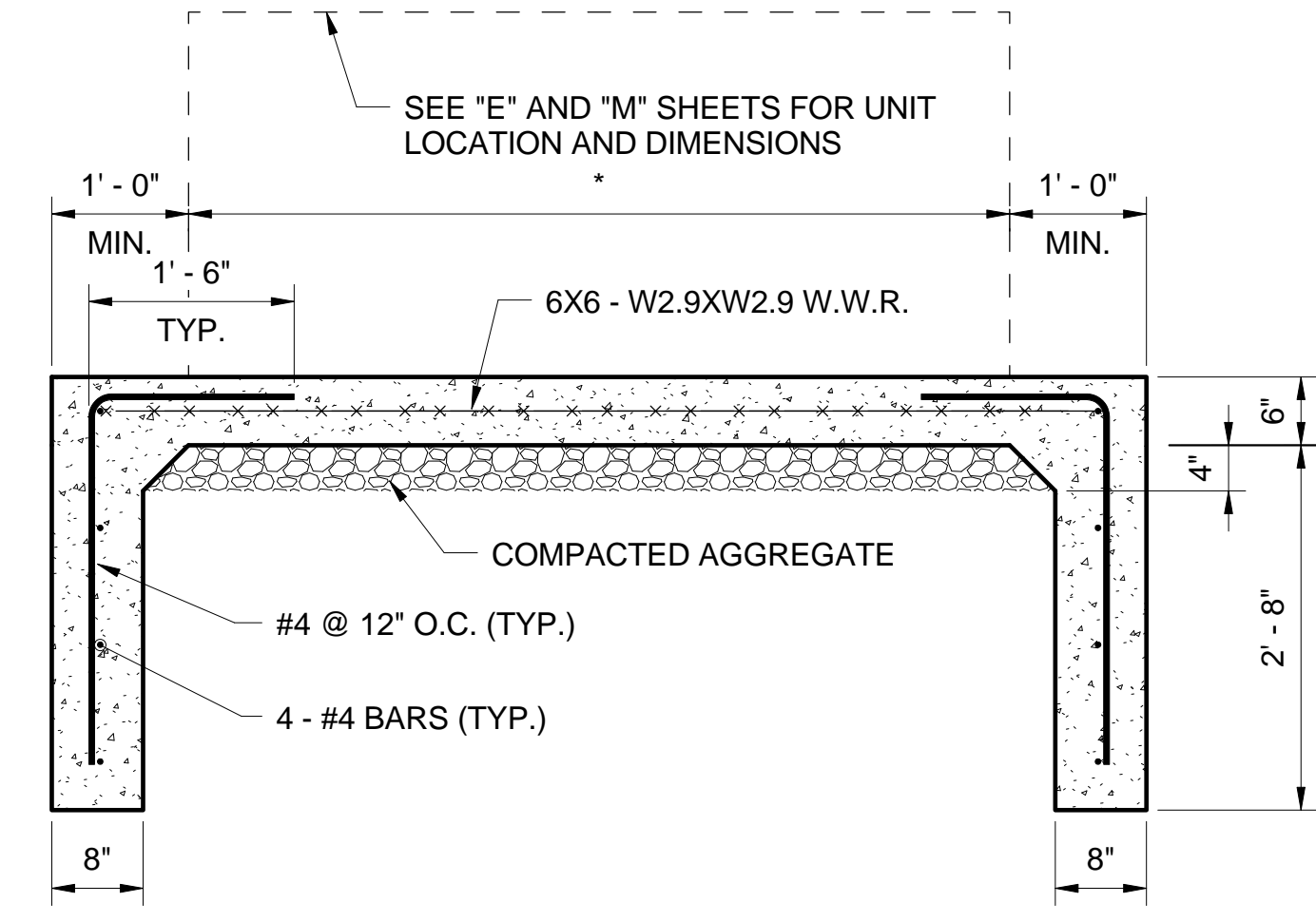
6 Typical Interior Slab-On-Grade Detail at Steel Columns
N.T.S.



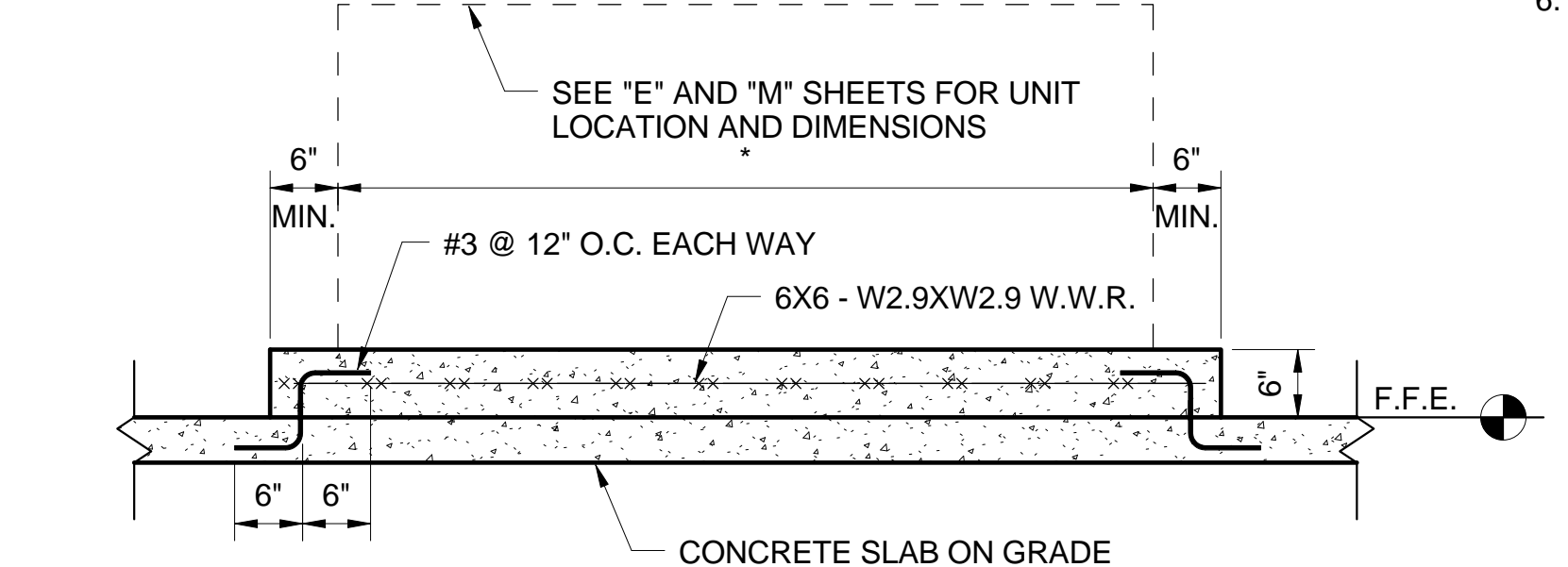
7 Typical Threshold Detail w/ Stoop
3/4" = 1'-0"



8 Typical Housekeeping Pad Plan
N.T.S.

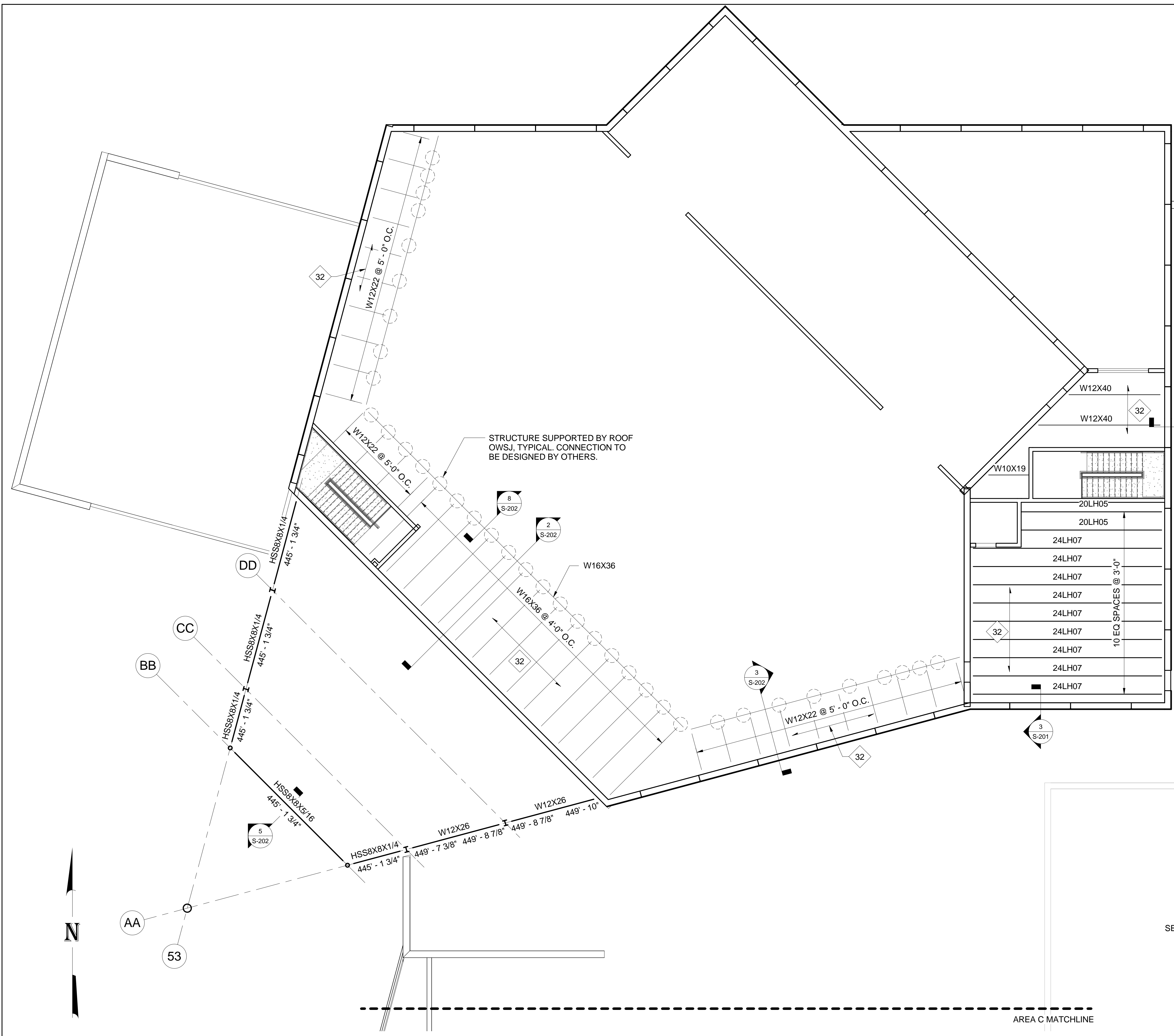


9 Section of Typical Housekeeping Pad Plan (Exposed to Weather)
N.T.S.



10 Section of Typical Housekeeping Pad Plan (Inside of Building)
N.T.S.

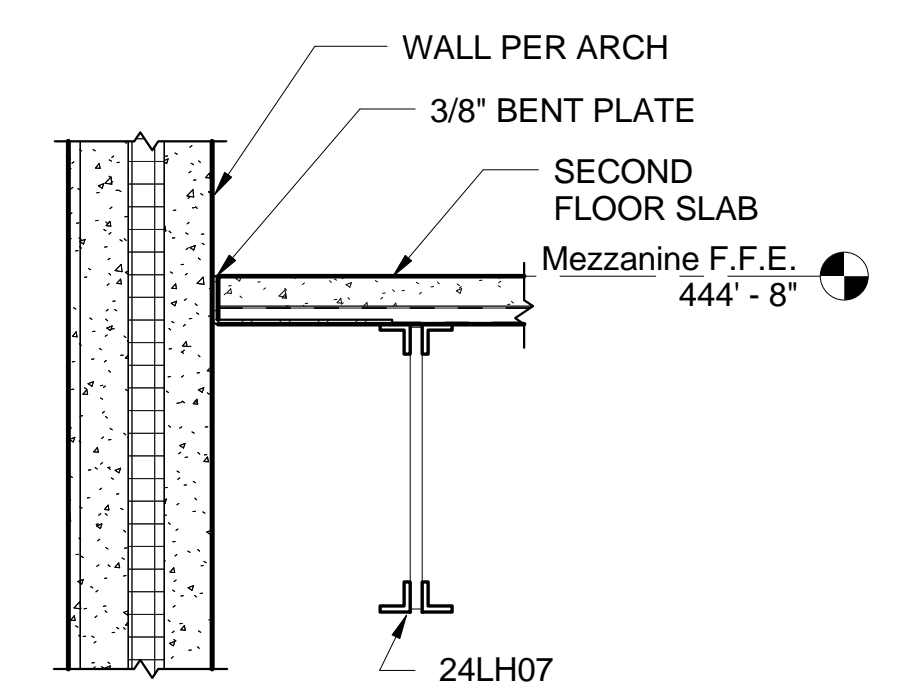
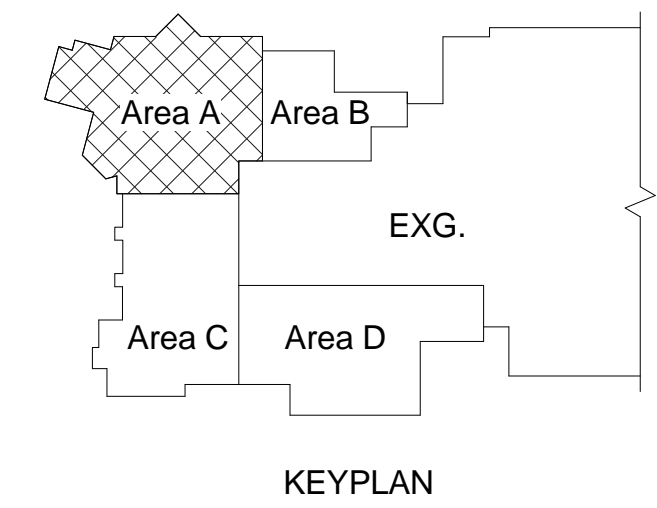
- GENERAL NOTES:**
1. 2" PERIMETER INSULATION SHALL BE PLACED ON THE INSIDE OF ALL EXTERIOR FOUNDATIONS. SEE ARCH SHEETS.
 2. REFER TO SHEET S-001 & S-002 FOR ADDITIONAL INFORMATION.
 3. WALL DETAILS SHOWN ARE FOR STRUCTURAL REPRESENTATION ONLY, SEE ARCH SHEETS FOR SIZE, TEXTURE, COLOR, ETC.
 4. SEE SHEET S-501 FOR COLUMN SCHEDULE.
 5. SEE ARCH & CIVIL FOR WEEP HOLE LOCATIONS AND DETAILS.
 6. FOUNDATION REINFORCEMENT HOOKED BAR DEVELOPMENT LENGTH AS FOLLOWS U.N.O.:
#5 BAR.....1' - 0"
#7 BAR.....1' - 6"



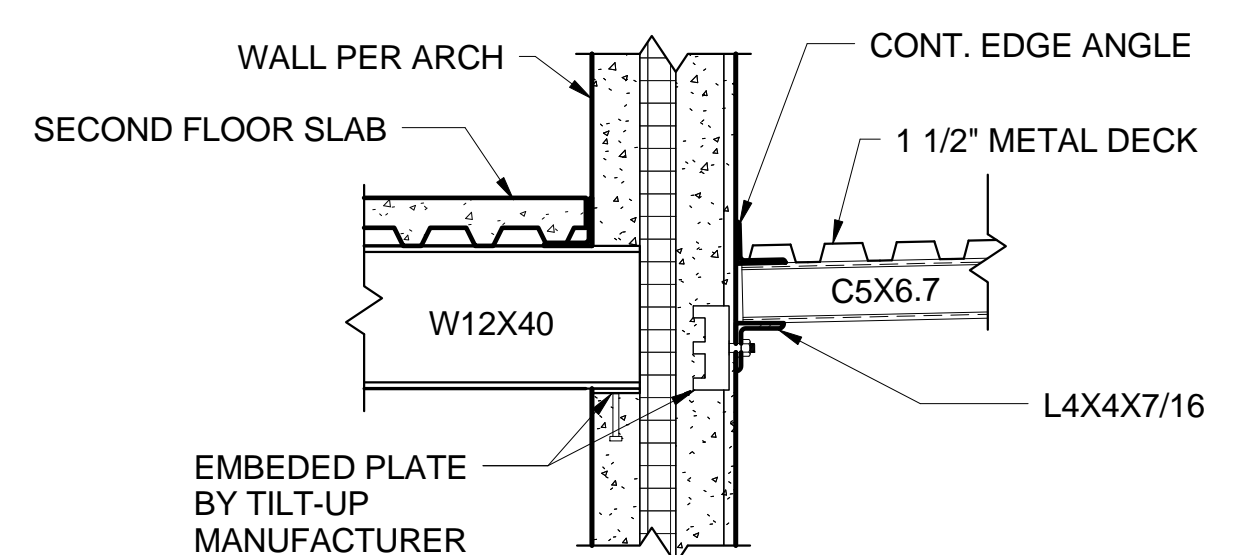
KEYNOTE LEGEND	
KEY VALUE	KEYNOTE TEXT
32	2 1/2" LIGHTWEIGHT CONCRETE OVER 1 1/2" - 18 GAUGE COMPOSITE METAL DECK (TOTAL THICKNESS = 4") - REINFORCED WITH 6X6 - W1.4X1.4

GENERAL NOTES:

- REFER TO SHEET S-001 & S-002 FOR ADDITIONAL INFORMATION.
- SPACE BETWEEN OWSJ AND ADJACENT BEAMWALL SHALL NOT EXCEED THE OWSJ SPACING.
- SPACE OWSJ TO AVOID INTERFERING WITH PARTITION WALLS.
- ELEVATIONS SHOWN WITH STRUCTURAL STEEL ARE TOP OF STEEL ELEVATIONS U.N.O.
- SEE SHEET S-401 FOR LINTEL SCHEDULE.
- SEE SHEET S-501 FOR COLUMN SCHEDULE.
- SEE ARCH/MECH SHEETS FOR LOCATIONS AND DIMENSIONS OF MECH UNITS/DUCTS
- SEE S-502 FOR FULL COLUMN GRID MARKS.
- TOP OF SLAB = 444'-8"
- ELEVATIONS SHOWN WITH STRUCTURAL STEEL ARE TOP OF STEEL ELEVATIONS U.N.O.



3 2nd Floor Section
3/4" = 1'-0"



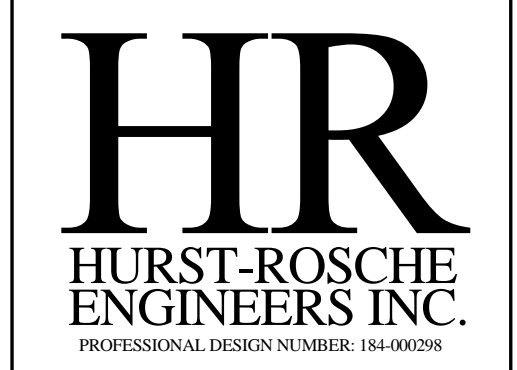
2 Second Floor Framing Section
3/4" = 1'-0"

1 2nd Floor Framing - Area A
1/8" = 1'-0"

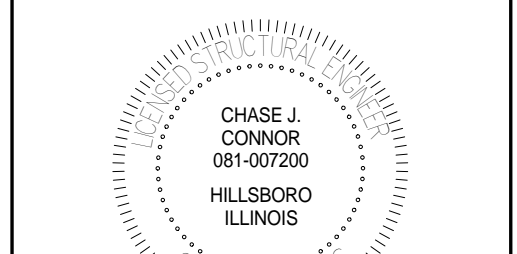


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NEOSHO, MO



ROSS CONSTRUCTION INC.
504 W. Jackson Marion, IL 62959
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SIGNATURE: *Chase Connor*
DATE: 03/19/15
DATE: 11-30-2016
LICENSE EXPIRES

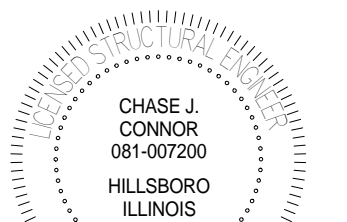
PHASE 6 - FINE ARTS BUILDING ADDITION
NEW MARION HIGH SCHOOL
MARION C.U.S.D. #2
MARION, WILLIAMSON COUNTY, ILLINOIS

Mark	Date	Description

DATE: 03/19/15
PROJECT NO: 360-2632
DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

2ND FLOOR FRAMING PLAN - AREA 'A'

S-201



Chase Connor
SIGNATURE

03/19/15

DATE

11-30-2016

LICENSE EXPIRES

PHASE 6 - FINE ARTS BUILDING ADDITION
NEW MARION HIGH SCHOOL
MARION C.U.S.D. #2
MARION, WILLIAMSON COUNTY, ILLINOIS

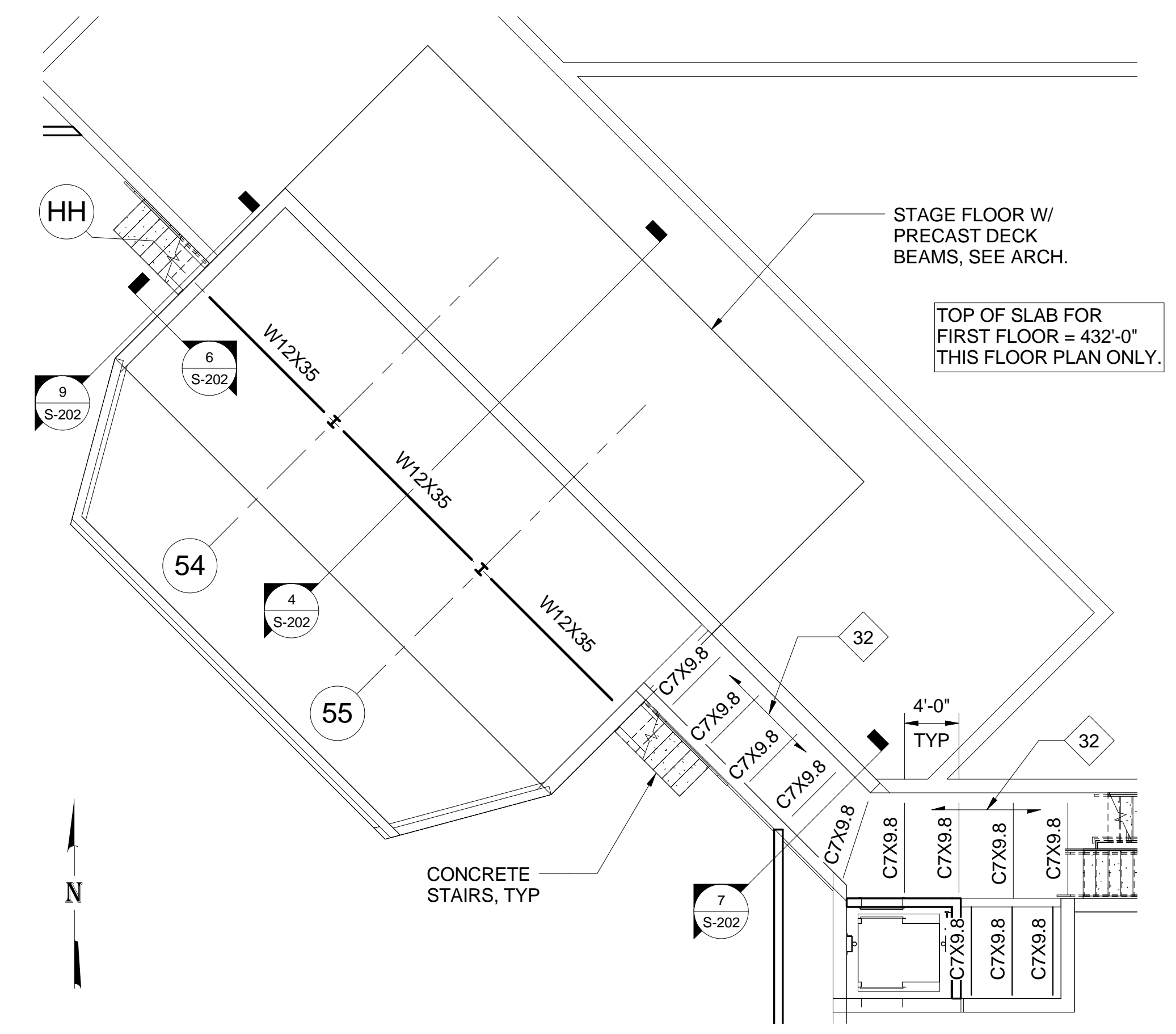
Mark	Date	Description

DATE: 03/19/15
PROJECT NO: 360-2632
DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

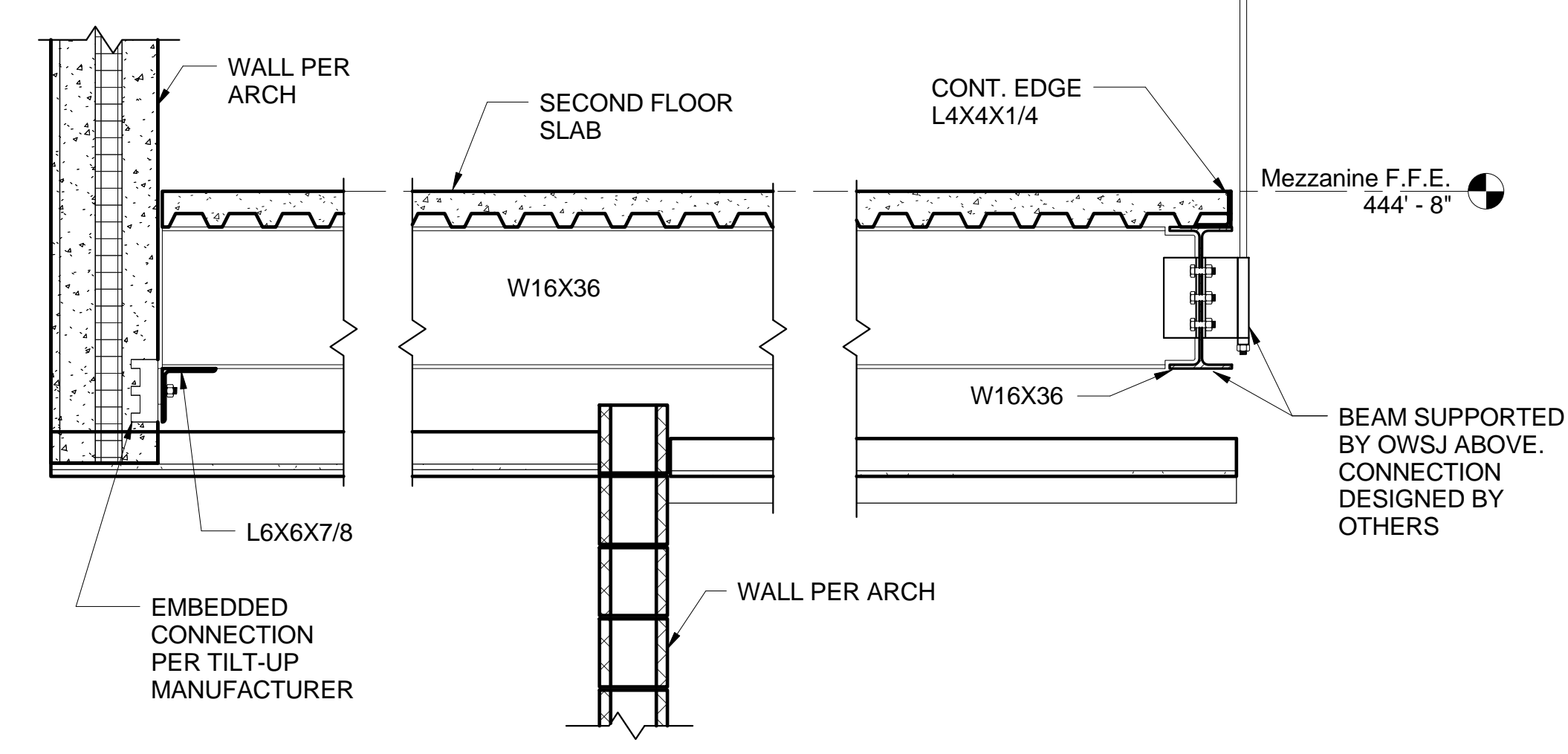
2ND FLOOR FRAMING
DETAILS - AREA 'A'

GENERAL NOTES:

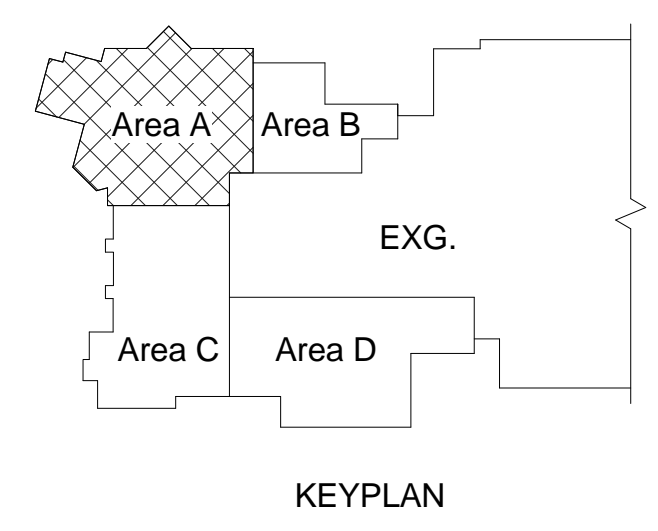
1. REFER TO SHEET S-001 & S-002 FOR ADDITIONAL INFORMATION.
2. SPACE BETWEEN OWSJ AND ADJACENT BEAM/WALL SHALL NOT EXCEED THE OWSJ SPACING.
3. SPACE OWSJ TO AVOID INTERFERING WITH PARTITION WALLS.
4. SPACING OF OWSJ IS MAXIMUM AS INDICATED. ADDITIONAL OWSJ MAY BE REQUIRED.
5. ALL BEAMS ARE SLOPING UNLESS NOTED OTHERWISE.
6. SEE SHEET S-401 FOR LINTEL SCHEDULE.
7. SEE SHEET S-501 FOR COLUMN SCHEDULE.
8. SEE ARCH/MECH SHEETS FOR LOCATIONS AND DIMENSIONS OF MECH UNITS/DUCTS
9. SEE S-502 FOR FULL COLUMN GRID MARKS.



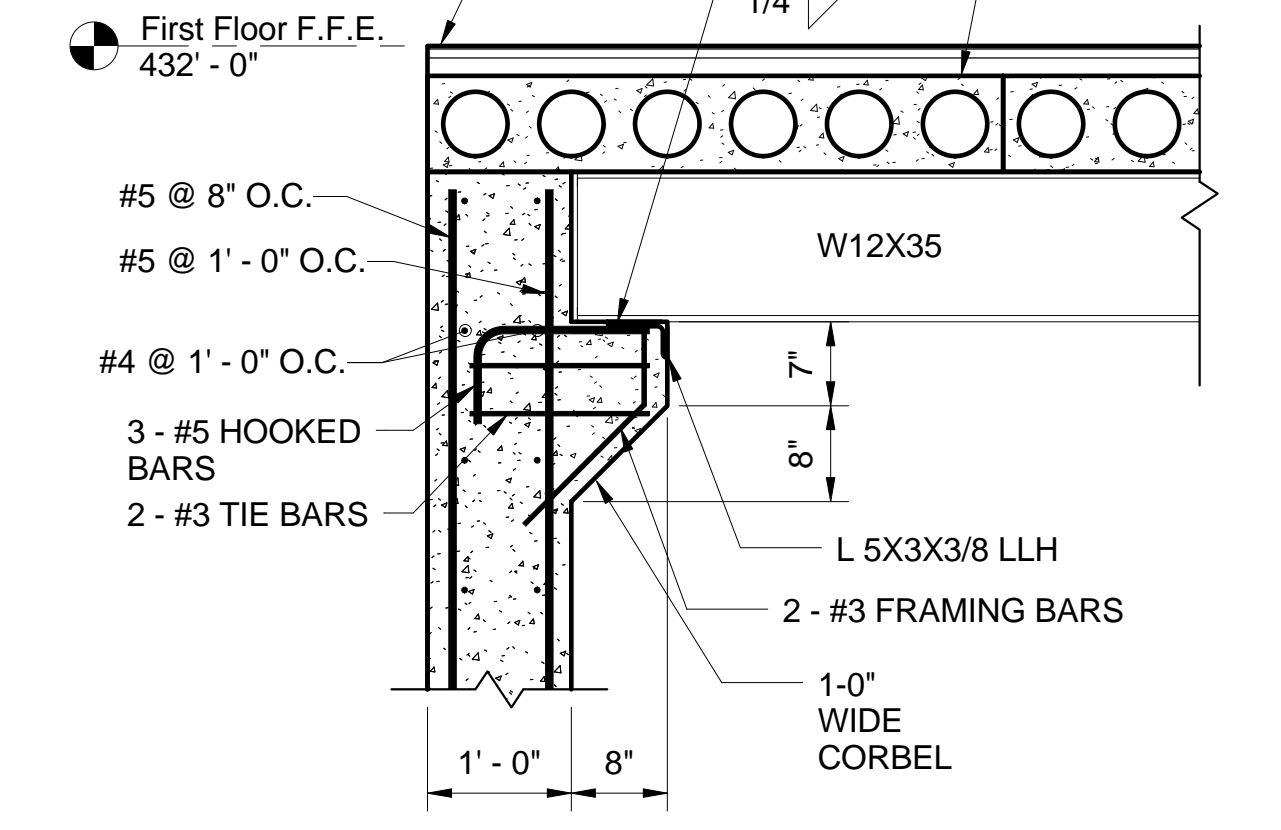
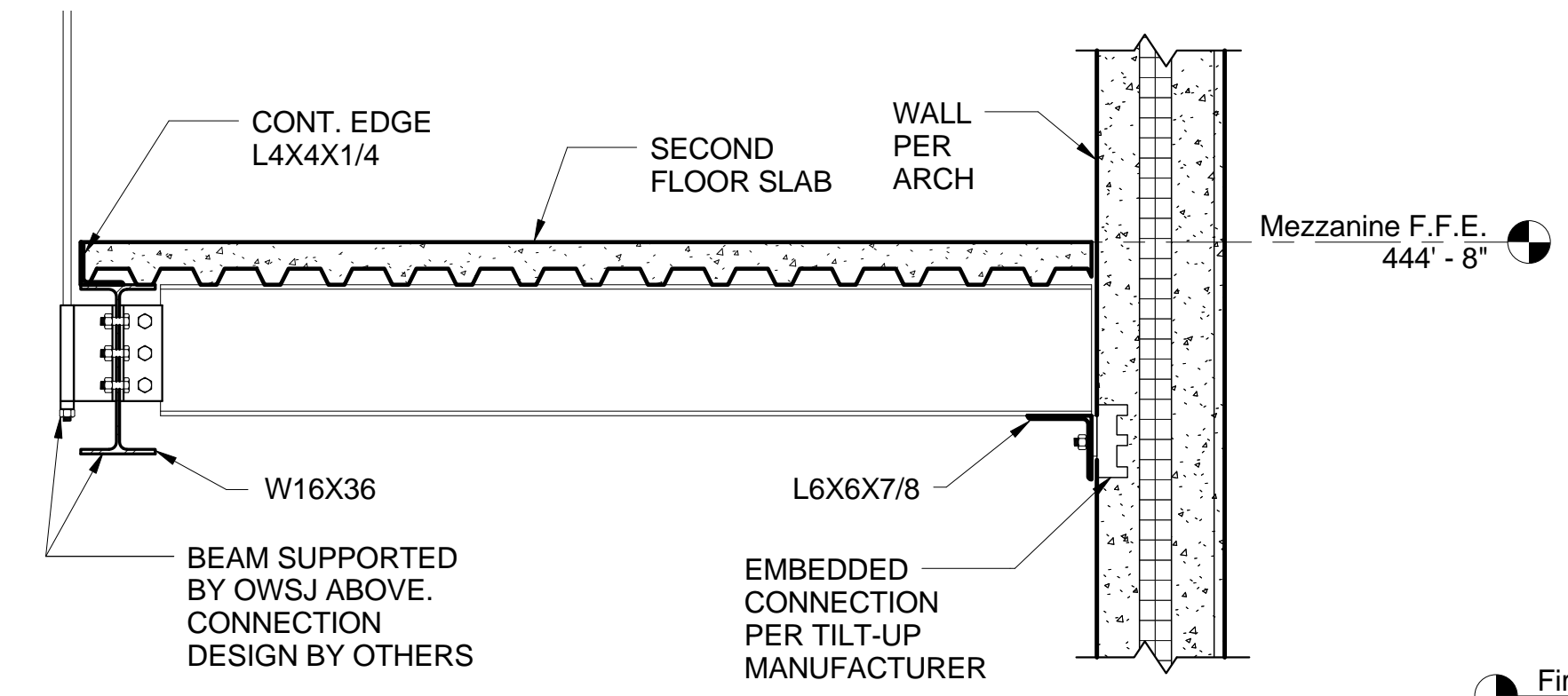
1 Stage and 1st Floor Framing
1/8" = 1'-0"



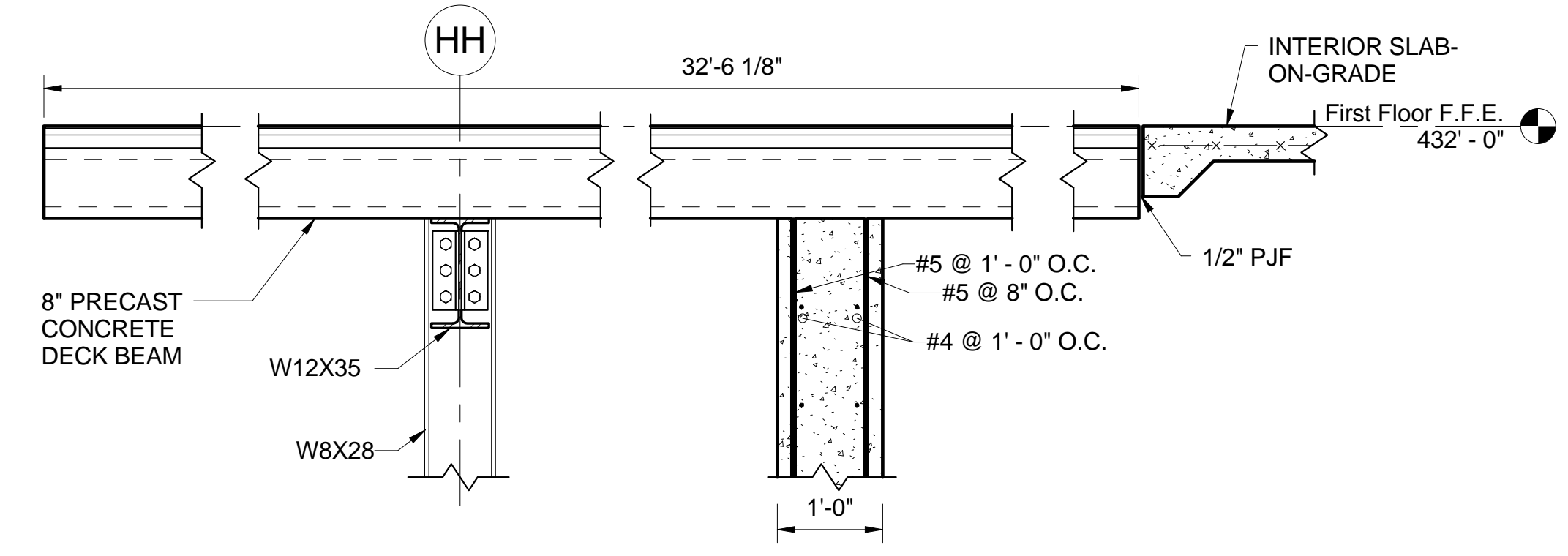
2 2nd Floor Section
3/4" = 1'-0"



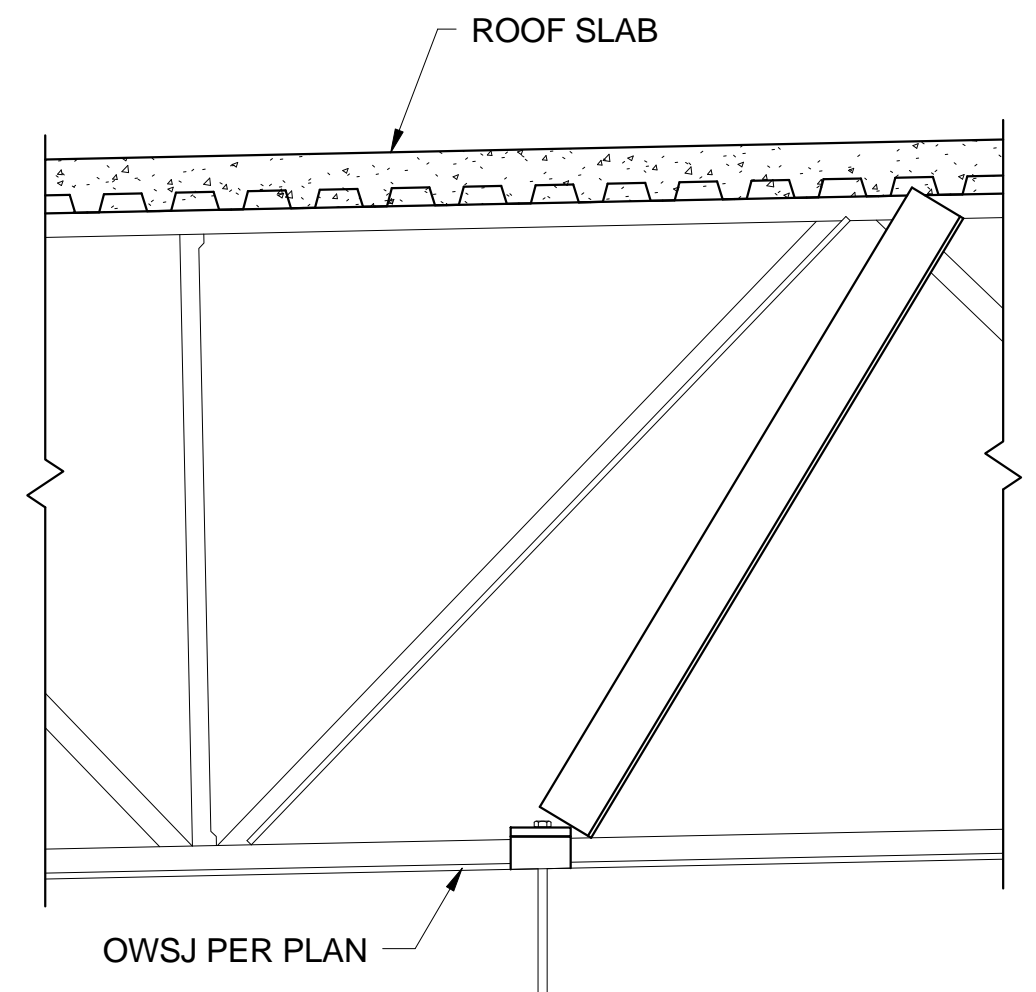
3 2nd Floor Section
3/4" = 1'-0"



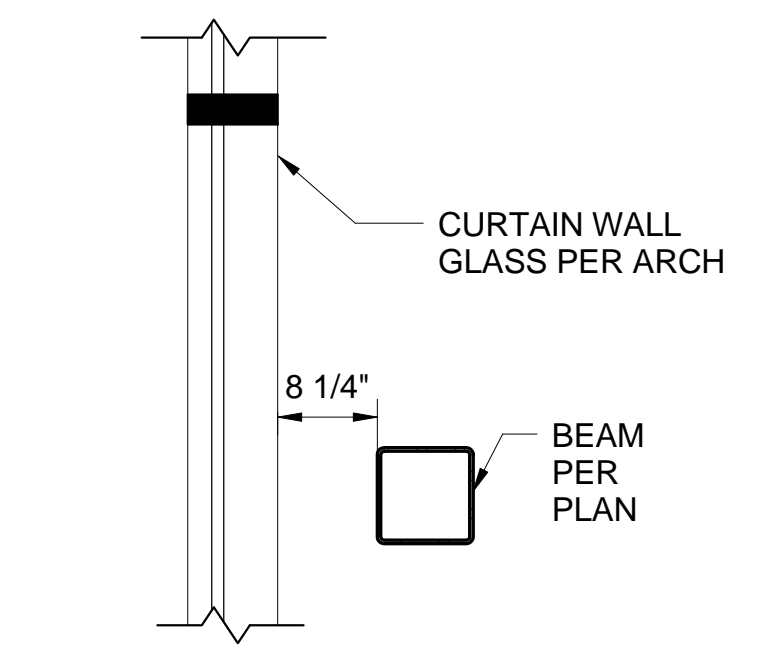
6 1st Floor Section
3/4" = 1'-0"



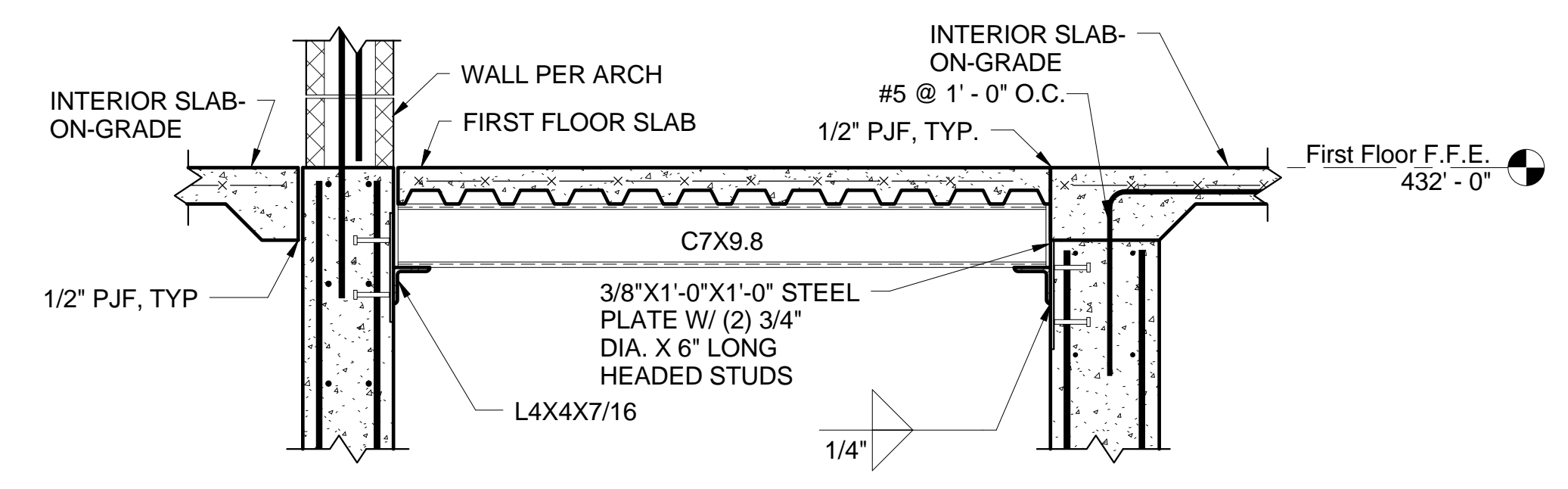
4 1st Floor Section
3/4" = 1'-0"



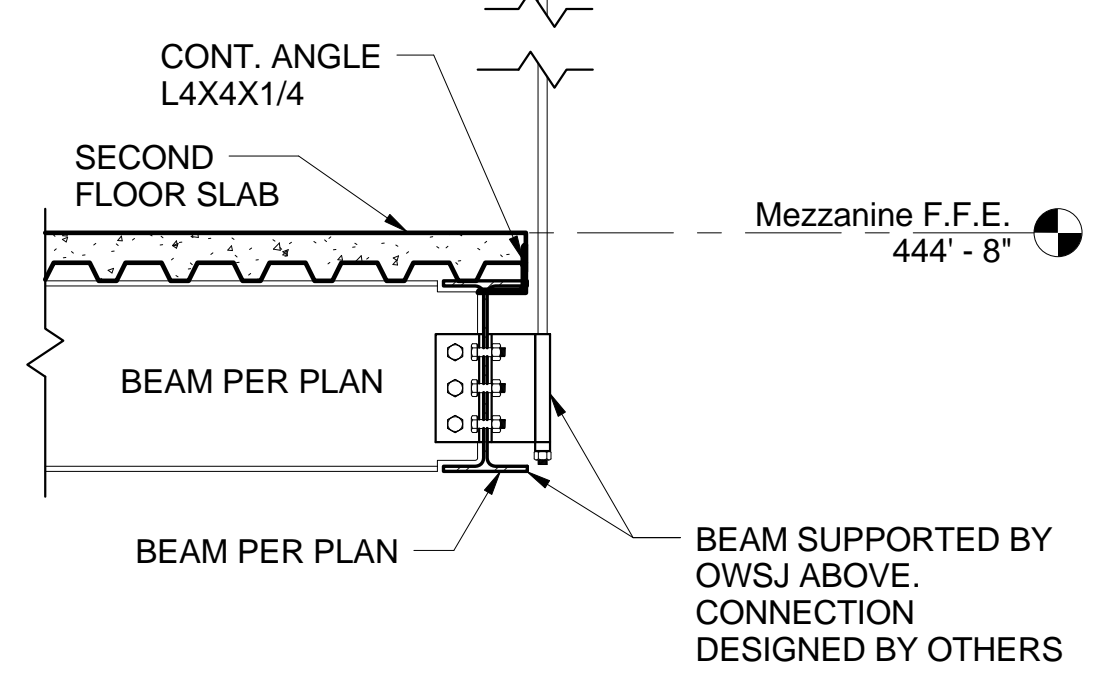
OWSJ PER PLAN



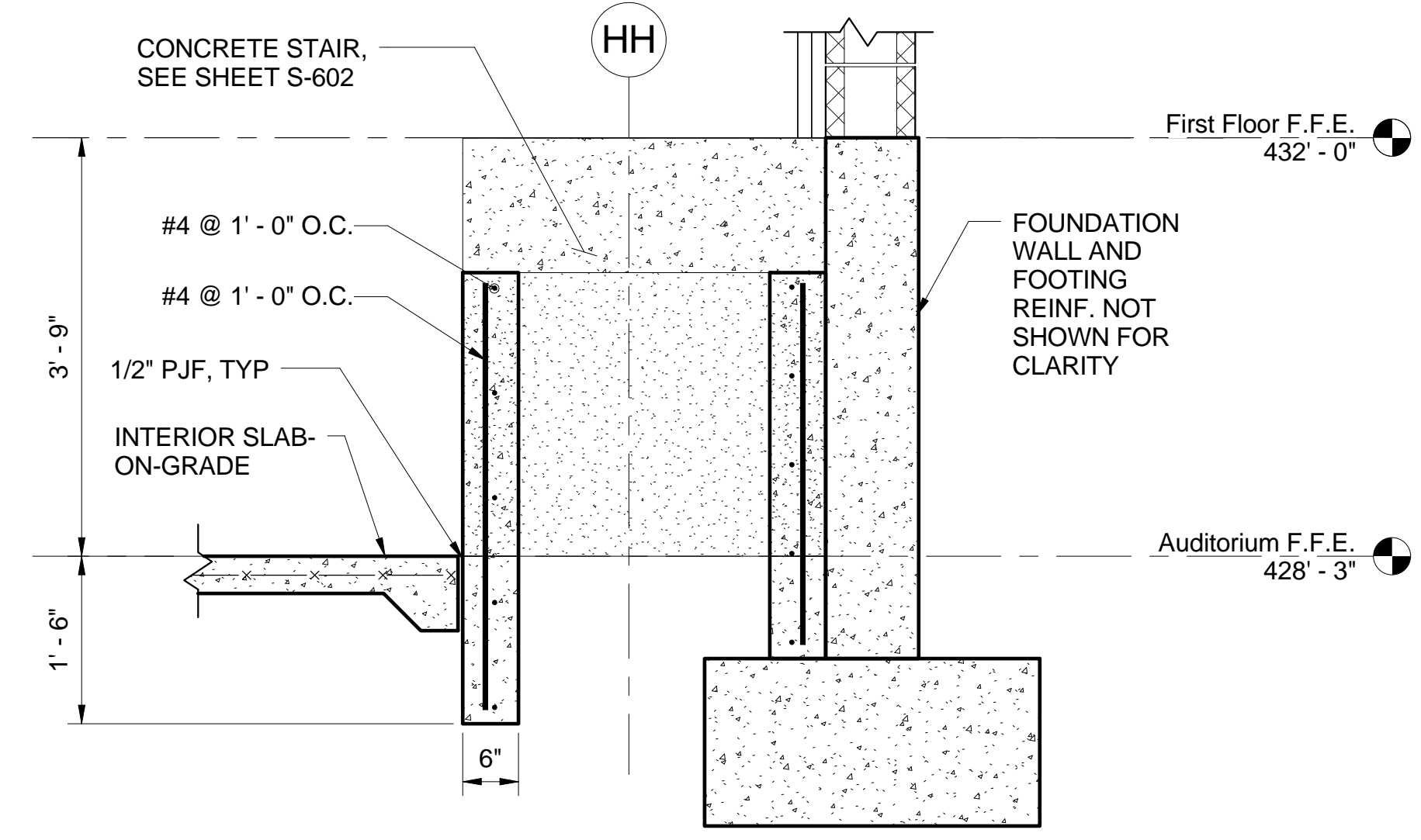
5 Curtain Wall Section
3/4" = 1'-0"



7 1st Floor Section
3/4" = 1'-0"



8 2nd Floor Section
3/4" = 1'-0"



9 1st Floor Section
3/4" = 1'-0"

KEYNOTE LEGEND	
KEY VALUE	KEYNOTE TEXT
1	K SERIES JOIST SHALL HAVE A SEAT DEPTH = 5"
30	2 1/2" LIGHTWEIGHT CONCRETE OVER 1 1/2" - 20 GAUGE COMPOSITE METAL DECK (TOTAL THICKNESS = 4") - REINFORCED WITH 6X6 - W1.4X1.4
31	1 1/2" - 18 GAUGE METAL ROOF DECK

- GENERAL NOTES:**
- REFER TO SHEET S-001 & S-002 FOR ADDITIONAL INFORMATION.
 - SPACE BETWEEN OWSJ AND ADJACENT BEAM/WALL SHALL NOT EXCEED THE OWSJ SPACING.
 - SPACE OWSJ TO AVOID INTERFERING WITH PARTITION WALLS.
 - SPACING OF OWSJ IS MAXIMUM AS INDICATED. ADDITIONAL OWSJ MAY BE REQUIRED.
 - ALL BEAMS ARE SLOPING UNLESS NOTED OTHERWISE.
 - SEE SHEET S-401 FOR LINTEL SCHEDULE.
 - SEE SHEET S-501 FOR COLUMN SCHEDULE.
 - SEE ARCH/MECH SHEETS FOR LOCATIONS AND DIMENSIONS OF MECH UNITS/DUCTS
 - SEE S-502 FOR FULL COLUMN GRID MARKS.
 - SEE SHEET S-302 FOR CLARIFICATION OF STEEL ELEVATIONS ALONG GRIDS AA & 53.
 - ELEVATIONS SHOWN WITH STRUCTURAL STEEL ARE TOP OF STEEL ELEVATIONS UNLESS NOTED OTHERWISE.

DAI
DESIGN ARCHITECTS, INC.
PROFESSIONAL DESIGN NUMBER: 184-000469

200 N. MARKET STREET
MARION, IL
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SPRINGFIELD, IL
ARNOLD, MO
NEOSHO, MO

HR
HURST-ROSCH
ENGINEERS INC.
PROFESSIONAL DESIGN NUMBER: 184-000258

ROSS
CONSTRUCTION INC.
504 W. Jackson Marion, IL 62959
p. 618.993.5904
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SIGNATURE: *Chase Connor*
DATE: 03/19/15
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LICENSE EXPIRES

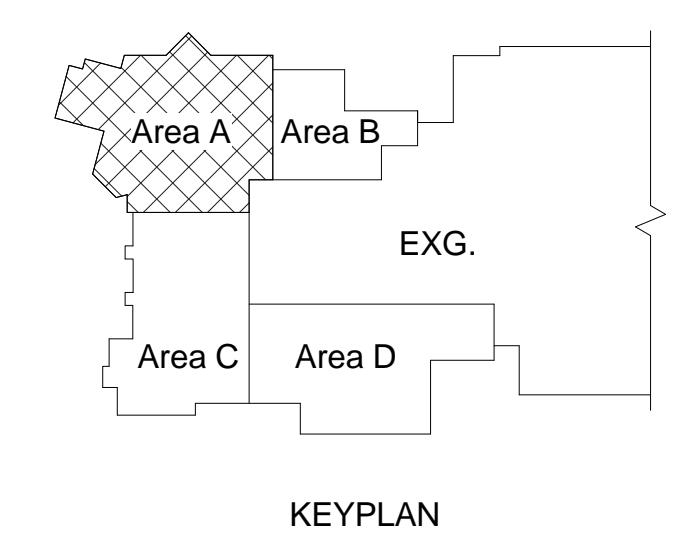
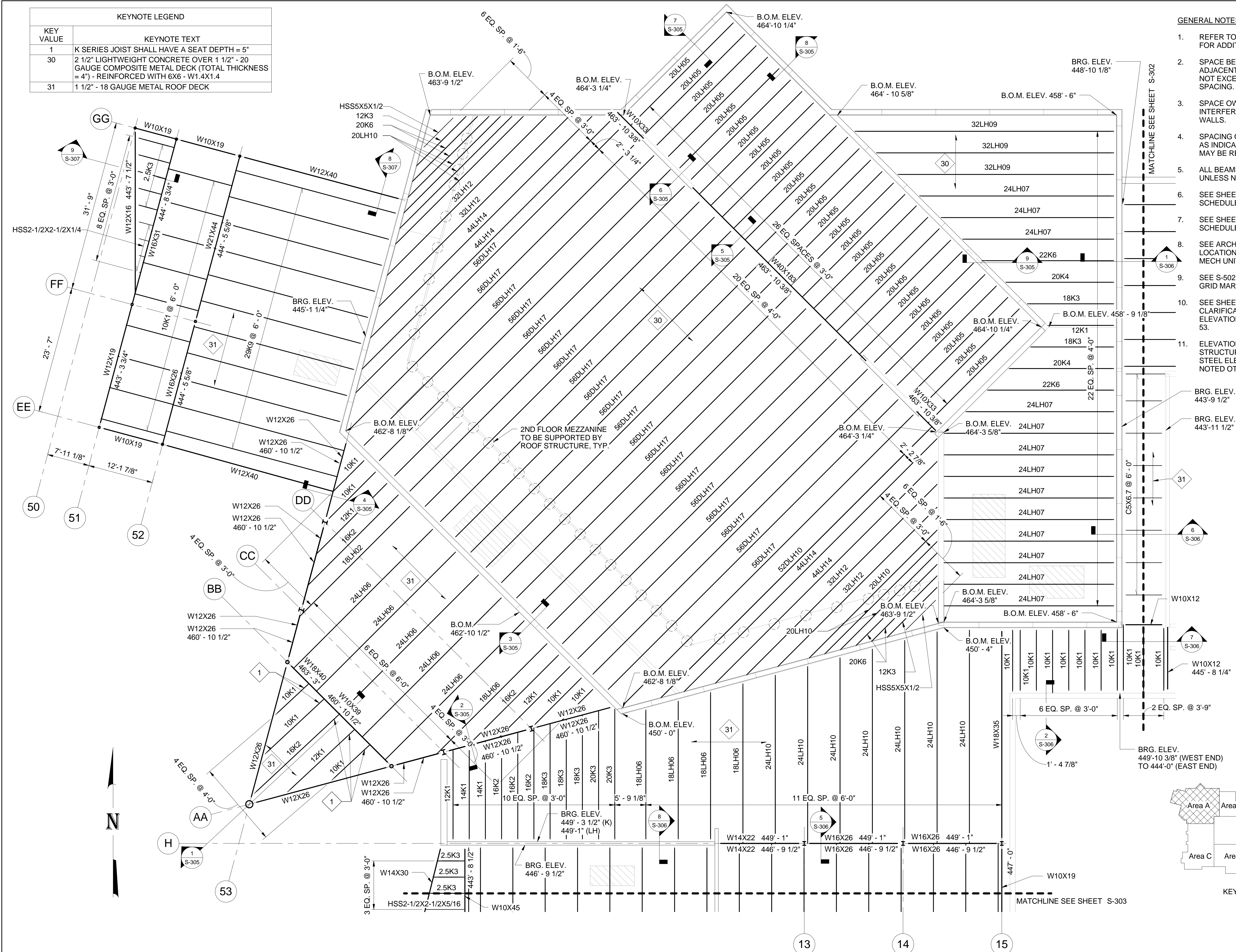
PHASE 6 - FINE ARTS BUILDING ADDITION
NEW MARION HIGH SCHOOL
MARION C.U.S.D. #2
MARION, WILLIAMSON COUNTY, ILLINOIS

Mark	Date	Description

DATE: 03/19/15
PROJECT NO: 360-2632
DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

ROOF FRAMING PLAN
- AREA 'A'

S-301



1 Roof Framing - Area A
1/8" = 1'-0"

KEYNOTE LEGEND	
KEY VALUE	KEYNOTE TEXT
31	1 1/2" - 18 GAUGE METAL ROOF DECK

GENERAL NOTES:

- REFER TO SHEET S-001 & S-002 FOR ADDITIONAL INFORMATION.
- SPACE BETWEEN OWSJ AND ADJACENT BEAM/WALL SHALL NOT EXCEED THE OWSJ SPACING.
- SPACE OWSJ TO AVOID INTERFERING WITH PARTITION WALLS.
- SPACING OF OWSJ IS MAXIMUM AS INDICATED. ADDITIONAL OWSJ MAY BE REQUIRED.
- ALL BEAMS ARE SLOPING UNLESS NOTED OTHERWISE.
- SEE SHEET S-401 FOR LINTEL SCHEDULE.
- SEE SHEET S-501 FOR COLUMN SCHEDULE.
- SEE ARCH/MECH SHEETS FOR LOCATIONS AND DIMENSIONS OF MECH UNITS/DUCTS
- SEE S-502 FOR FULL COLUMN GRID MARKS.
- ELEVATIONS SHOWN WITH STRUCTURAL STEEL ARE TOP OF STEEL ELEVATIONS UNLESS NOTED OTHERWISE.

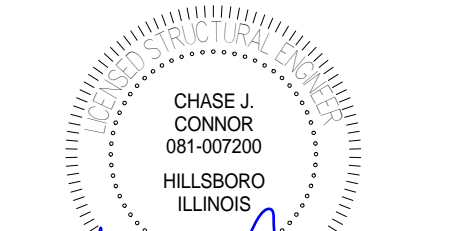


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Chase Connor
SIGNATURE

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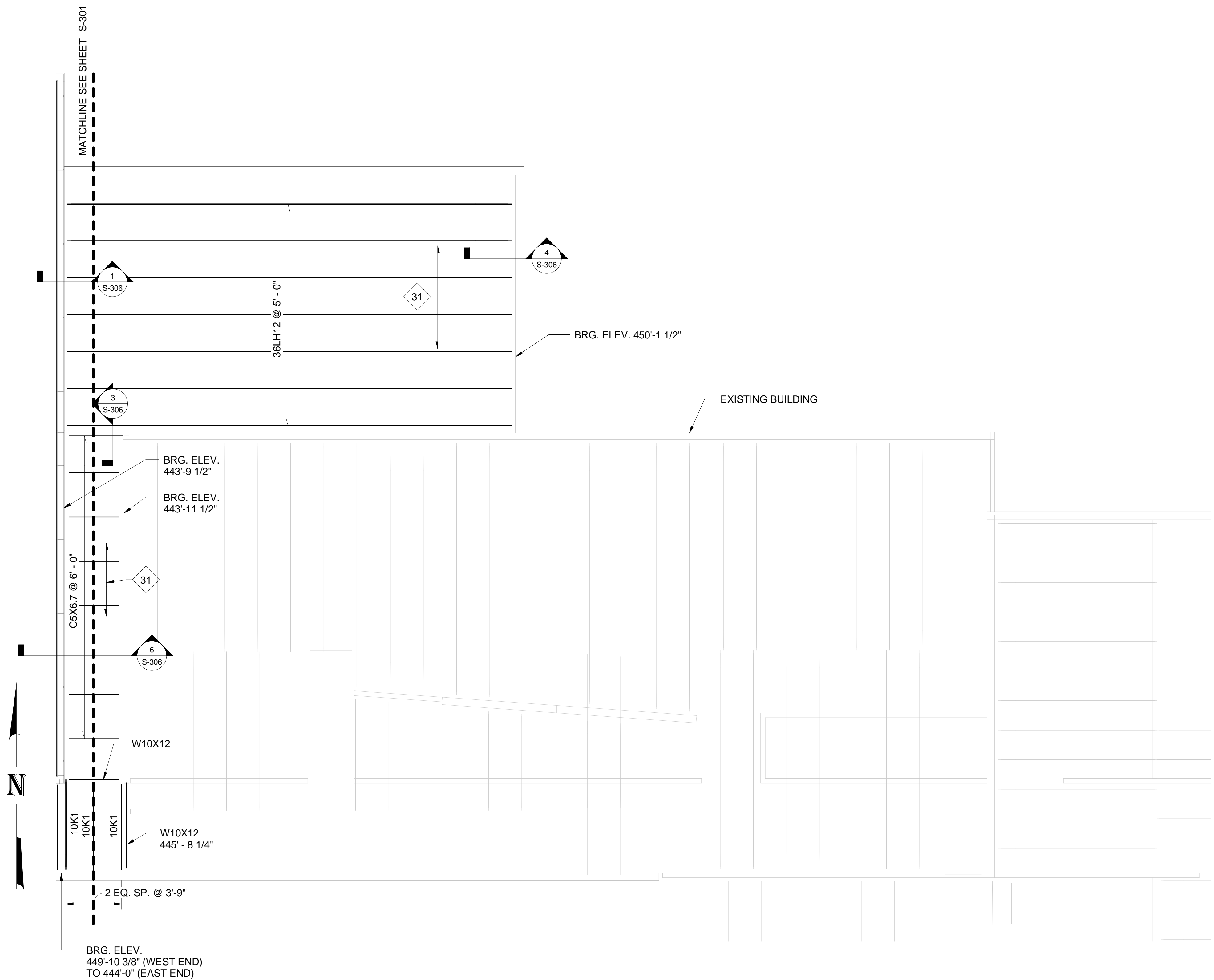
PHASE 6 - FINE ARTS BUILDING ADDITION
NEW MARION HIGH SCHOOL
MARION C.U.S.D. #2
MARION, WILLIAMSON COUNTY, ILLINOIS

Mark	Date	Description

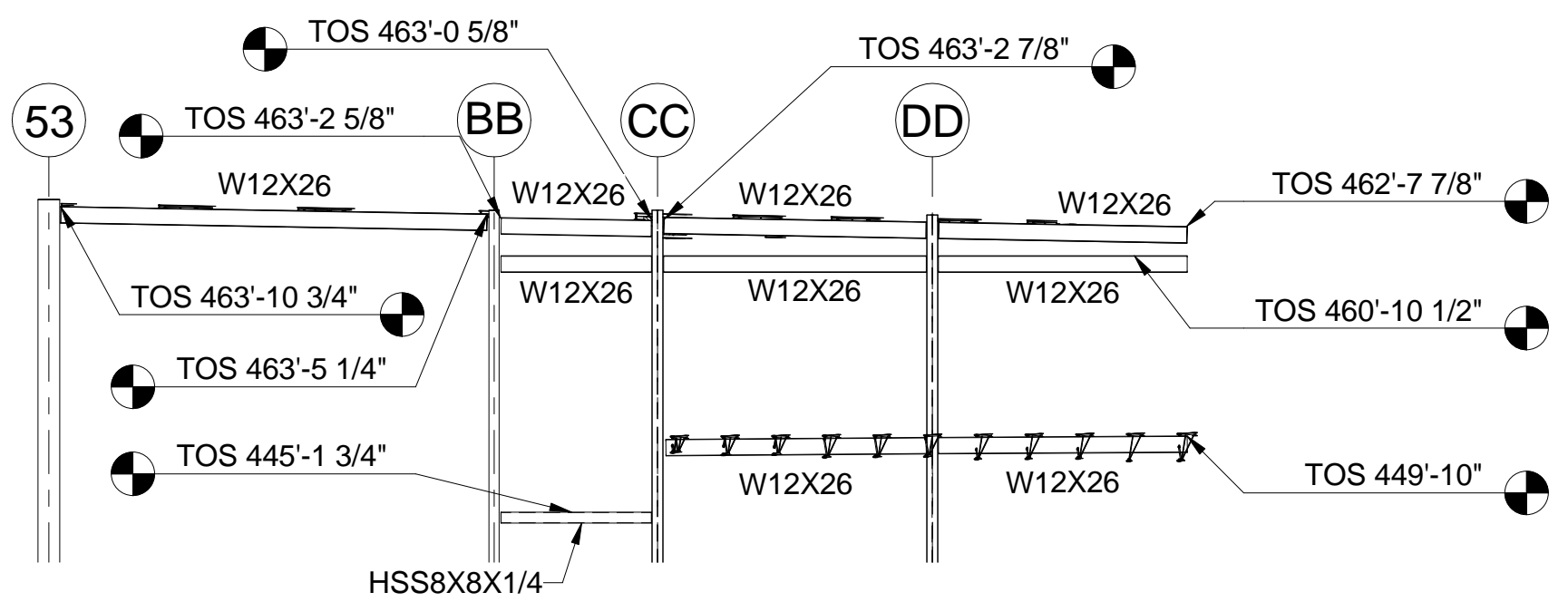
DATE: 03/19/15
PROJECT NO: 360-2632
DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

ROOF FRAMING PLAN - AREA 'B'

S-302

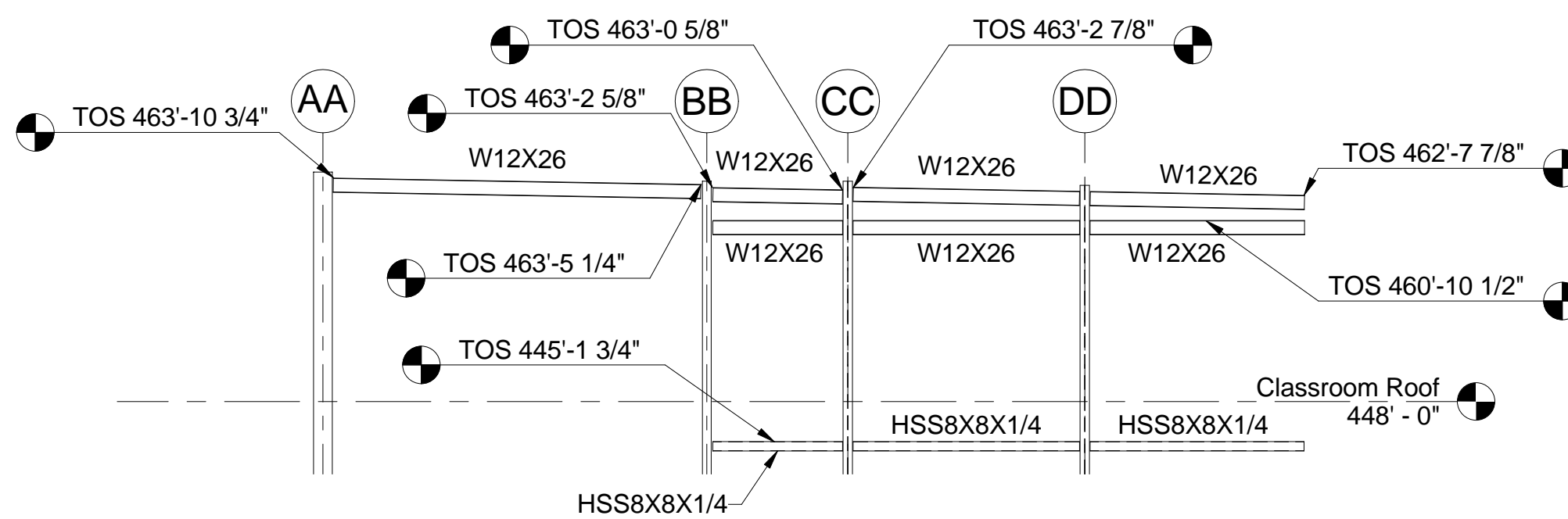


1 Roof Framing - Area B
1/8" = 1'-0"

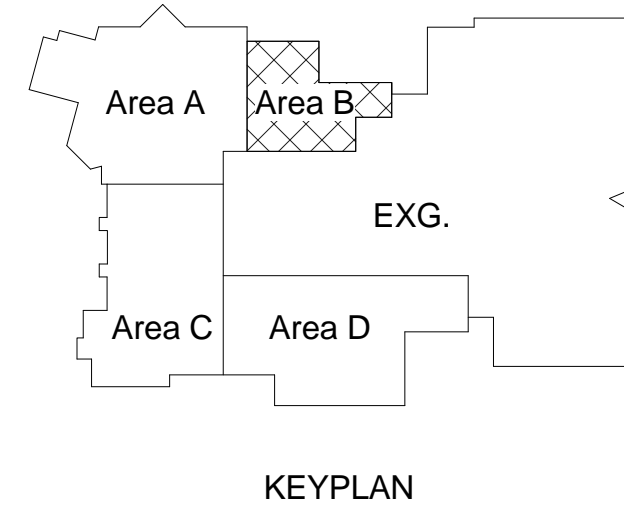


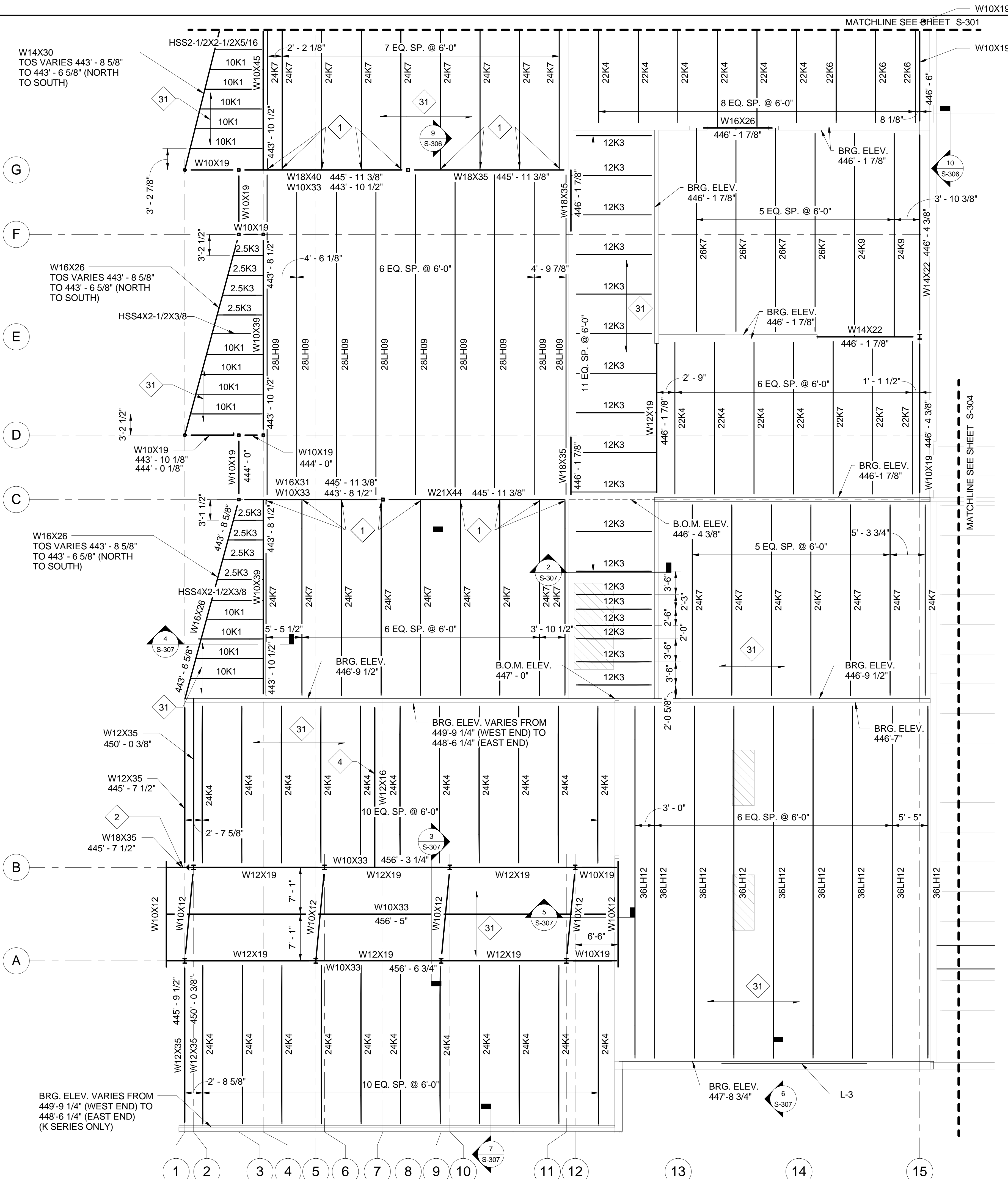
2 Column Grid Line AA (Facing Northwest)
3/32" = 1'-0"

DETAILS 2 & 3/S-302 PERTAIN TO AREA 'A' FRAMING. SEE SHEET S-301 FOR LOCATIONS.



3 Column Grid Line 53 (Facing Northwest)
3/32" = 1'-0"

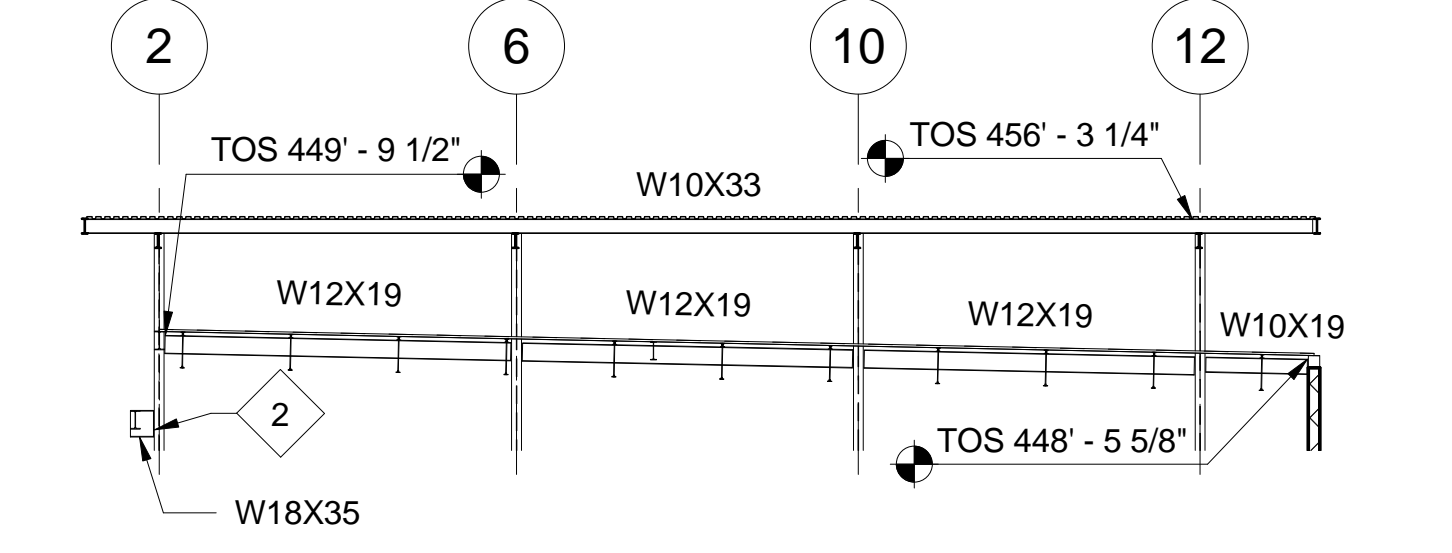




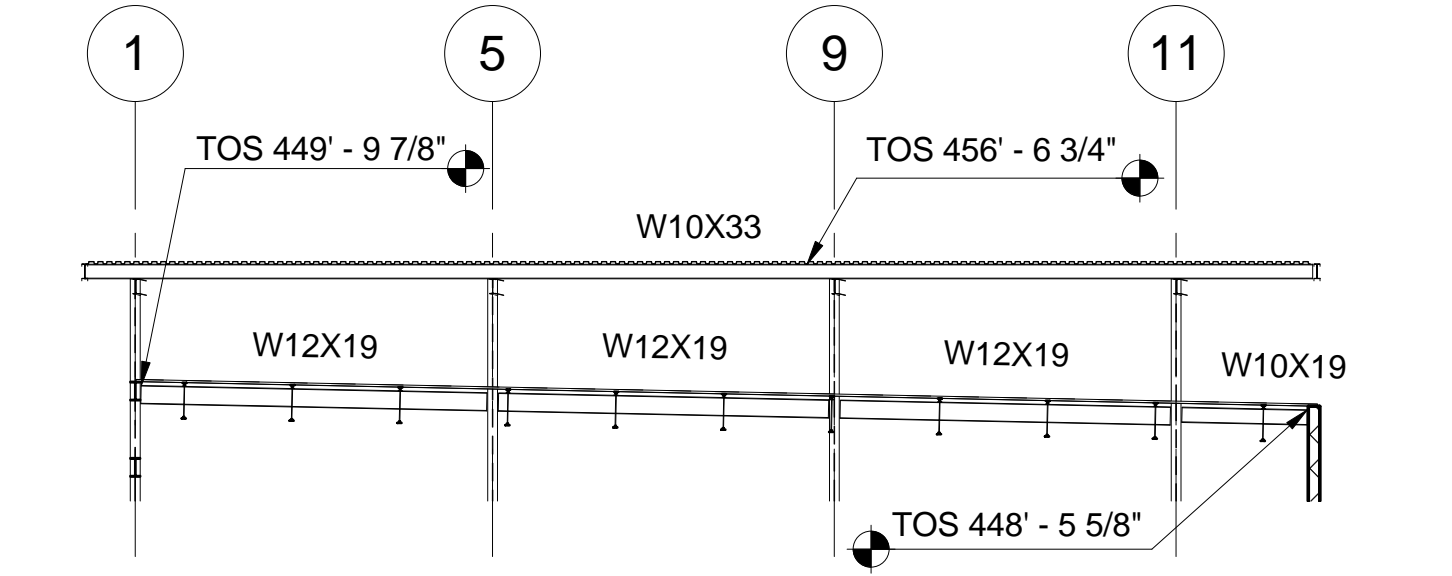
KEY VALUE	KEYNOTE TEXT
1	K SERIES JOIST SHALL HAVE A SEAT DEPTH = 5"
2	MOMENT CONNECTION LRFD FACTORED DESIGN LOAD = 20 K-ft
4	BEAM SHALL BE PLACED ABOVE OPERABLE PARTITION. SEE ARCH SHEETS FOR LOCATION.
31	1 1/2" - 18 GAUGE METAL ROOF DECK

GENERAL NOTES:

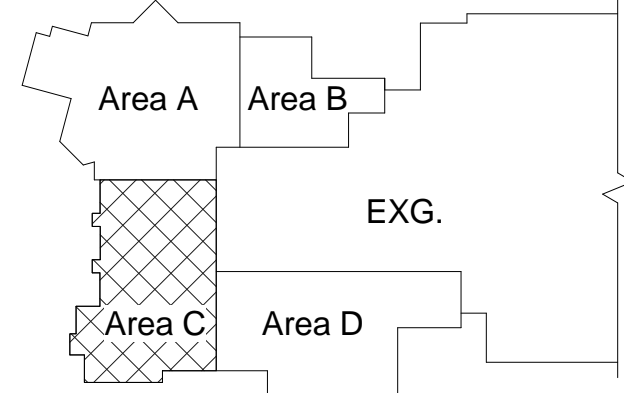
- REFER TO SHEET S-001 & S-002 FOR ADDITIONAL INFORMATION.
- SPACE BETWEEN OWSJ AND ADJACENT BEAM/WALL SHALL NOT EXCEED THE OWSJ SPACING.
- SPACE OWSJ TO AVOID INTERFERING WITH PARTITION WALLS.
- OWSJ SHALL BE SPACED AT 3'-0" O.C. UNLESS NOTED OTHERWISE.
- SPACING OF OWSJ IS MAXIMUM AS INDICATED. ADDITIONAL OWSJ MAY BE REQUIRED.
- ALL BEAMS ARE SLOPING UNLESS NOTED OTHERWISE.
- SEE SHEET S-401 FOR LINTEL SCHEDULE.
- SEE SHEET S-501 FOR COLUMN SCHEDULE.
- SEE ARCH/MECH SHEETS FOR LOCATIONS AND DIMENSIONS OF MECH UNITS/DUCTS
- SEE S-502 FOR FULL COLUMN GRID MARKS.
- ELEVATIONS SHOWN WITH STRUCTURAL STEEL ARE TOP OF STEEL ELEVATIONS UNLESS NOTED OTHERWISE.



2) Column Grid Line B (Facing North)
3/32" = 1'-0"



3) Column Grid Line A (Facing North)
3/32" = 1'-0"

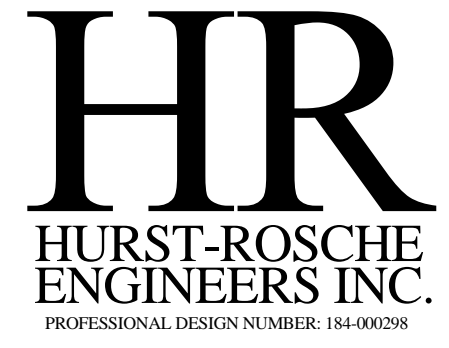


KEYPLAN

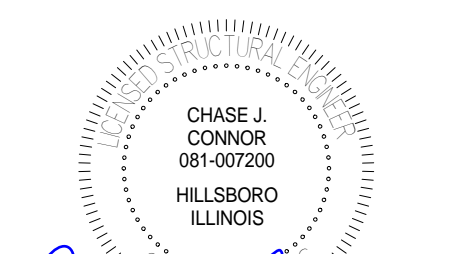


200 N. MARKET STREET
MARION, IL
PH: 618.998.0075
F: 618.998.0076

HILLSBORO, IL
EAST ST. LOUIS, IL
SPRINGFIELD, IL
ARNOLD, MO
NEOSHO, MO



504 W. Jackson Marion, IL 62959
p. 618.993.5904
f. 618.993.5624



SIGNATURE: *Chase Connor*
DATE: 03/19/15
11-30-2016
LICENSE EXPIRES

PHASE 6 - FINE ARTS BUILDING ADDITION
NEW MARION HIGH SCHOOL
MARION C.U.S.D. #2
MARION, WILLIAMSON COUNTY, ILLINOIS

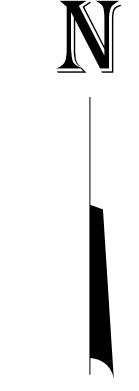
Mark	Date	Description

DATE: 03/19/15
PROJECT NO: 360-2632
DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

ROOF FRAMING PLAN
- AREA 'C'

S-303

1) Roof Framing - Area C
1/8" = 1'-0"





Chase Connor
 SIGNATURE

03/19/15
 DATE

11-30-2016
 LICENSE EXPIRES

LICENSE EXPIRES

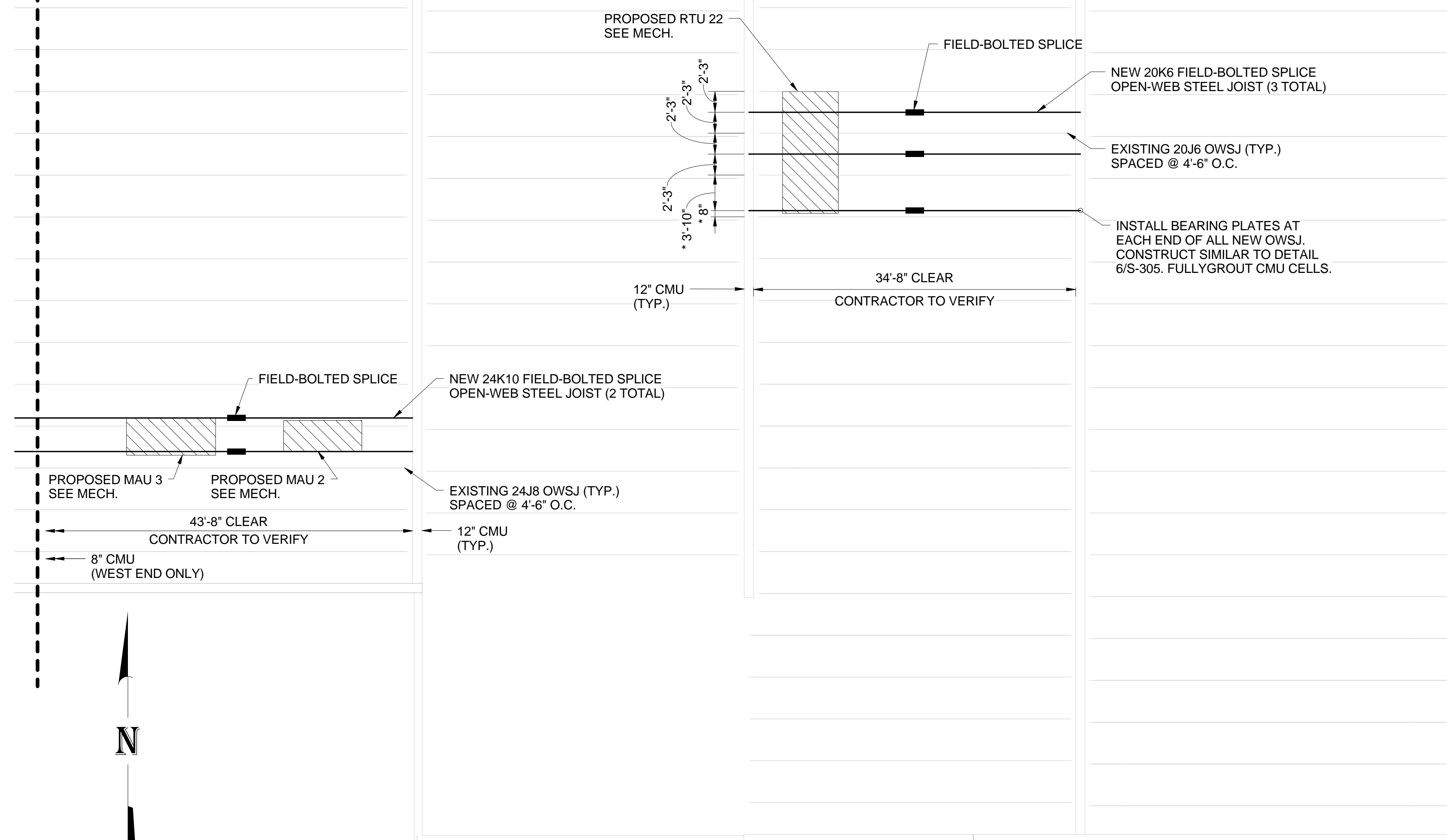
PHASE 6 - FINE ARTS BUILDING ADDITION
 NEW MARION HIGH SCHOOL
 MARION C.U.S.D. #2
 MARION, WILLIAMSON COUNTY, ILLINOIS

GENERAL NOTES:

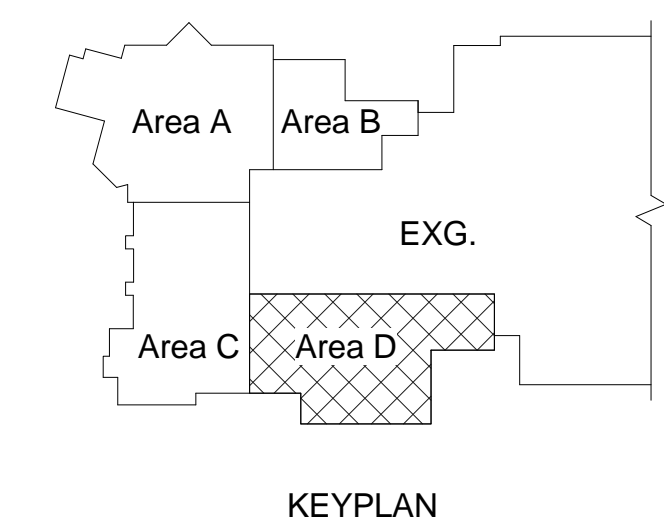
- REFER TO S-001 & S-002 FOR ADDITIONAL INFORMATION
- INSTALLATION OF JOISTS SHALL BE COMPLETED PRIOR TO INSTALLING ROOFTOP MECHANICAL EQUIPMENT.
- PERMANENT AND TEMPORARY BRACING AND BRIDGING OF BOTH NEW AND EXISTING OWSJ SHALL BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH S.J.I. SPECIFICATIONS.

* COORDINATE LOCATION OF NEW OWSJ AS CLOSE AS PRACTICAL WITH CENTERLINE OF PROPOSED SEISMIC ISOLATION CURB.

MATCHLINE SEE SHEET S-303



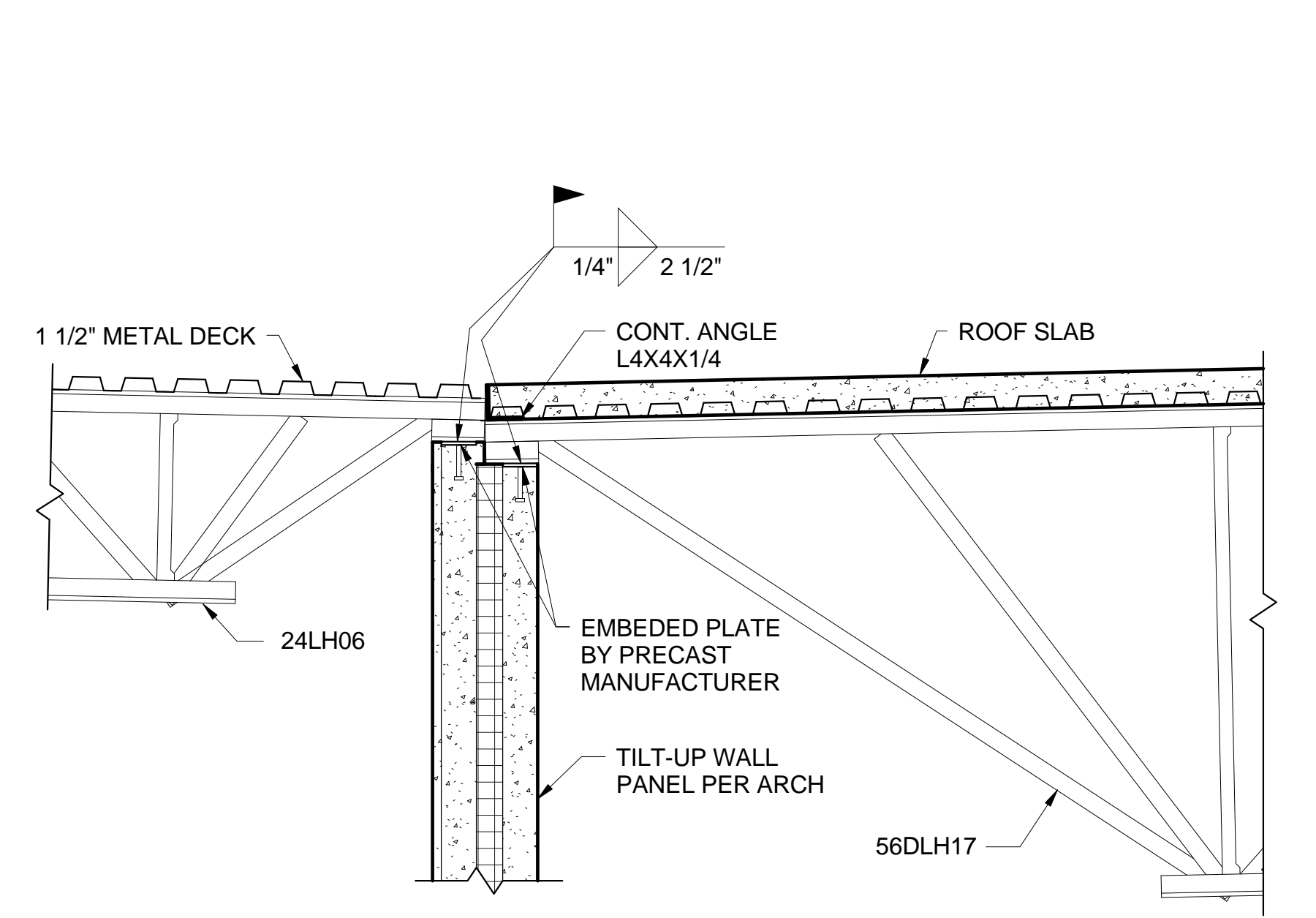
1 Roof Framing - Area D
 1/8" = 1'-0"



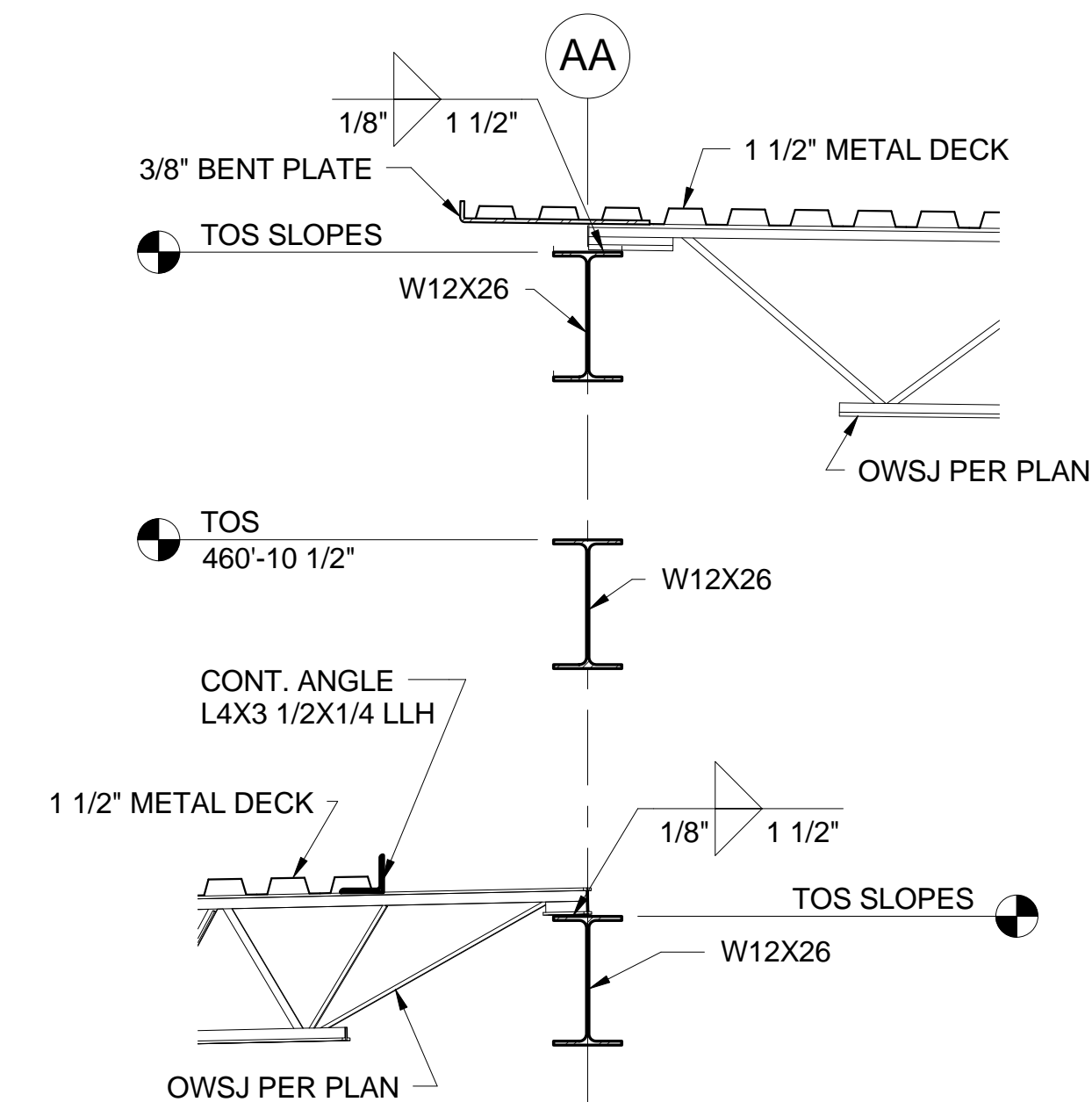
Mark	Date	Description

DATE: 03/19/15
 PROJECT NO: 360-2632
 DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

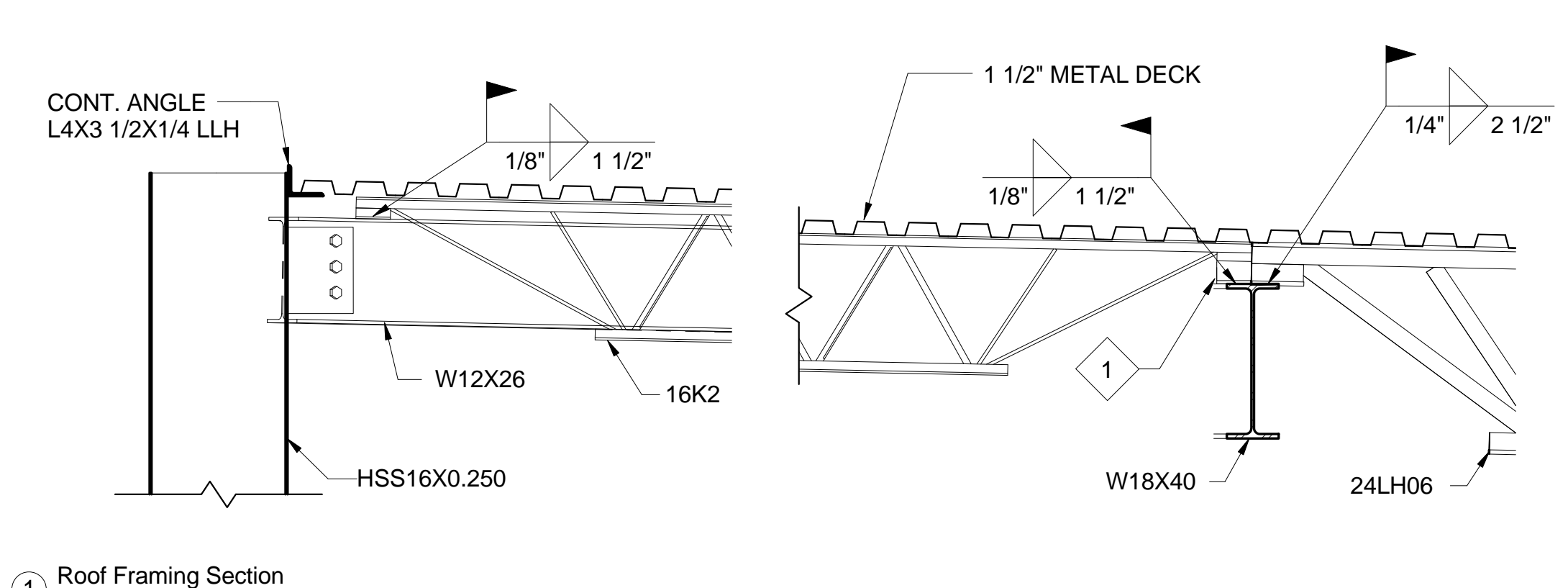
ROOF FRAMING PLAN - AREA 'D'



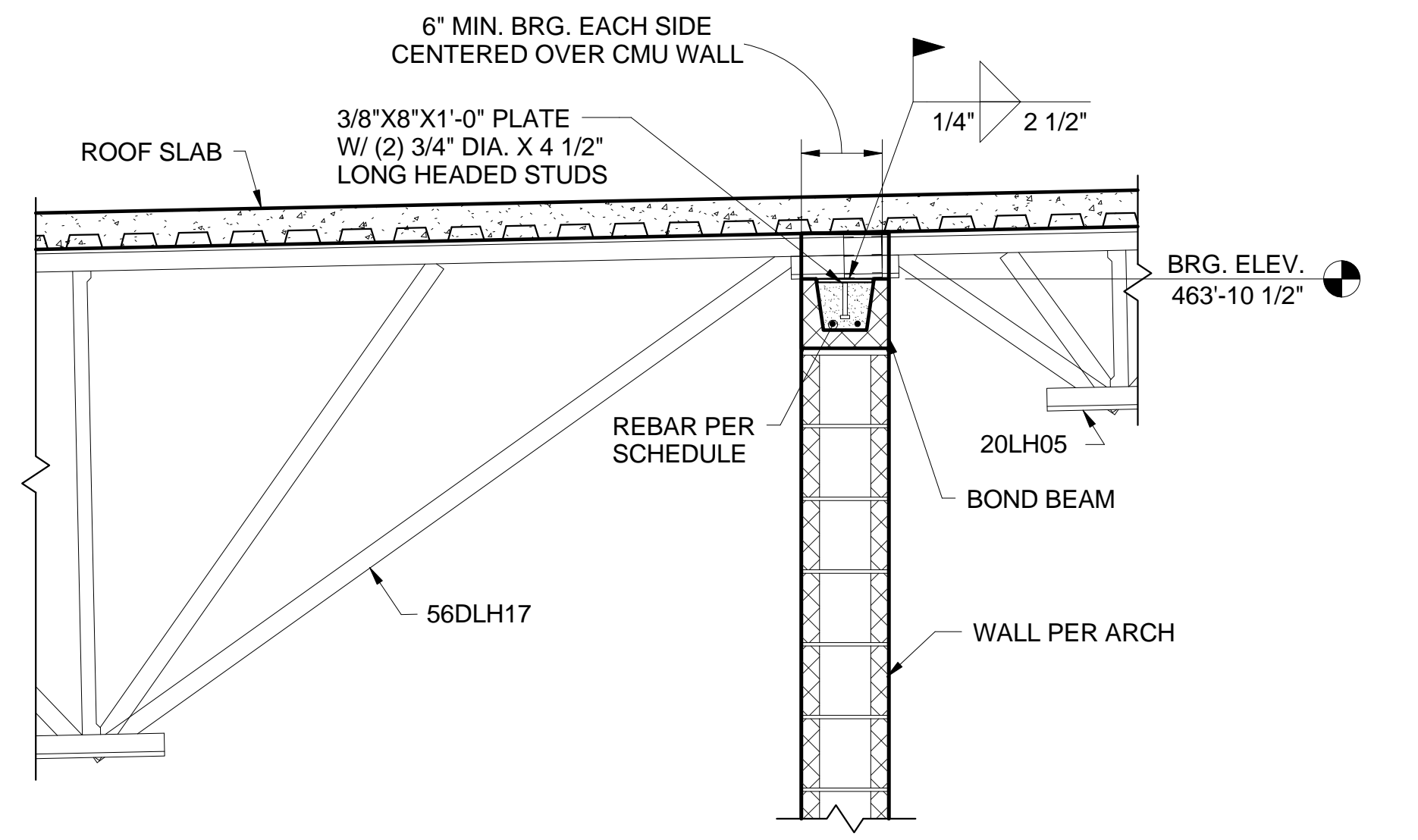
3 Roof Framing Section
3/4" = 1'-0"



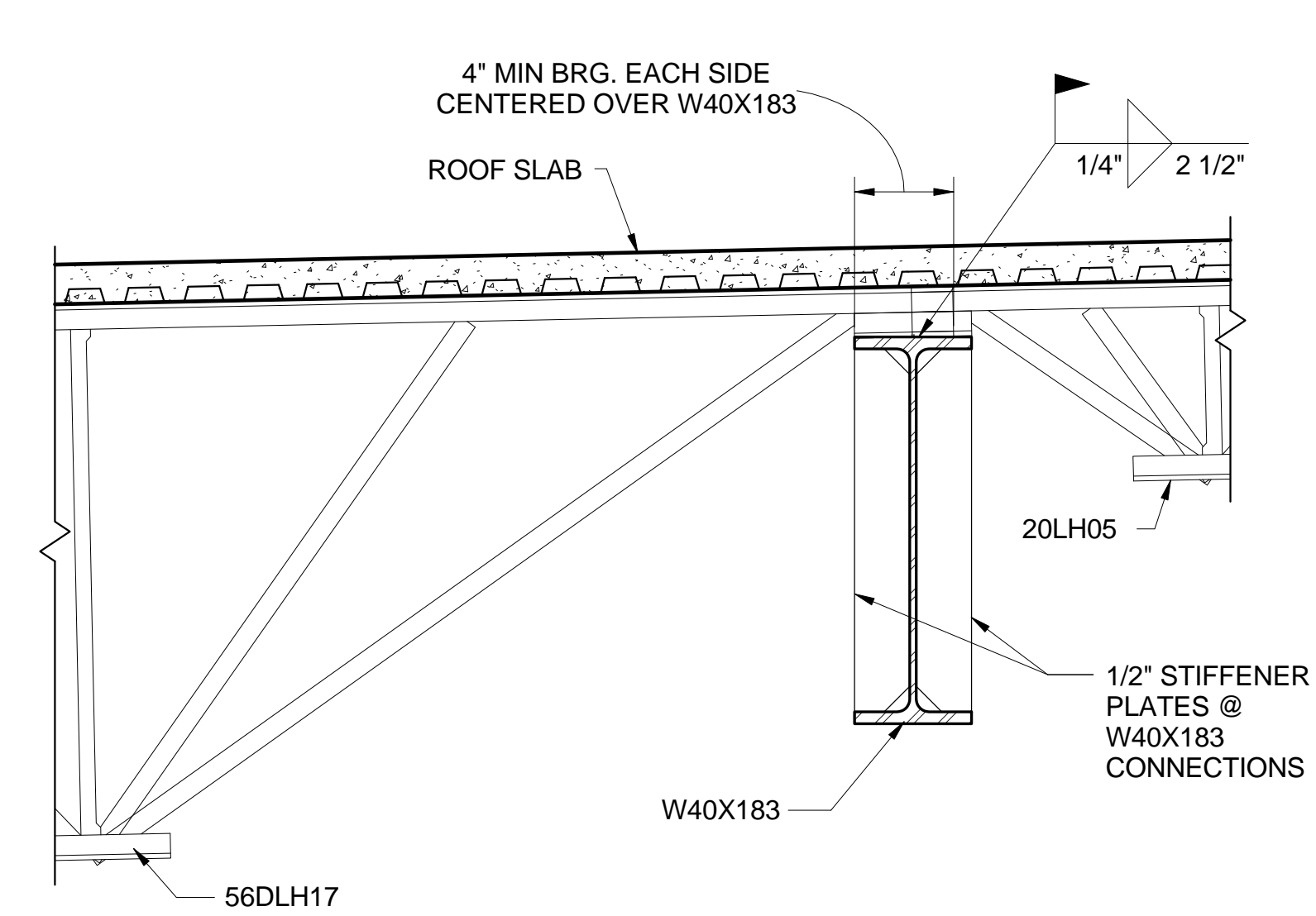
2 Roof Framing Section
3/4" = 1'-0"



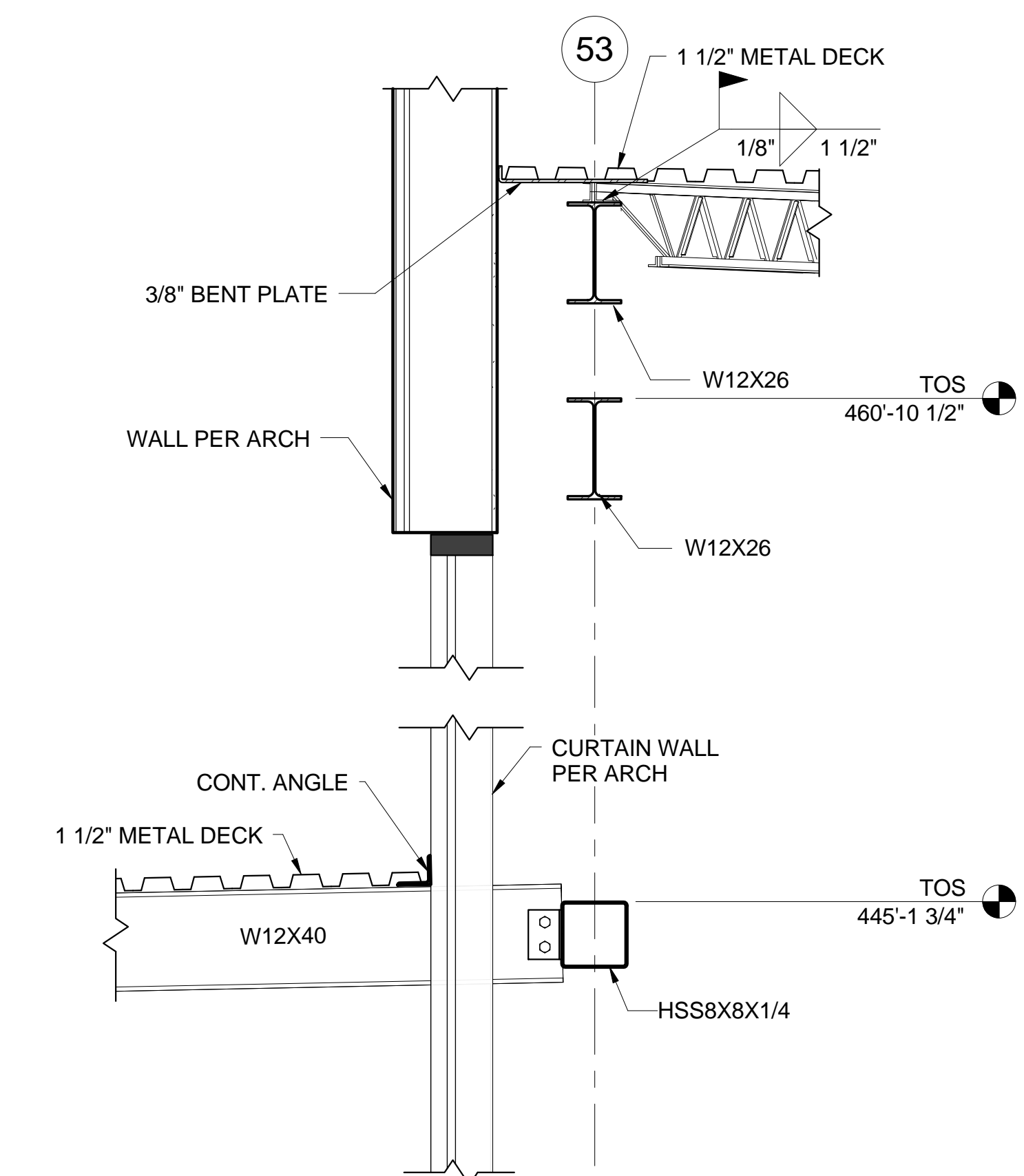
1 Roof Framing Section
3/4" = 1'-0"



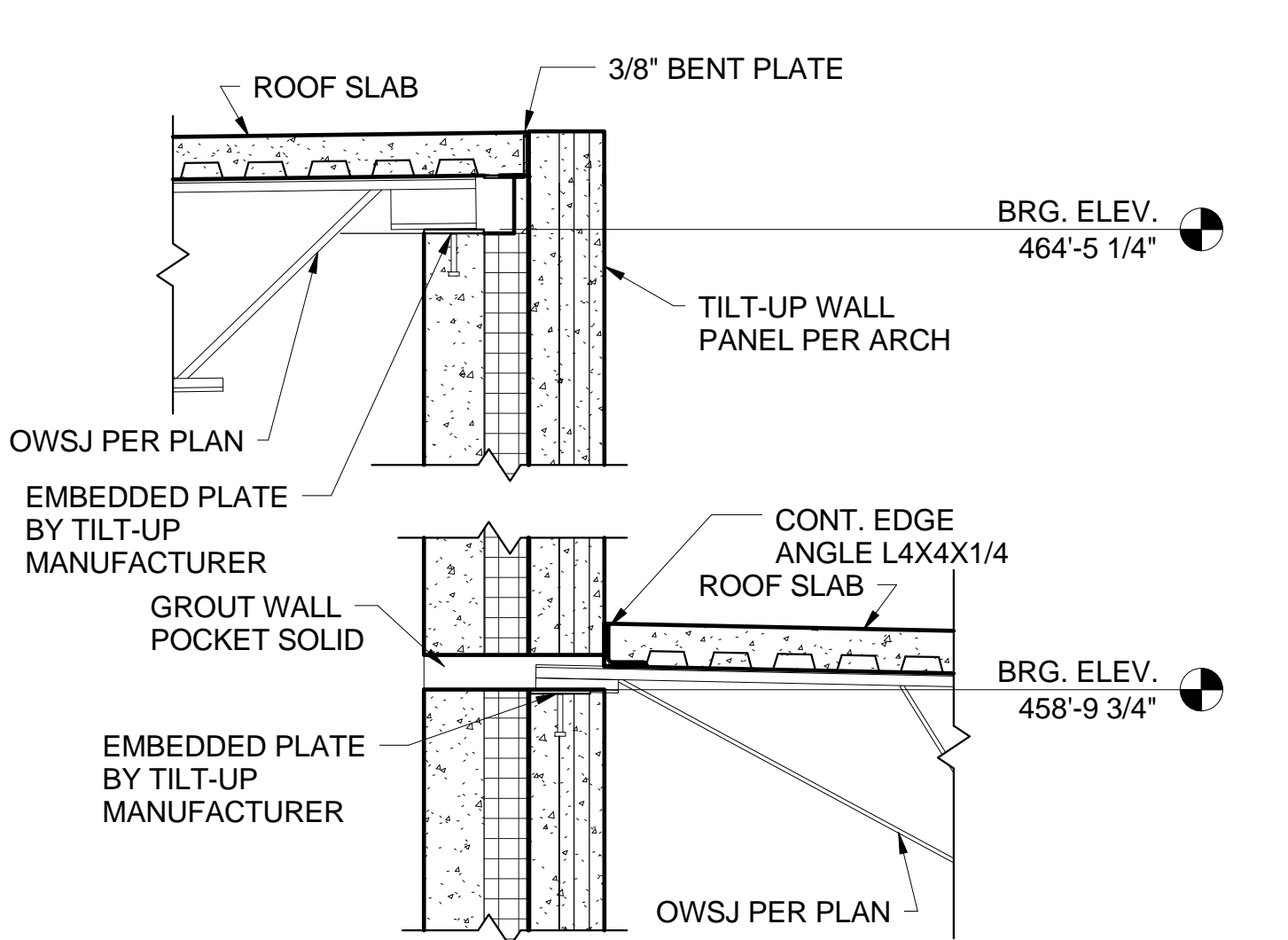
6 Roof Framing Section
3/4" = 1'-0"



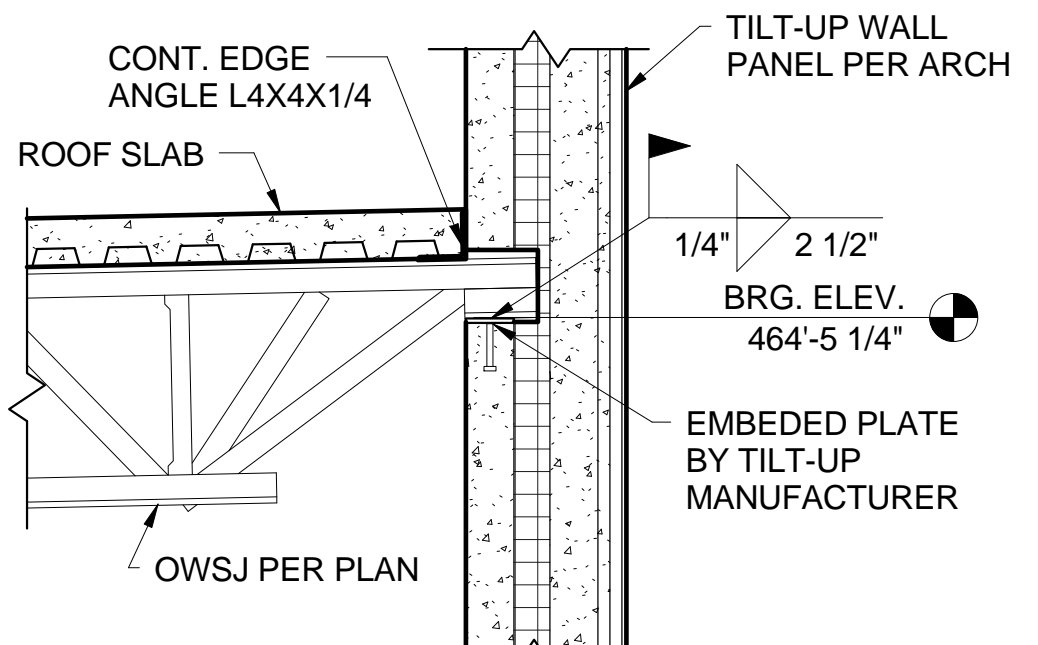
5 Roof Framing Section
3/4" = 1'-0"



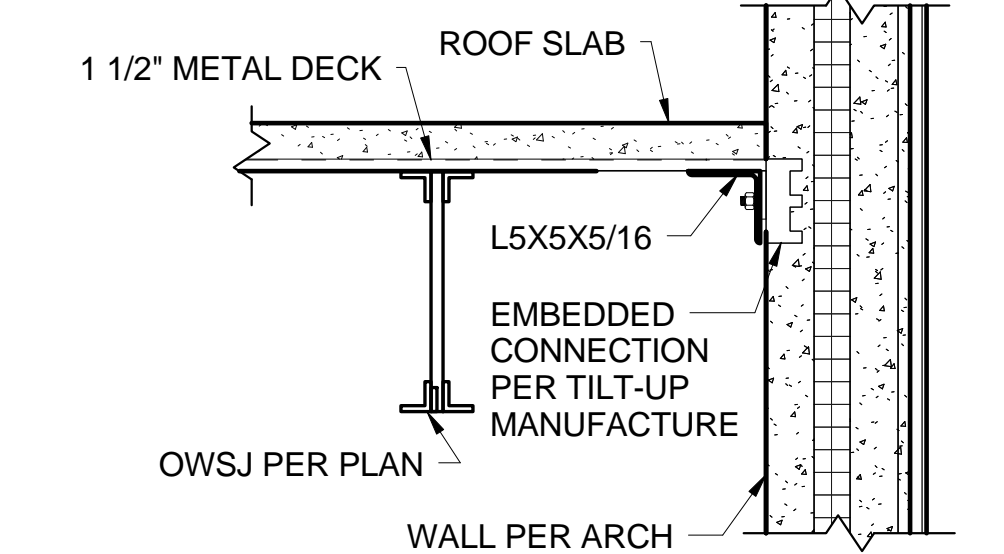
4 Roof Framing Section
3/4" = 1'-0"



9 Roof Framing Section
3/4" = 1'-0"



8 Roof Framing Section
3/4" = 1'-0"

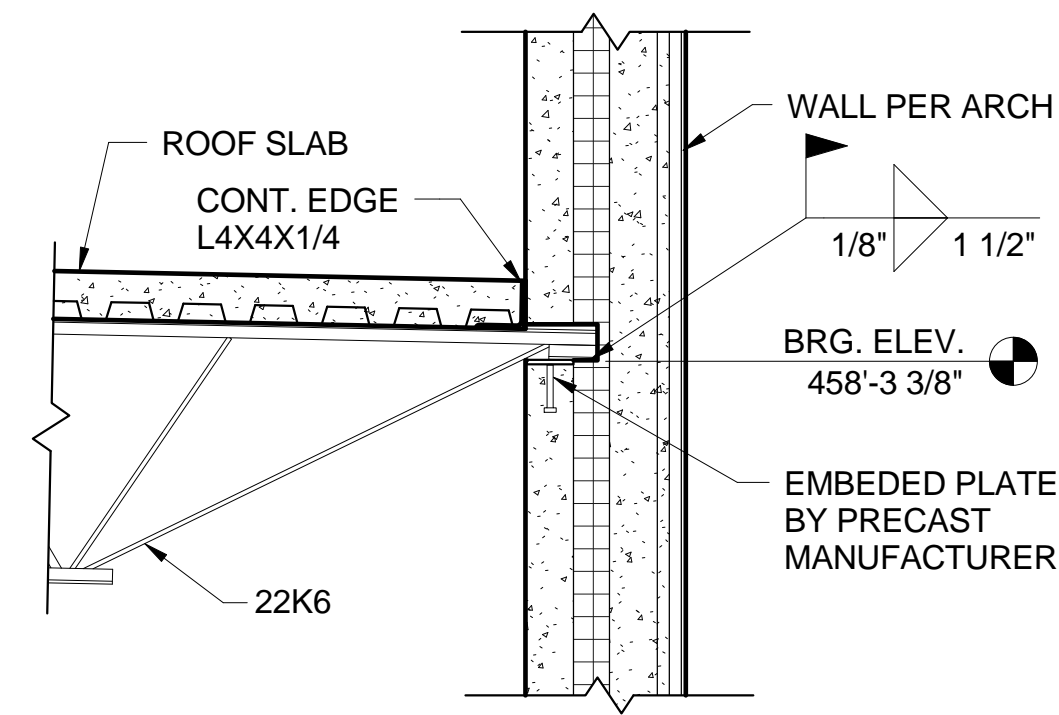


7 Roof Framing Section
3/4" = 1'-0"

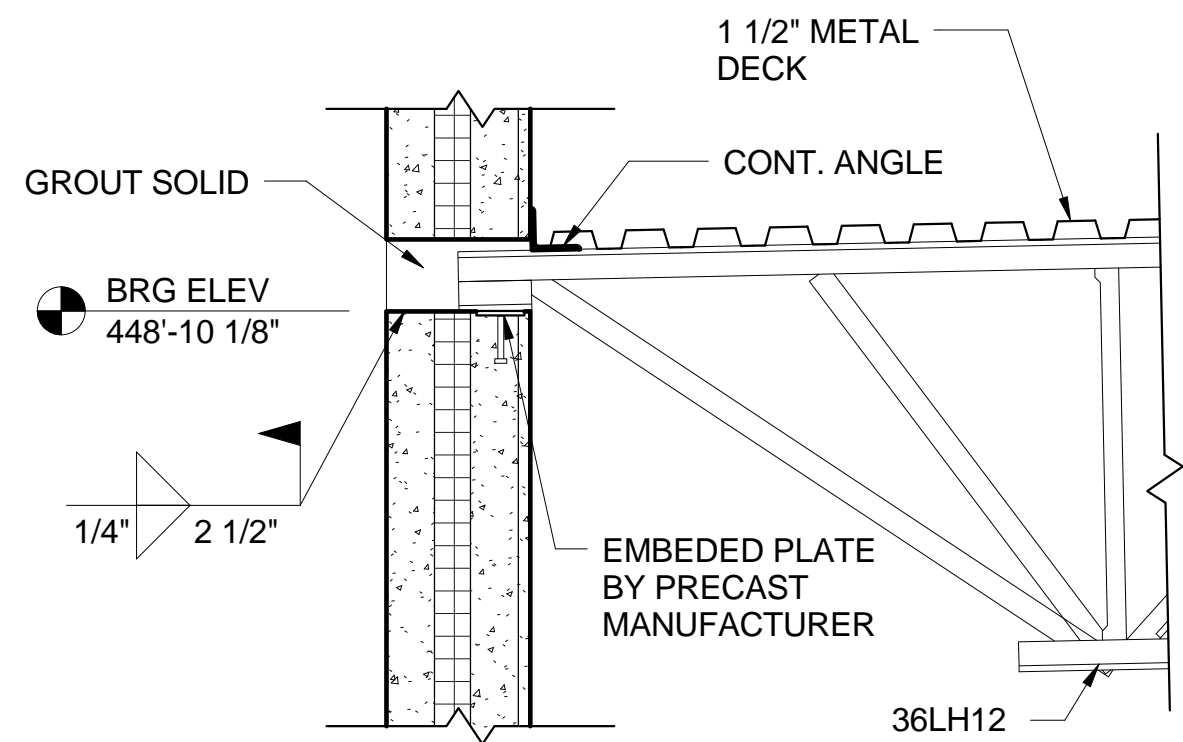
KEYNOTE LEGEND	
KEY VALUE	KEYNOTE TEXT
1	K SERIES JOIST SHALL HAVE A SEAT DEPTH = 5"

GENERAL NOTES:

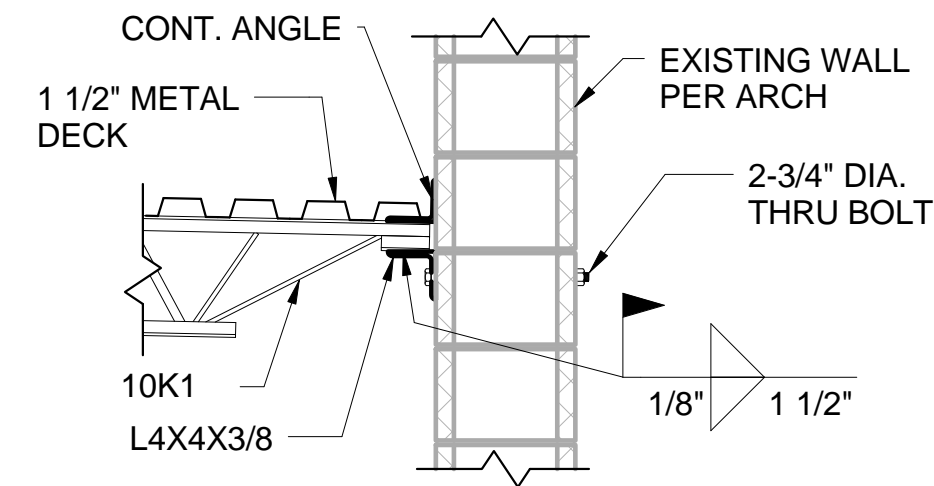
- REFER TO SHEET S-001 & S-002 FOR ADDITIONAL INFORMATION.
- WALL DETAILS SHOWN ARE FOR STRUCTURAL REPRESENTATION ONLY, SEE ARCH SHEETS FOR SIZE, TEXTURE, COLOR, ETC.
- WALL PANEL POCKETS SHALL BE SIZED TO ALLOW FOR JOIST ERECTION.



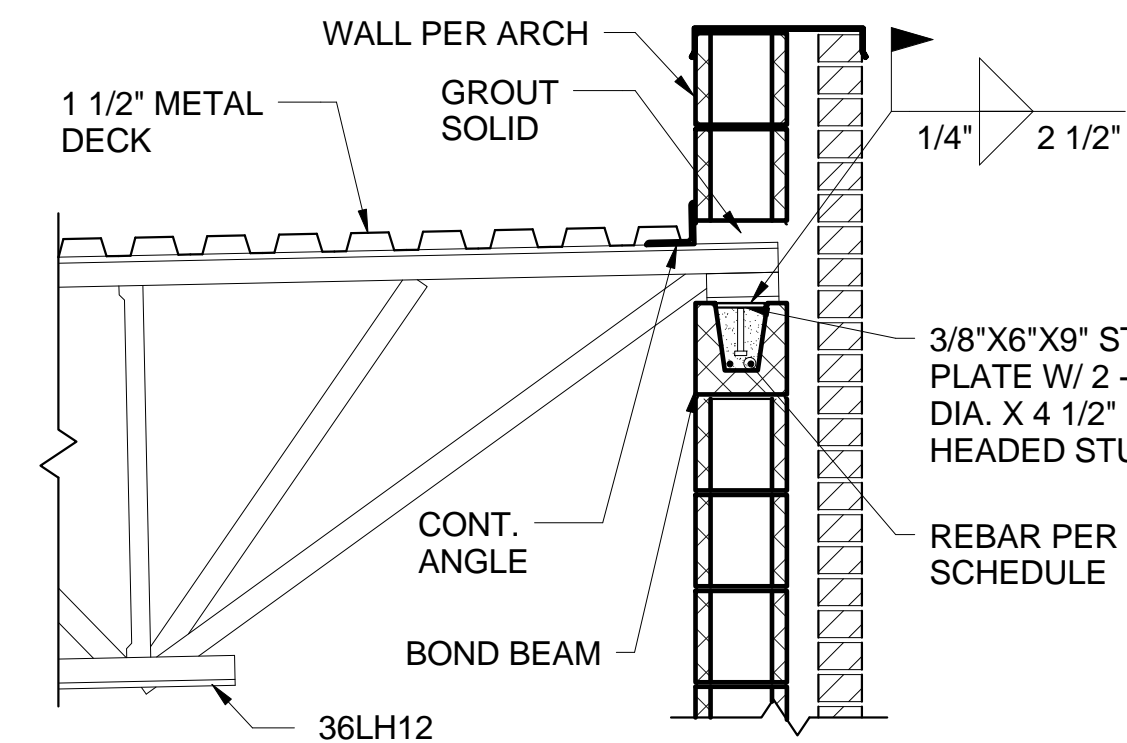
1 Roof Framing Section
3/4" = 1'-0"



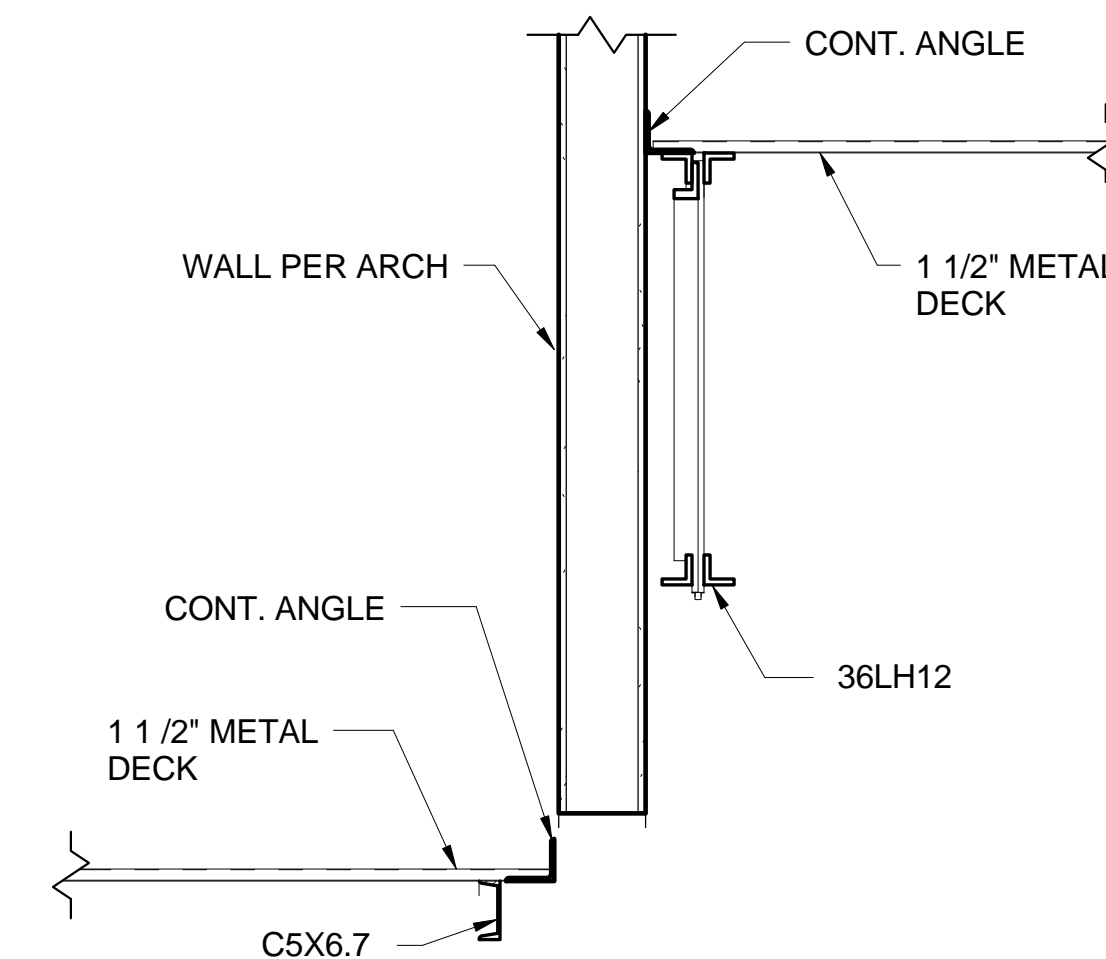
2 Roof Framing Section
3/4" = 1'-0"



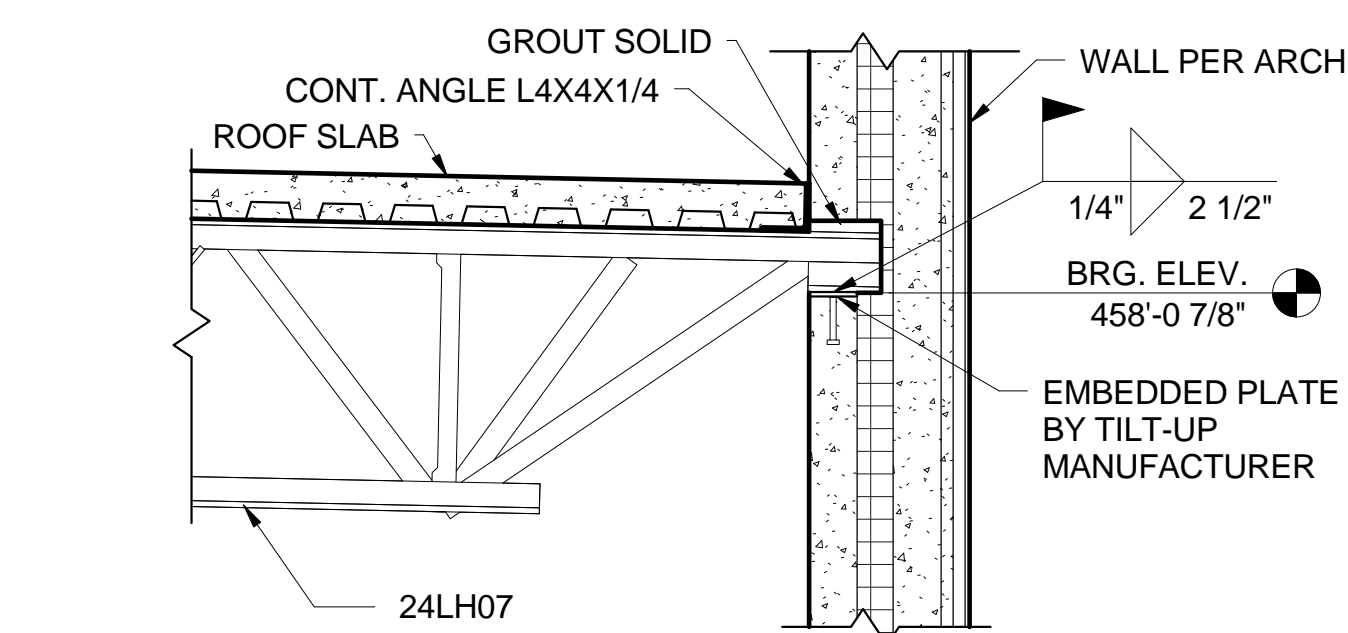
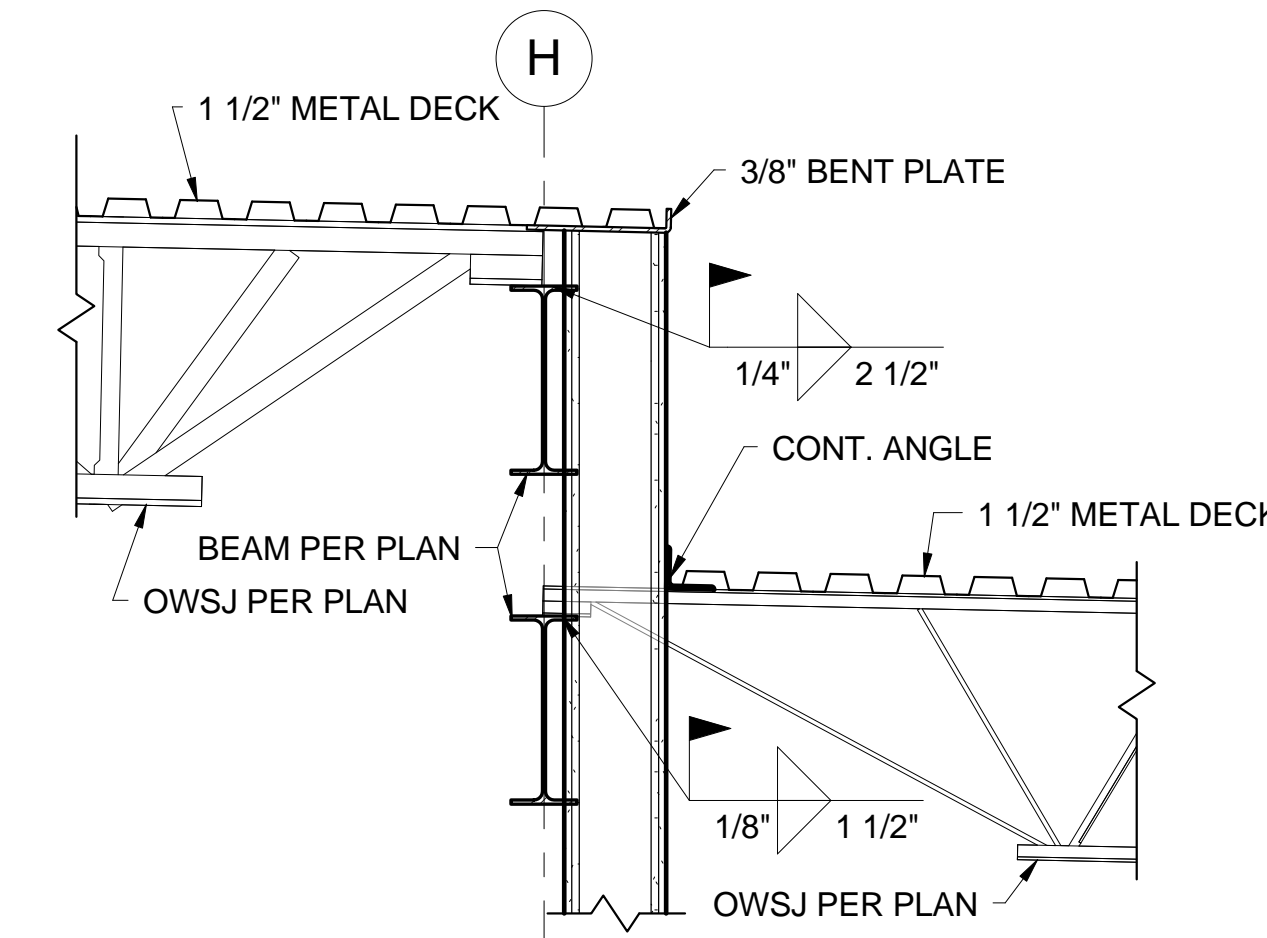
3 Roof Framing Section
3/4" = 1'-0"



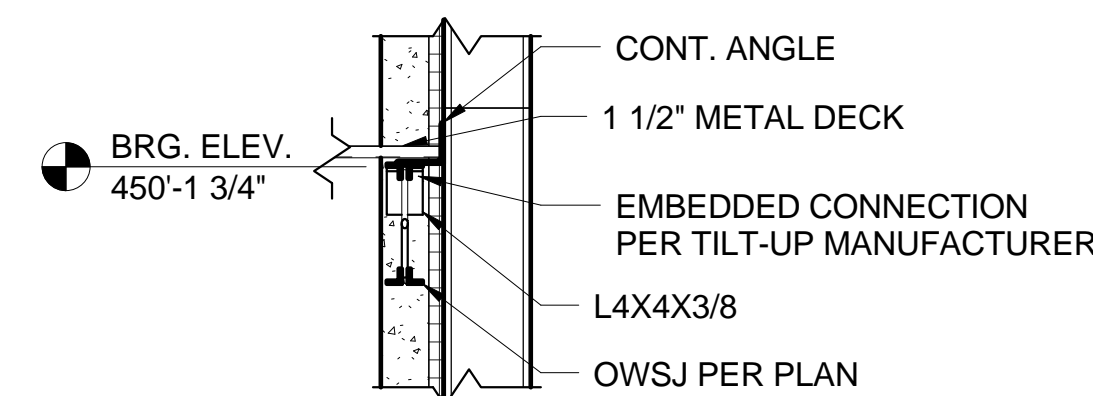
4 Roof Framing Section
3/4" = 1'-0"



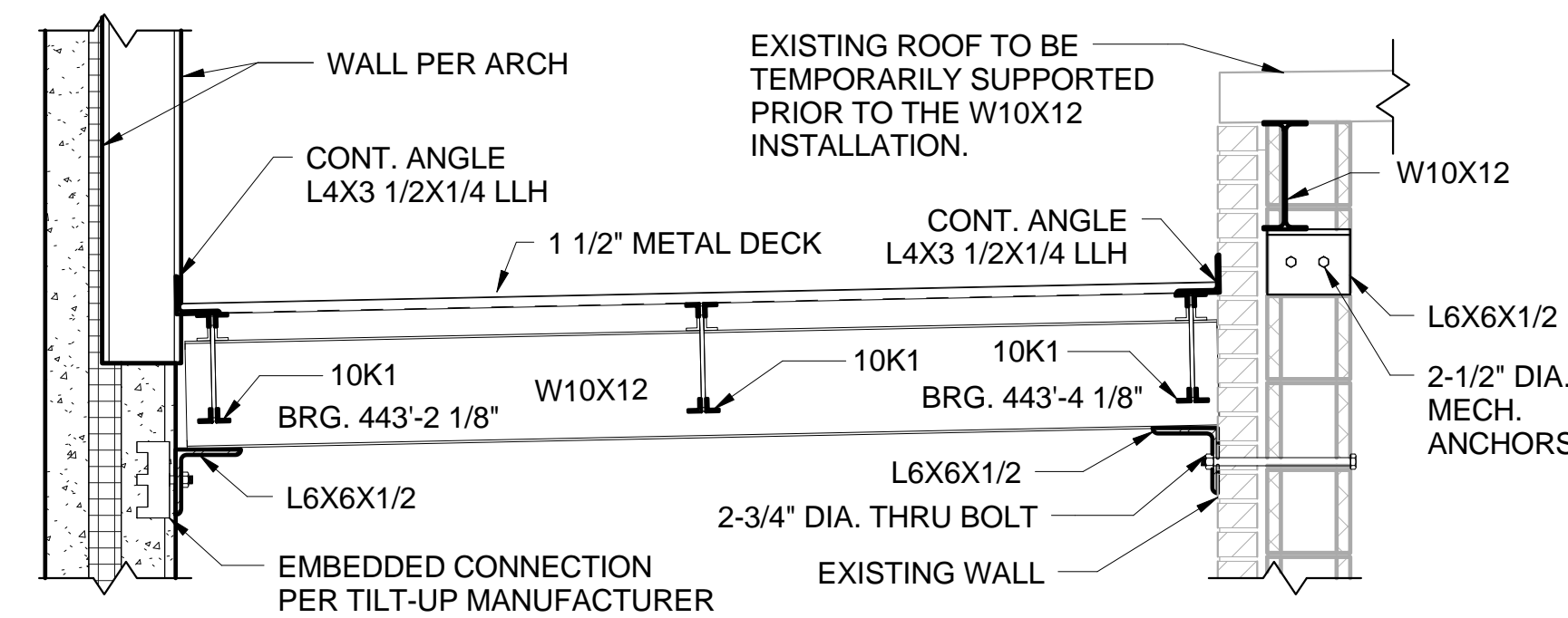
5 Roof Framing Section
3/4" = 1'-0"



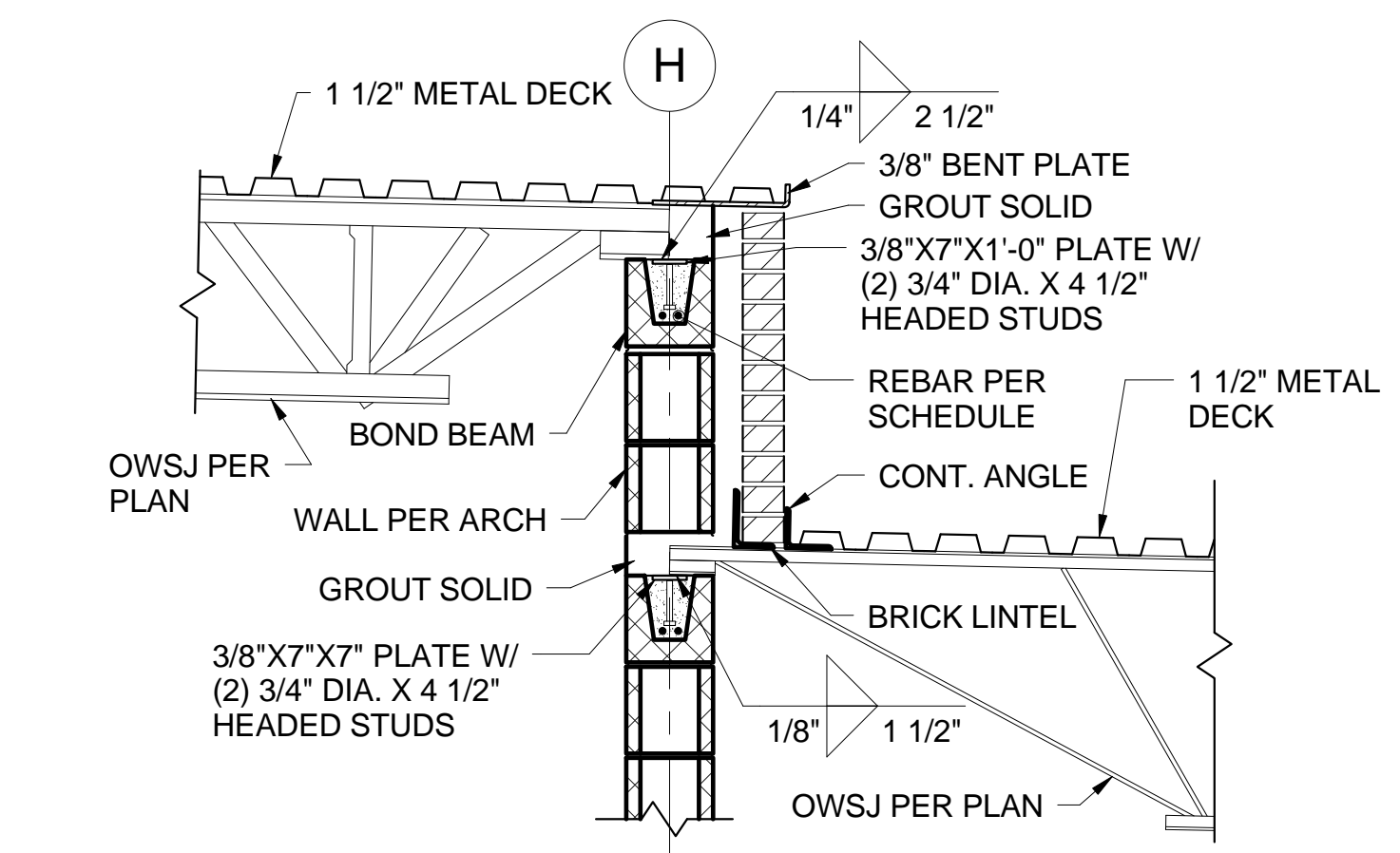
6 Roof Framing Section
3/4" = 1'-0"



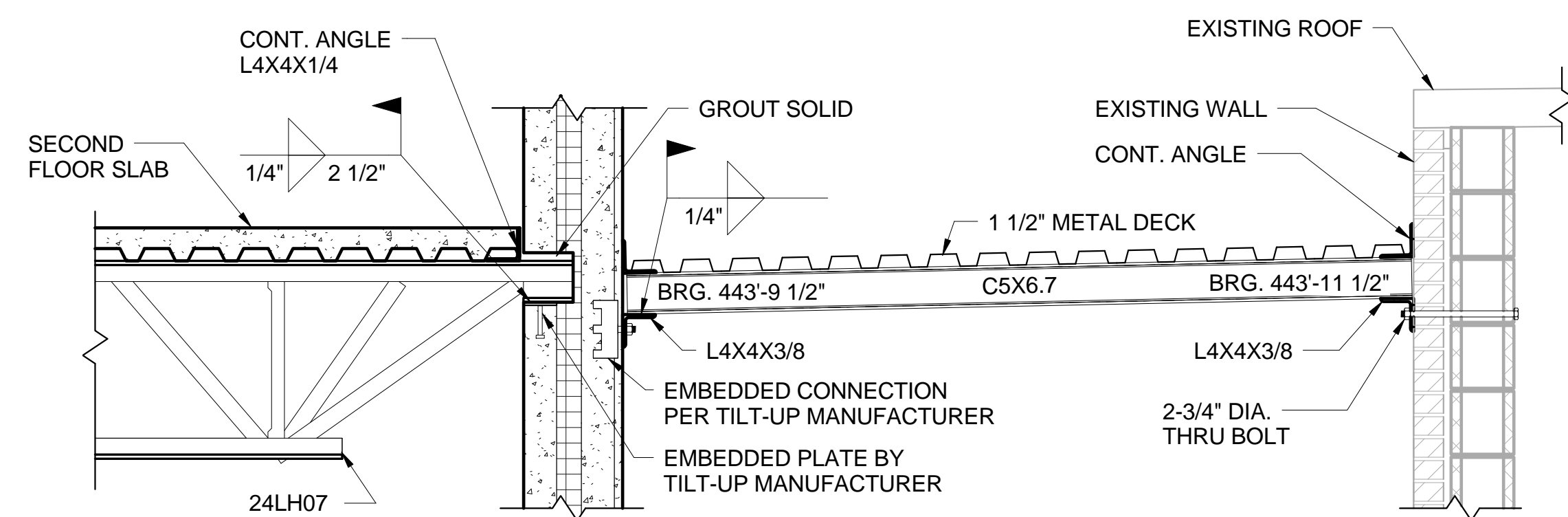
7 Roof Framing Section
3/4" = 1'-0"



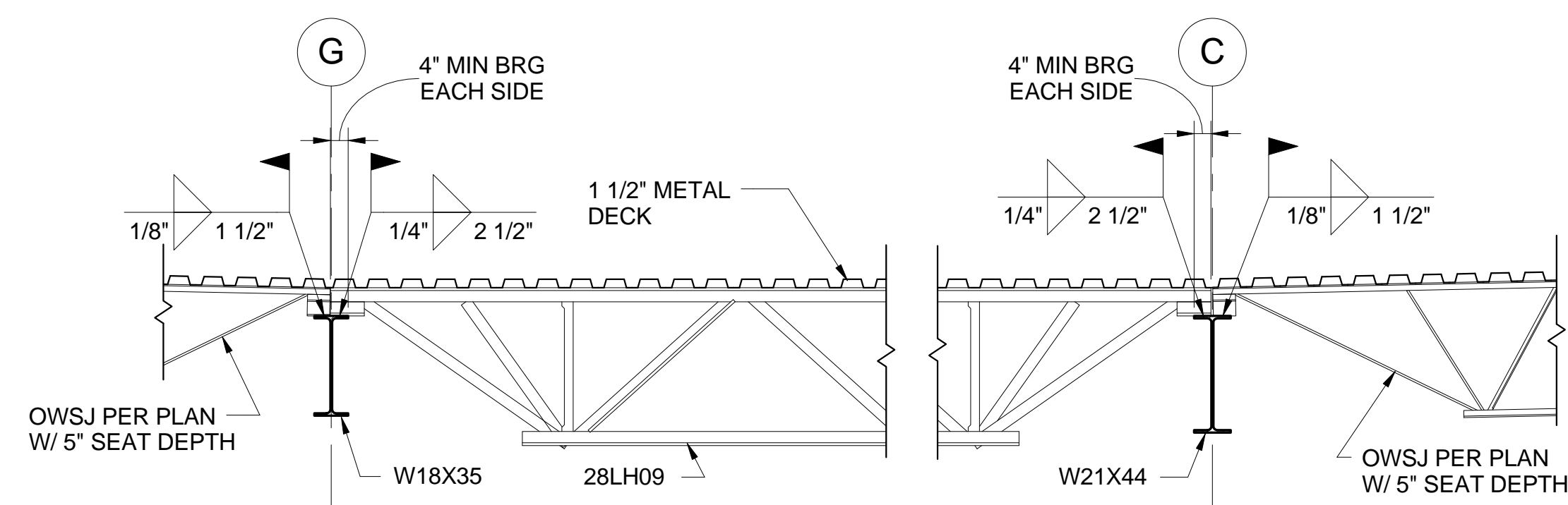
8 Roof Framing Section
3/4" = 1'-0"



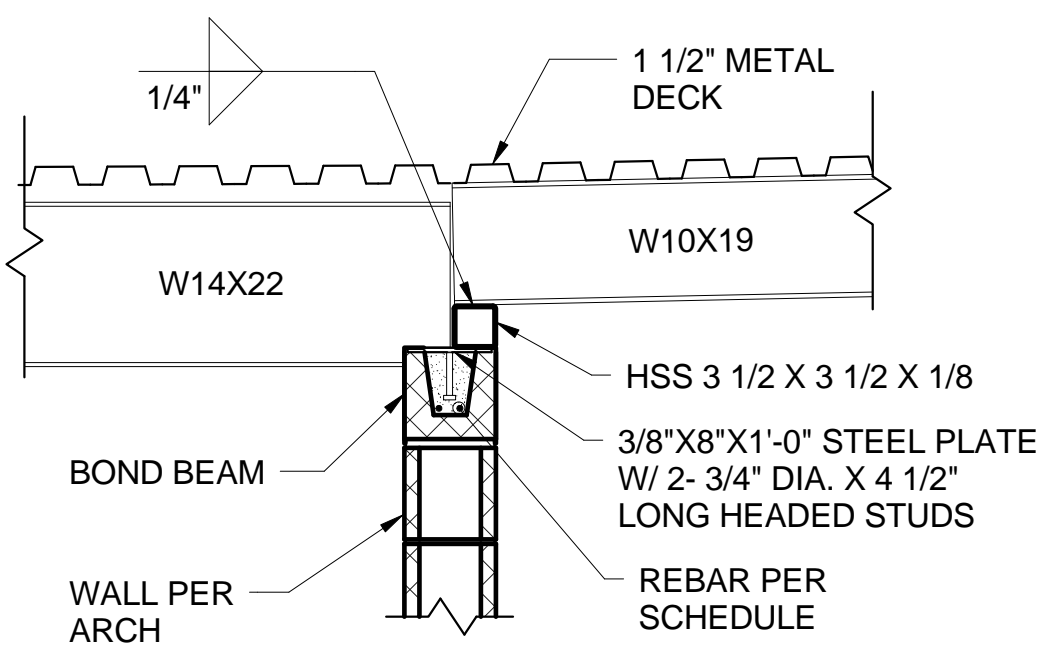
9 Roof Framing Section
3/4" = 1'-0"



10 Roof Framing Section
3/4" = 1'-0"



11 Roof Framing Section
1/2" = 1'-0"



12 Roof Framing Section
3/4" = 1'-0"

GENERAL NOTES:

1. REFER TO SHEET S-001 & S-002 FOR ADDITIONAL INFORMATION.
2. WALL DETAILS SHOWN ARE FOR STRUCTURAL REPRESENTATION ONLY, SEE ARCH SHEETS FOR SIZE, TEXTURE, COLOR, ETC.
3. WALL PANEL POCKETS SHALL BE SIZED TO ALLOW FOR JOIST ERECTION.

Mark	Date	Description

DATE: 03/19/15		
PROJECT NO: 360-2632		
DESIGN: C.J.C.	DRAWN: N.A.L.	CHECK: R.V.B.

ROOF FRAMING
DETAILS



Chase Connor
 SIGNATURE

03/19/15
 DATE

11-30-2016
 LICENSE EXPIRES

PHASE 6 - FINE ARTS BUILDING ADDITION
 NEW MARION HIGH SCHOOL
 MARION C.U.S.D. #2
 MARION, WILLIAMSON COUNTY, ILLINOIS

Mark	Date	Description

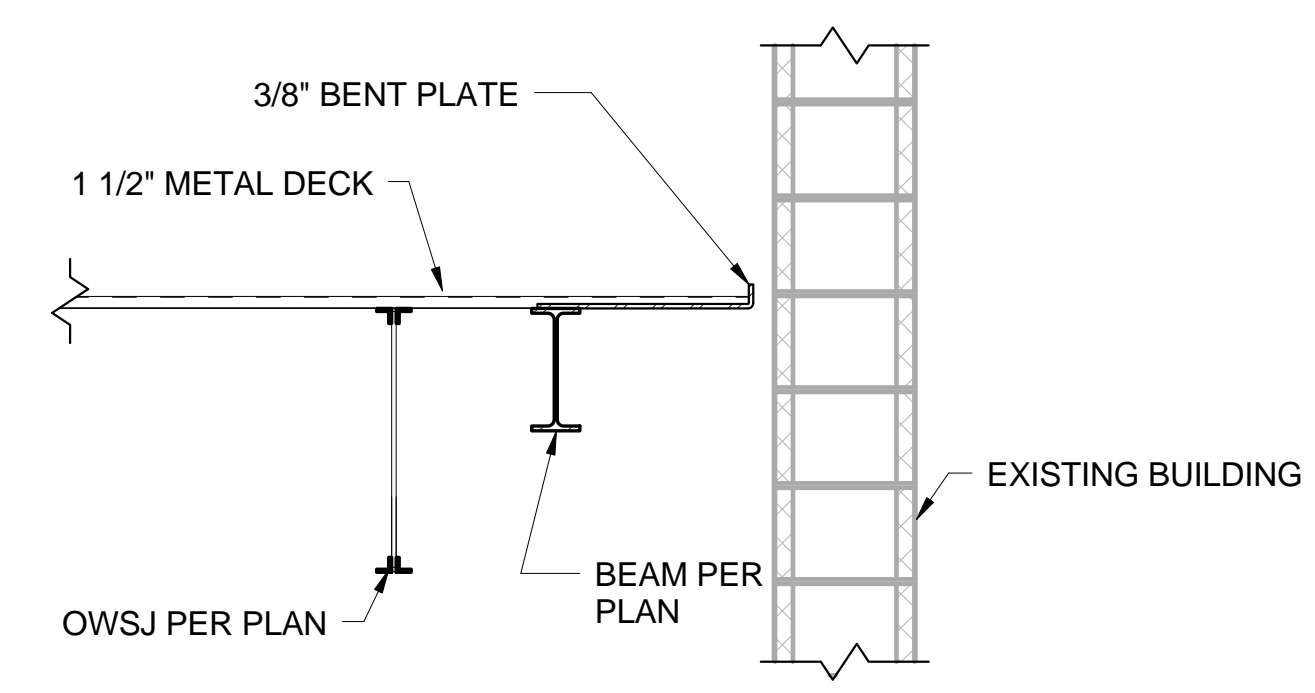
DATE: 03/19/15
 PROJECT NO: 360-2632
 DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

ROOF FRAMING
 DETAILS

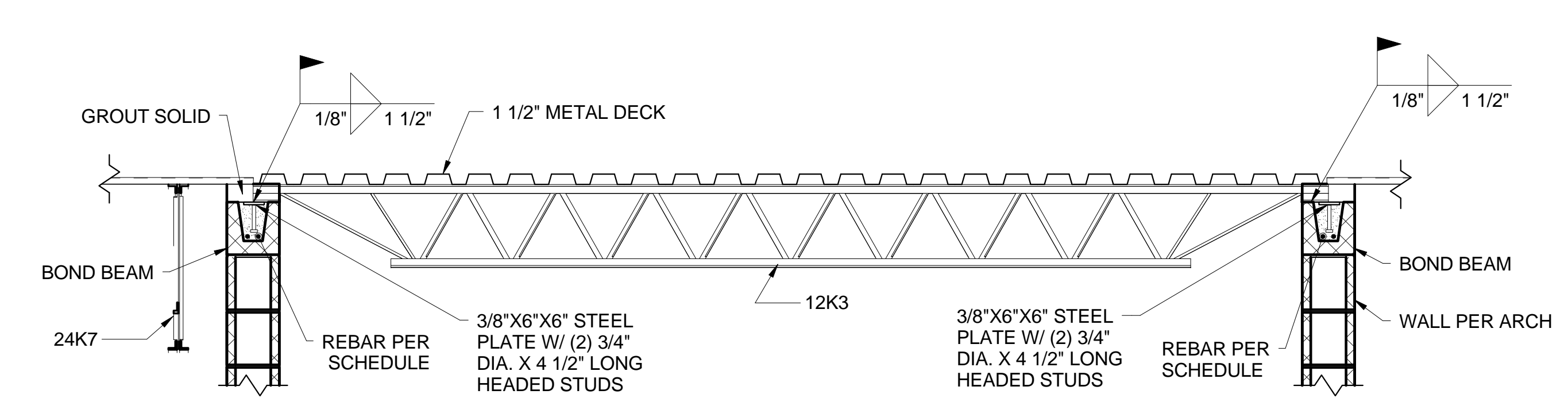
S-307

GENERAL NOTES:

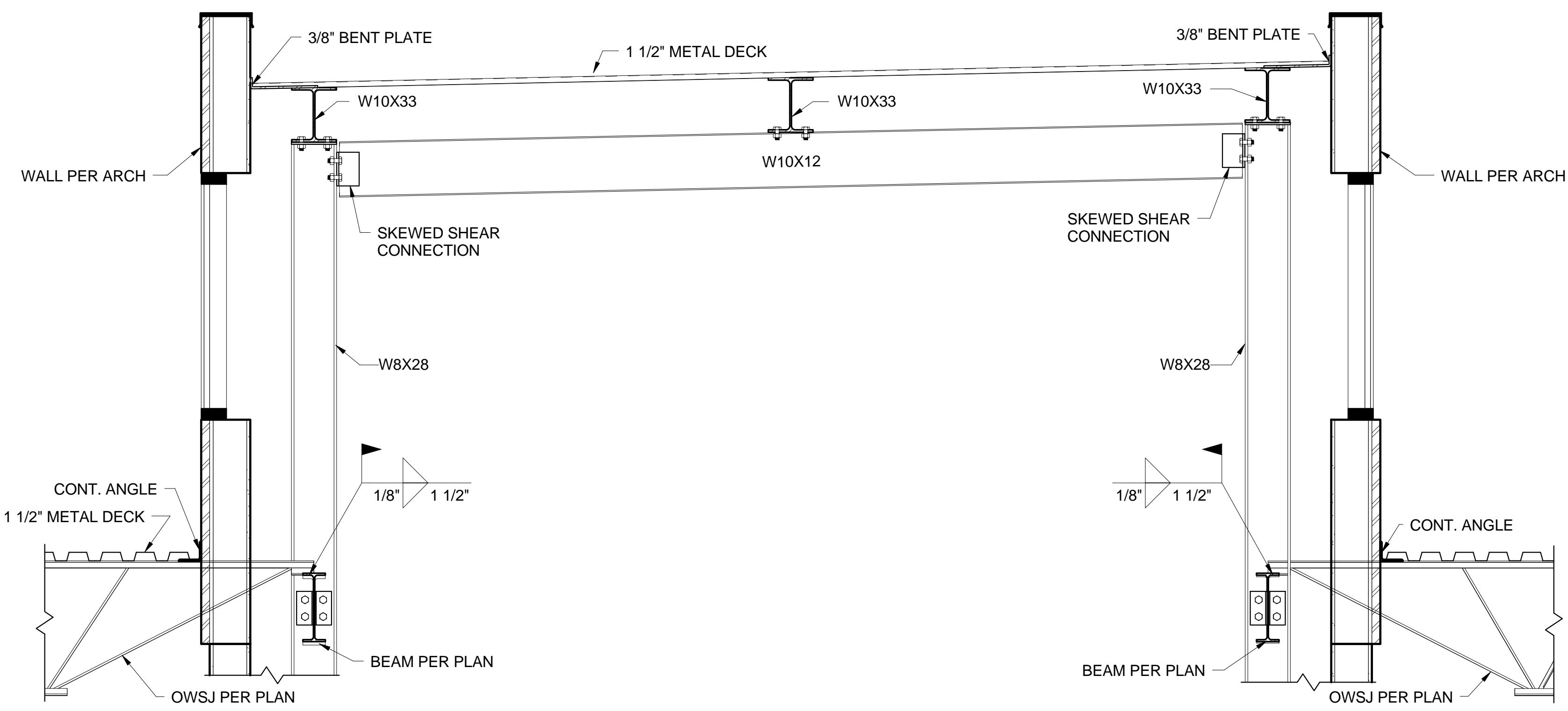
1. REFER TO SHEET S-001 & S-002 FOR ADDITIONAL INFORMATION.
2. WALL DETAILS SHOWN ARE FOR STRUCTURAL REPRESENTATION ONLY. SEE ARCH SHEETS FOR SIZE, TEXTURE, COLOR, ETC.
3. WALL PANEL POCKETS SHALL BE SIZED TO ALLOW FOR JOIST ERECTION.



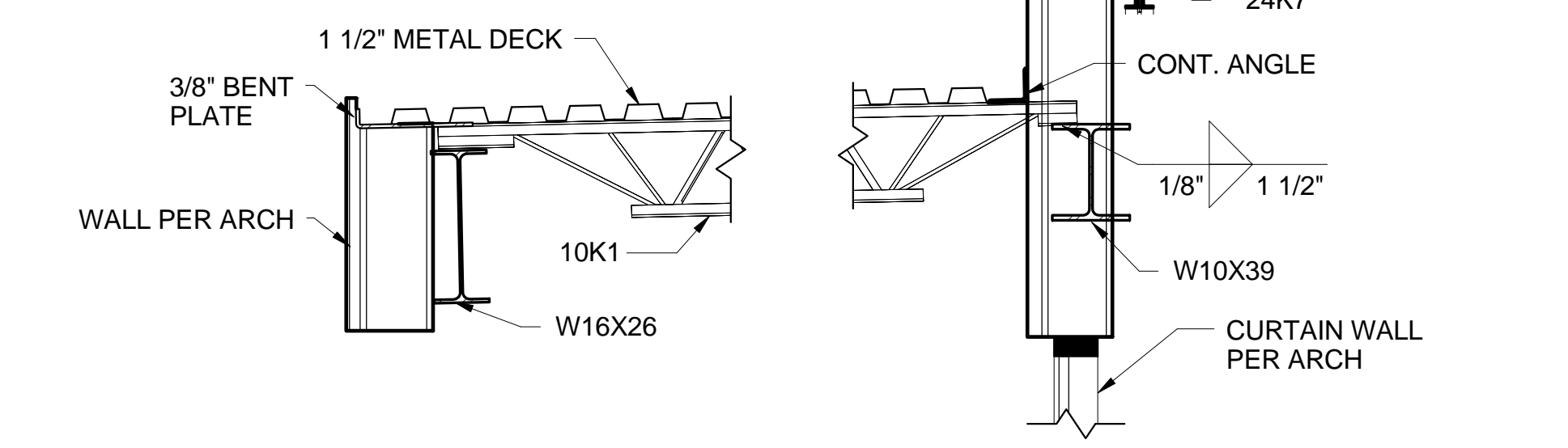
1 Roof Framing Section
 3/4" = 1'-0"



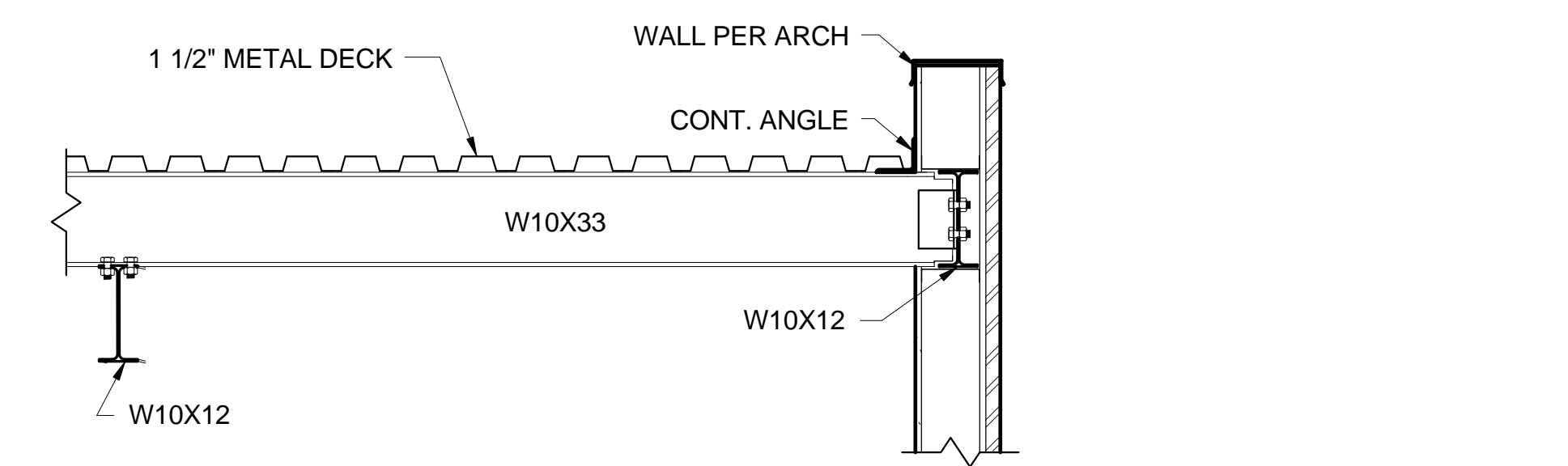
2 Roof Framing Section
 3/4" = 1'-0"



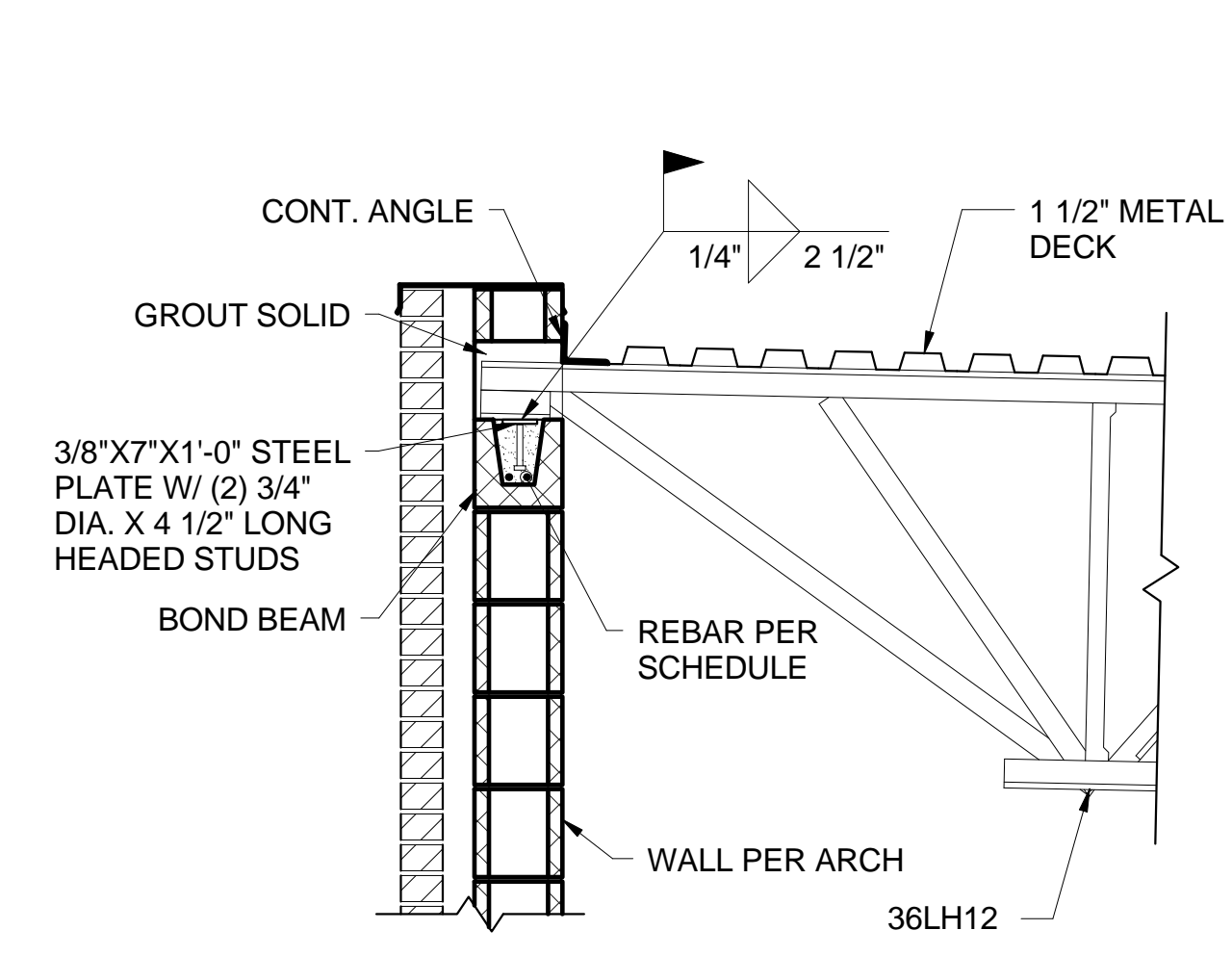
3 Roof Framing Section
 3/4" = 1'-0"



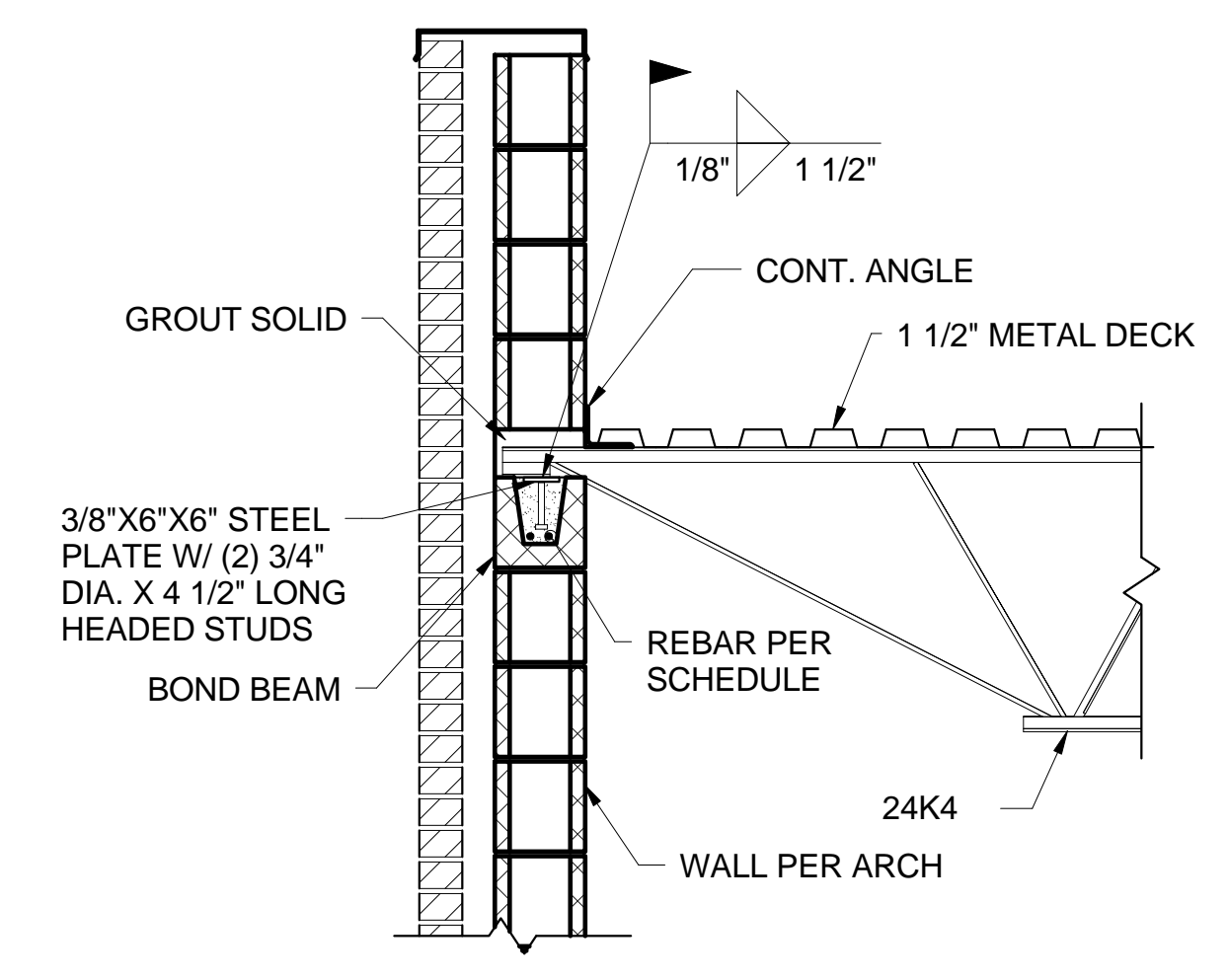
4 Roof Framing Section
 3/4" = 1'-0"



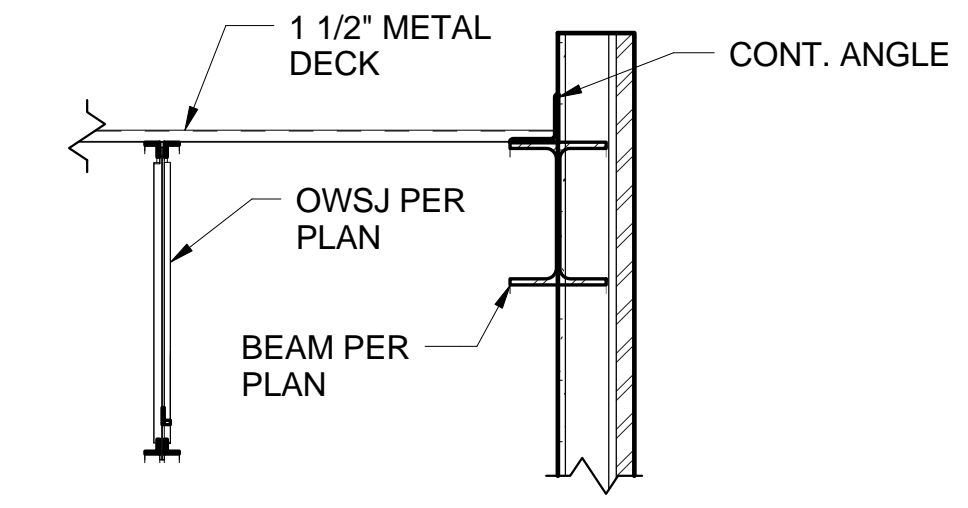
5 Roof Framing Section
 3/4" = 1'-0"



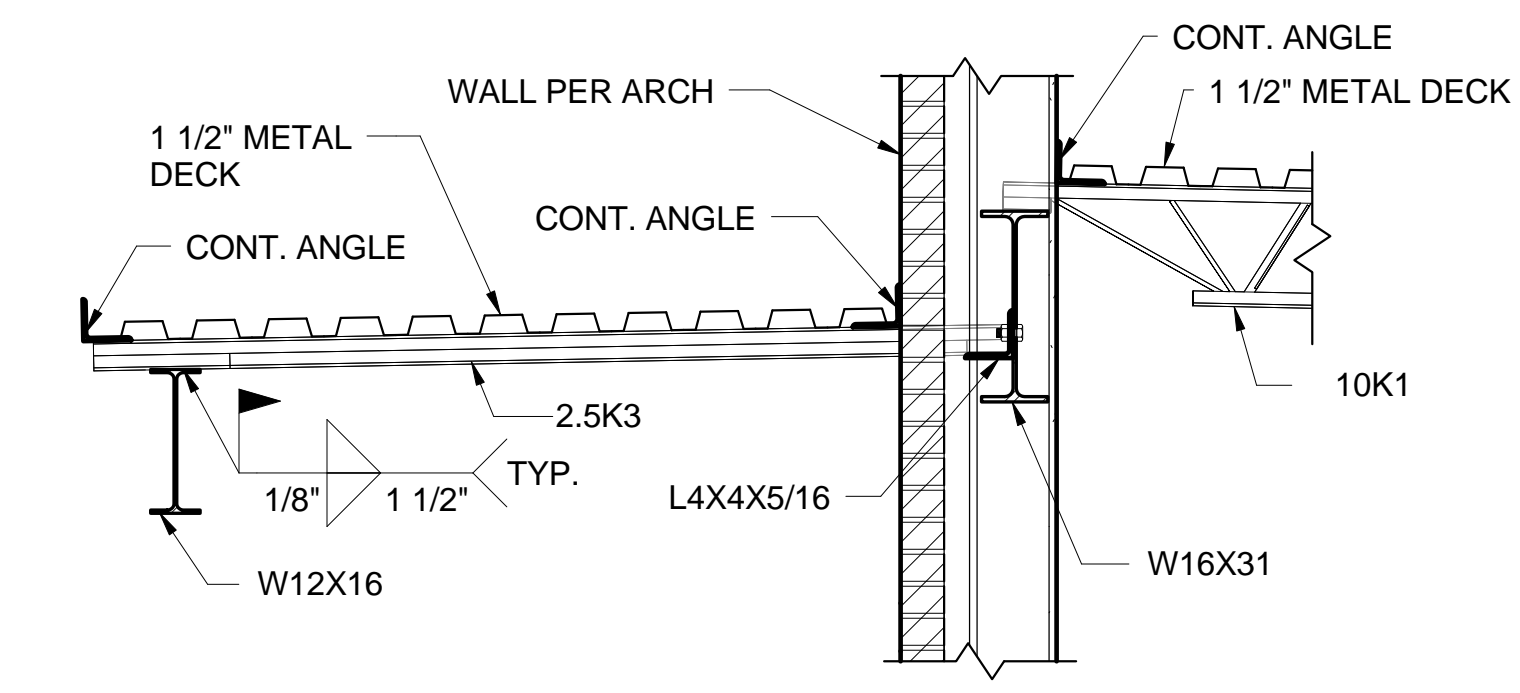
6 Roof Framing Section
 3/4" = 1'-0"



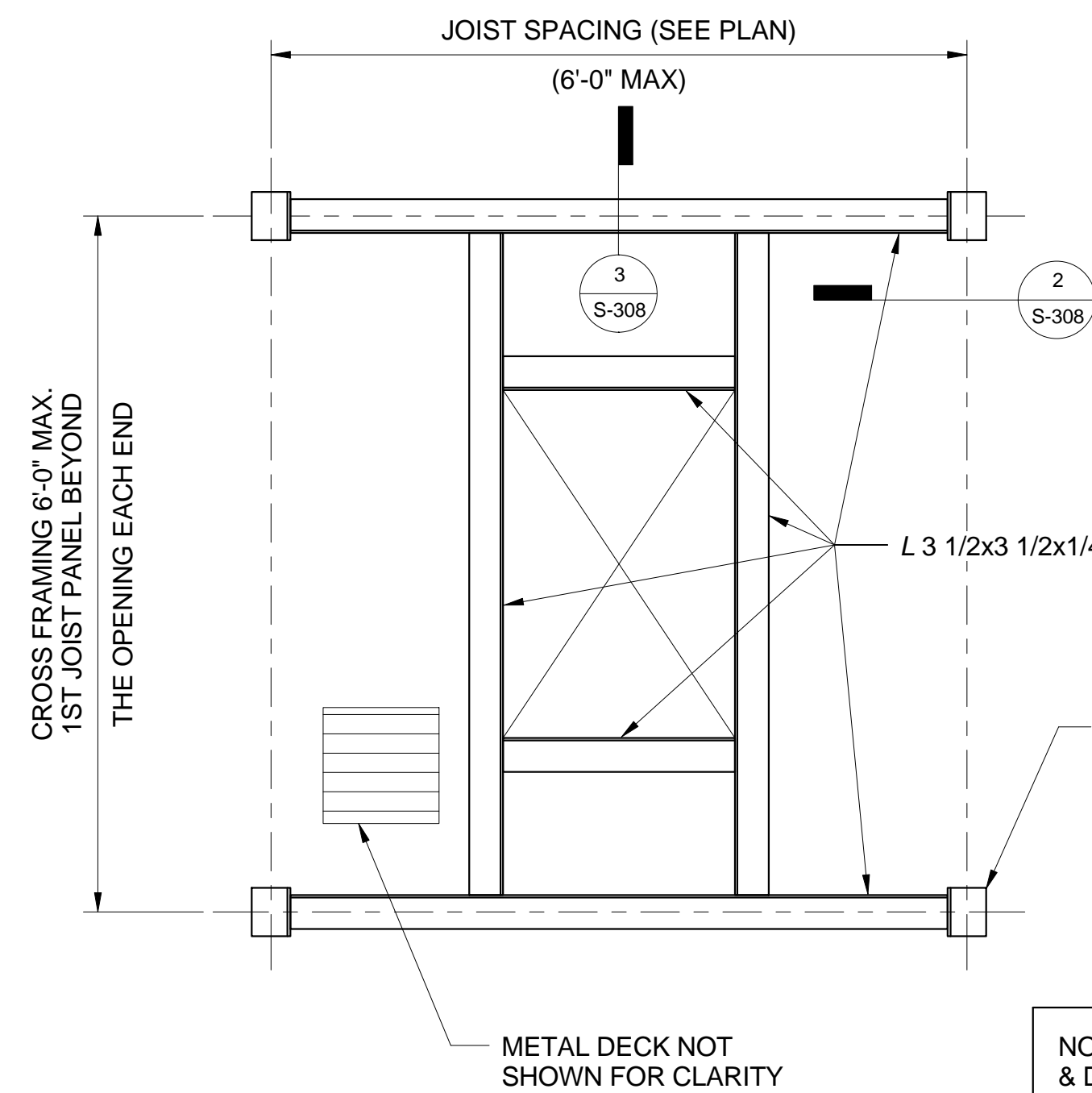
7 Roof Framing Section
 3/4" = 1'-0"



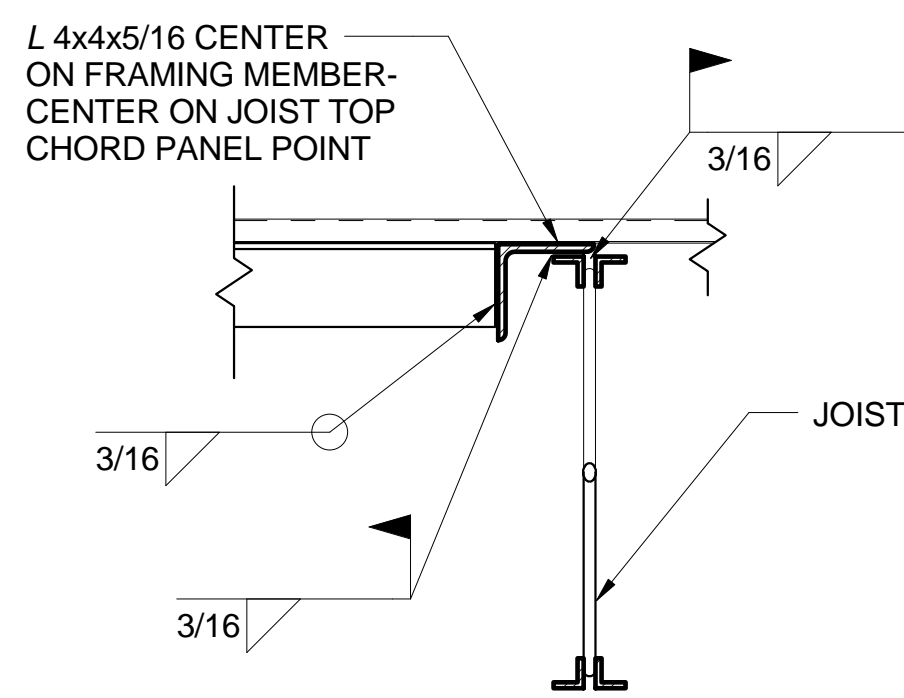
8 Roof Framing Section
 3/4" = 1'-0"



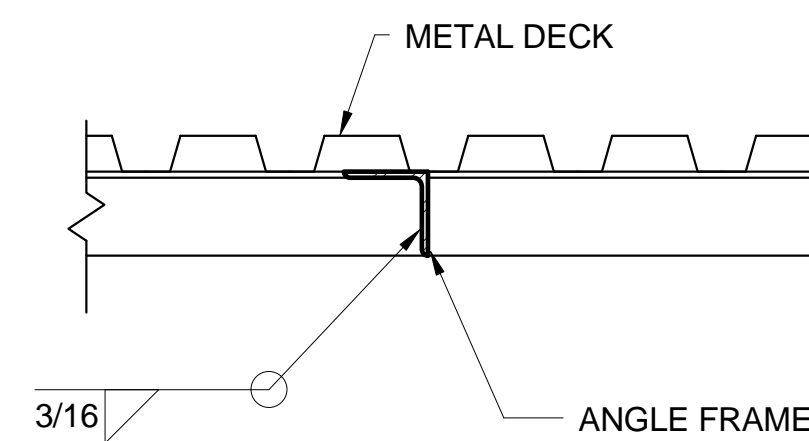
9 Roof Framing Section
 3/4" = 1'-0"



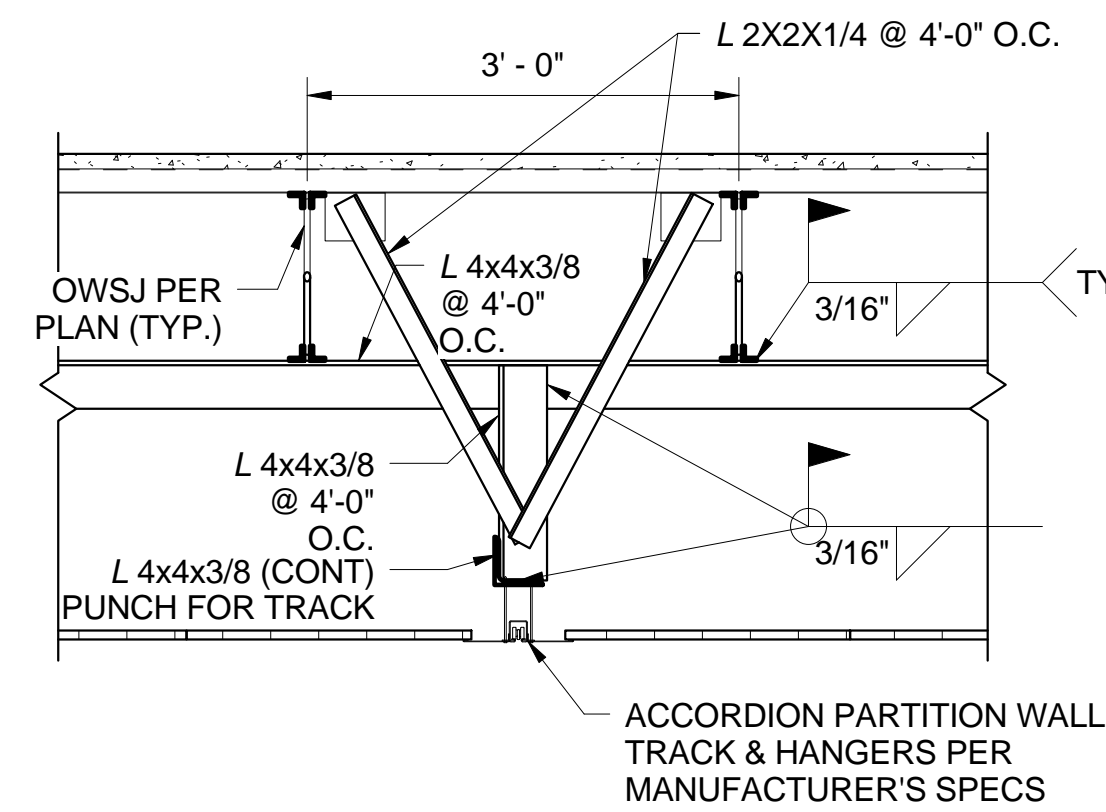
1 Typ. Reinf. For Roof/Floor Opening And RTU Support (TYP.)
3/4" = 1'-0"



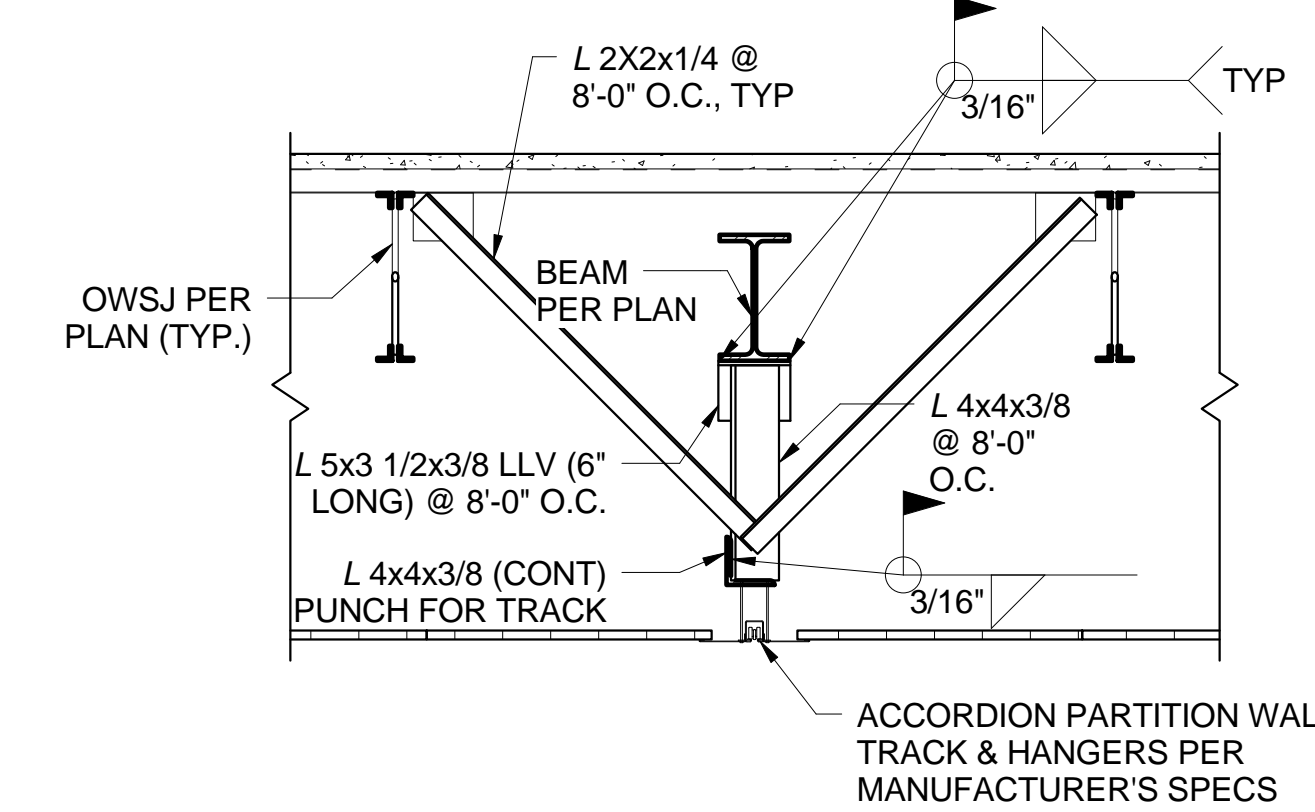
2 Section A
1 1/2" = 1'-0"



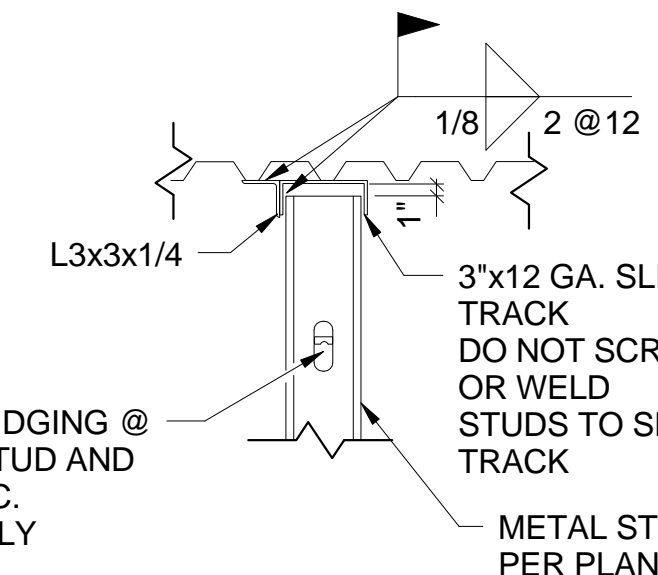
3 Section B
1 1/2" = 1'-0"



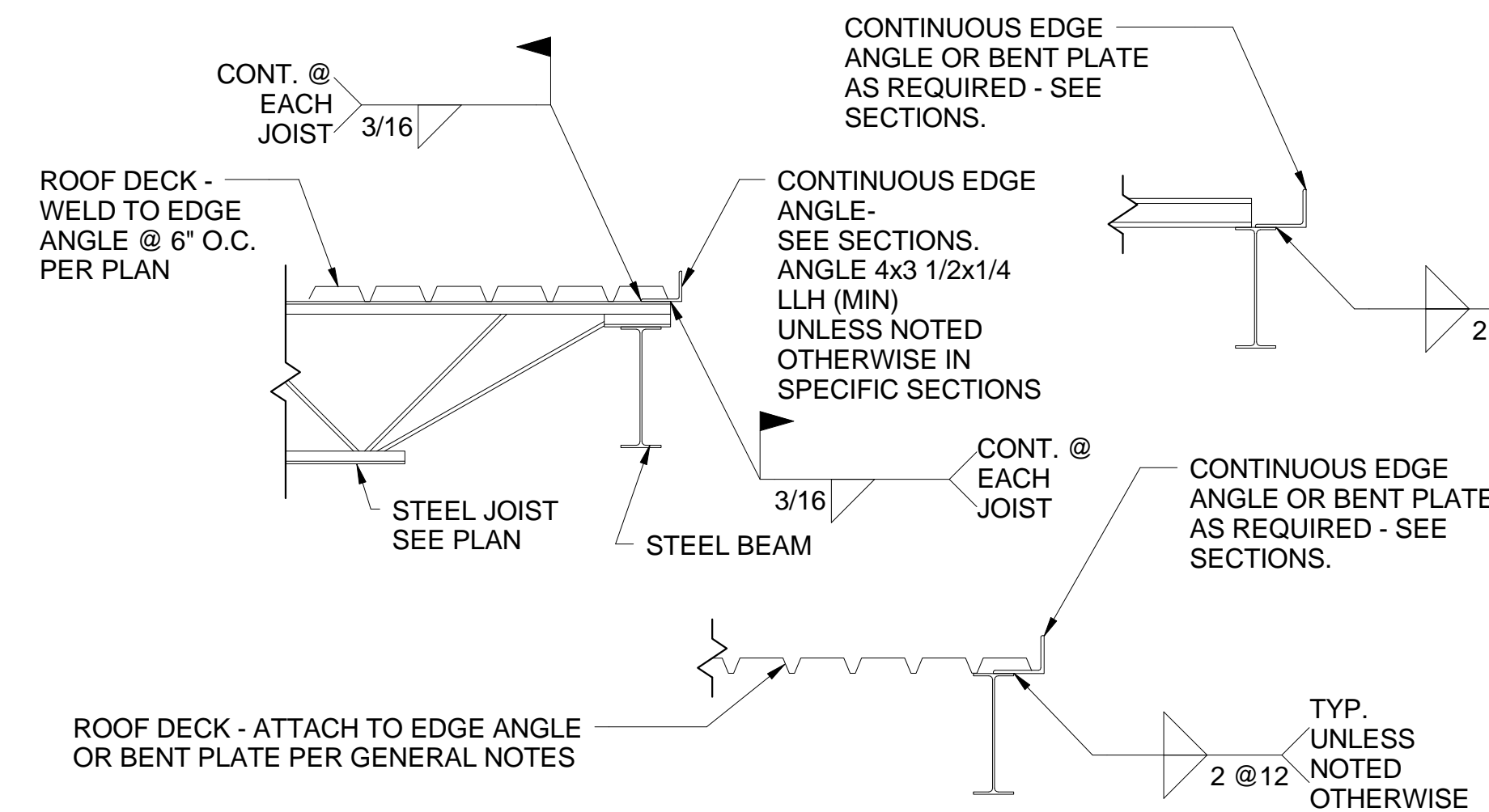
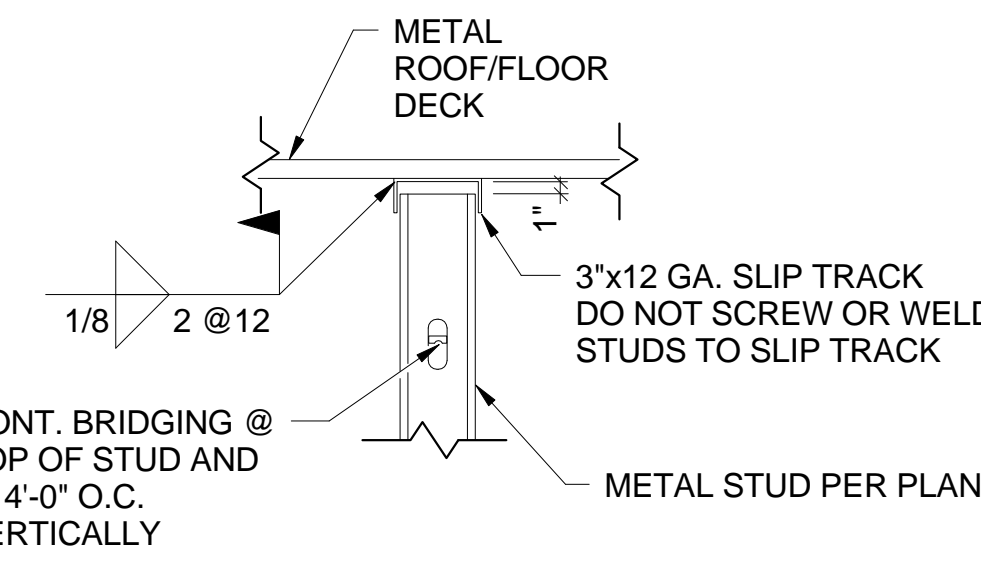
4 Typical Operable Partition Detail Supported by OWSJ
3/4" = 1'-0"



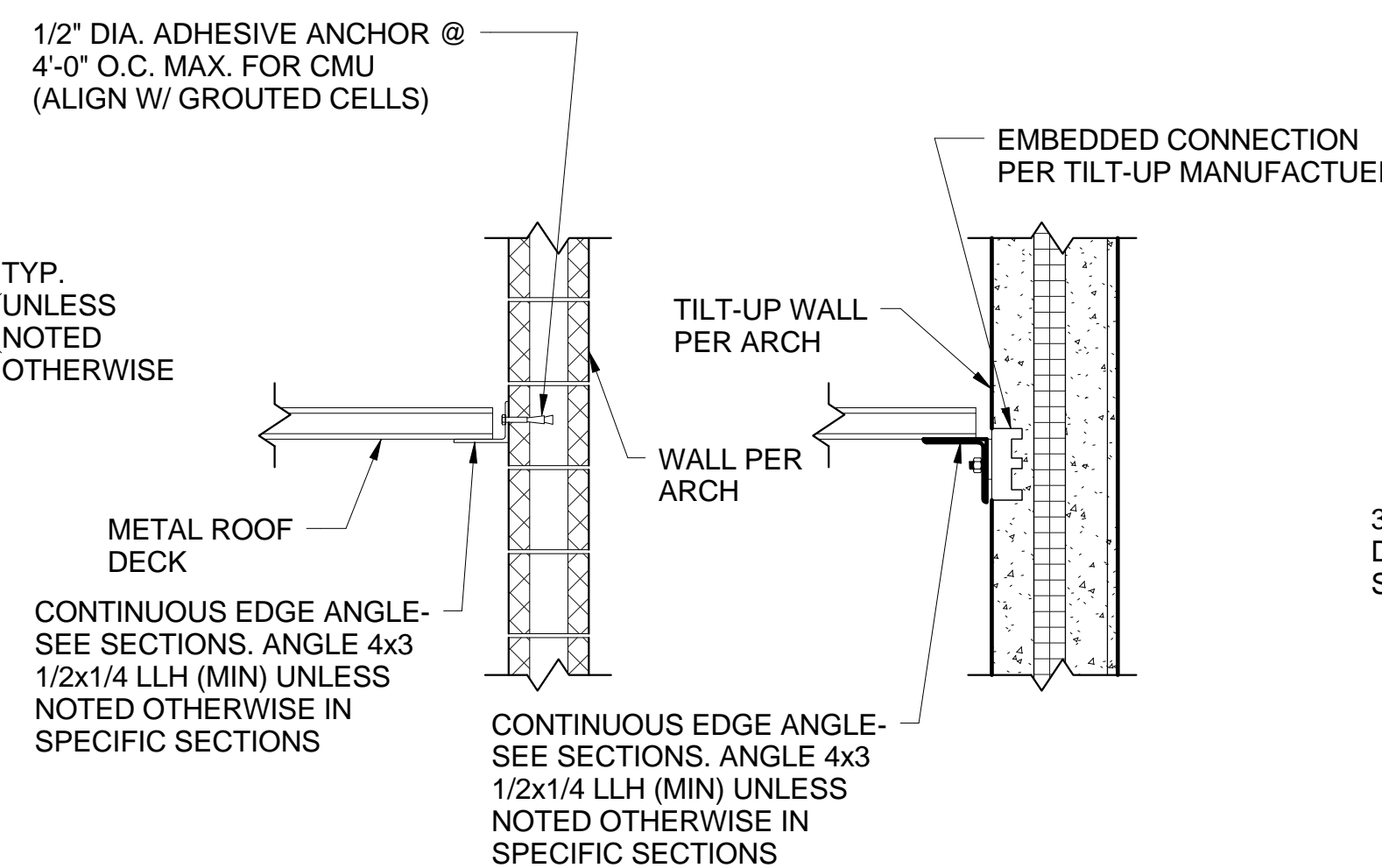
5 Typical Operable Partition Detail Supported by WF Beam
3/4" = 1'-0"



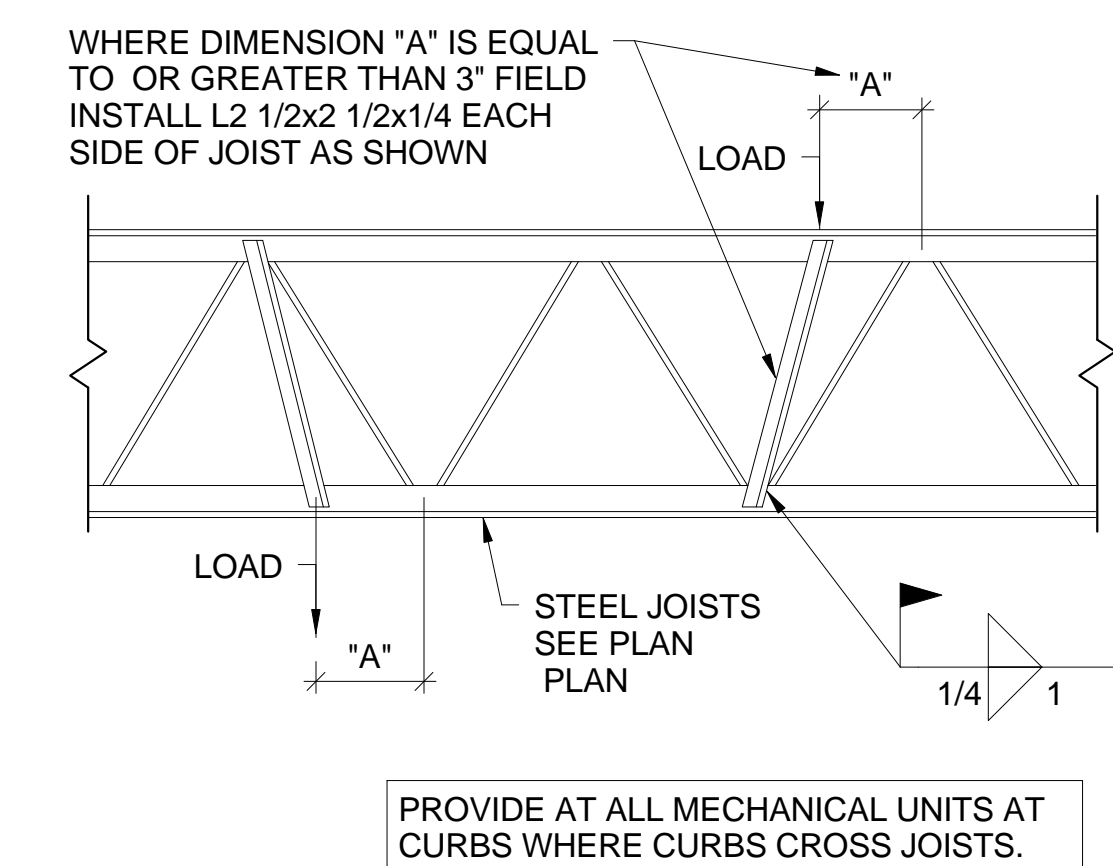
6 Typ. Detail - Metal Stud Wall Bracing @ Bottom of Metal Deck
3/4" = 1'-0"



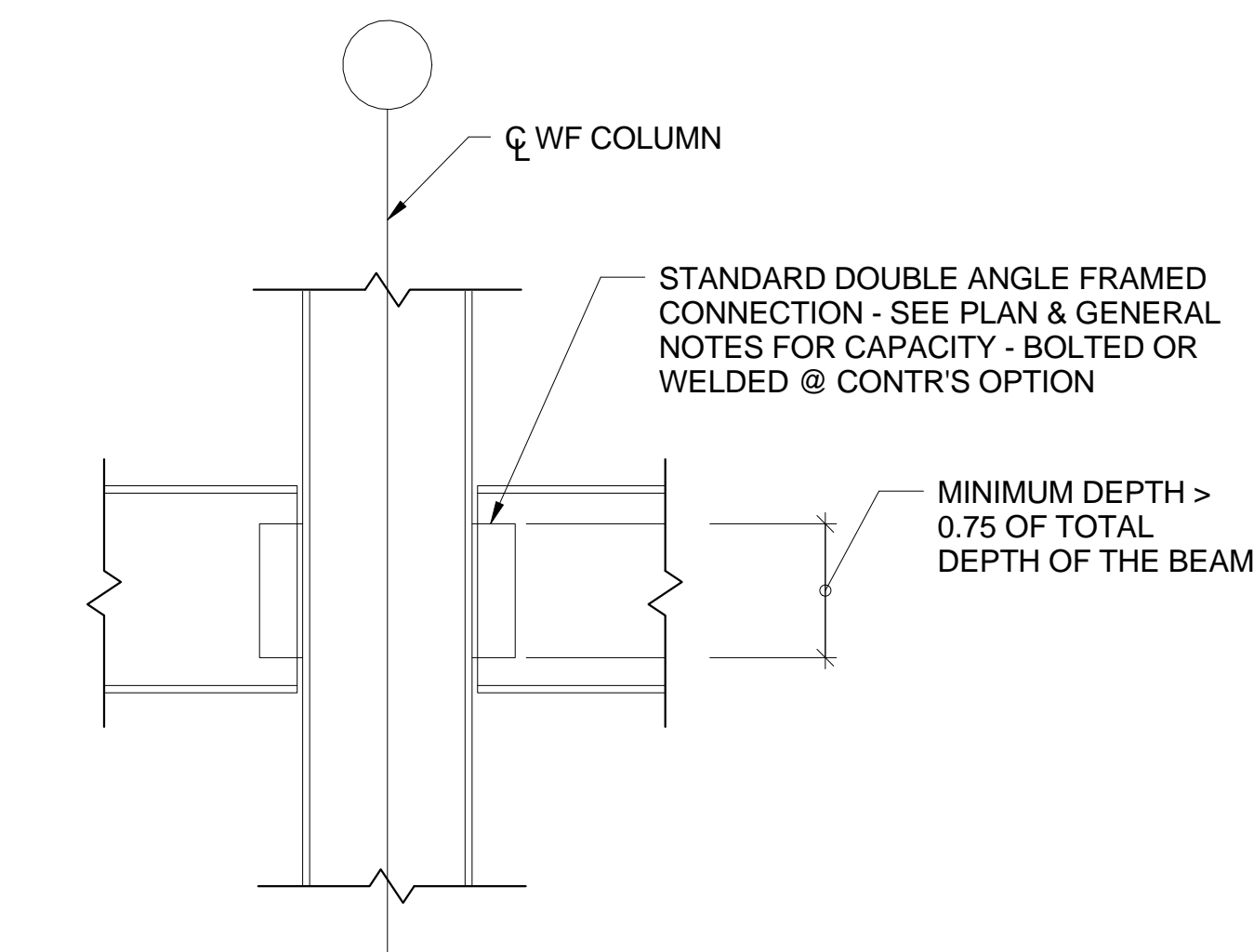
7 Typ. Framing Edge Details
3/4" = 1'-0"



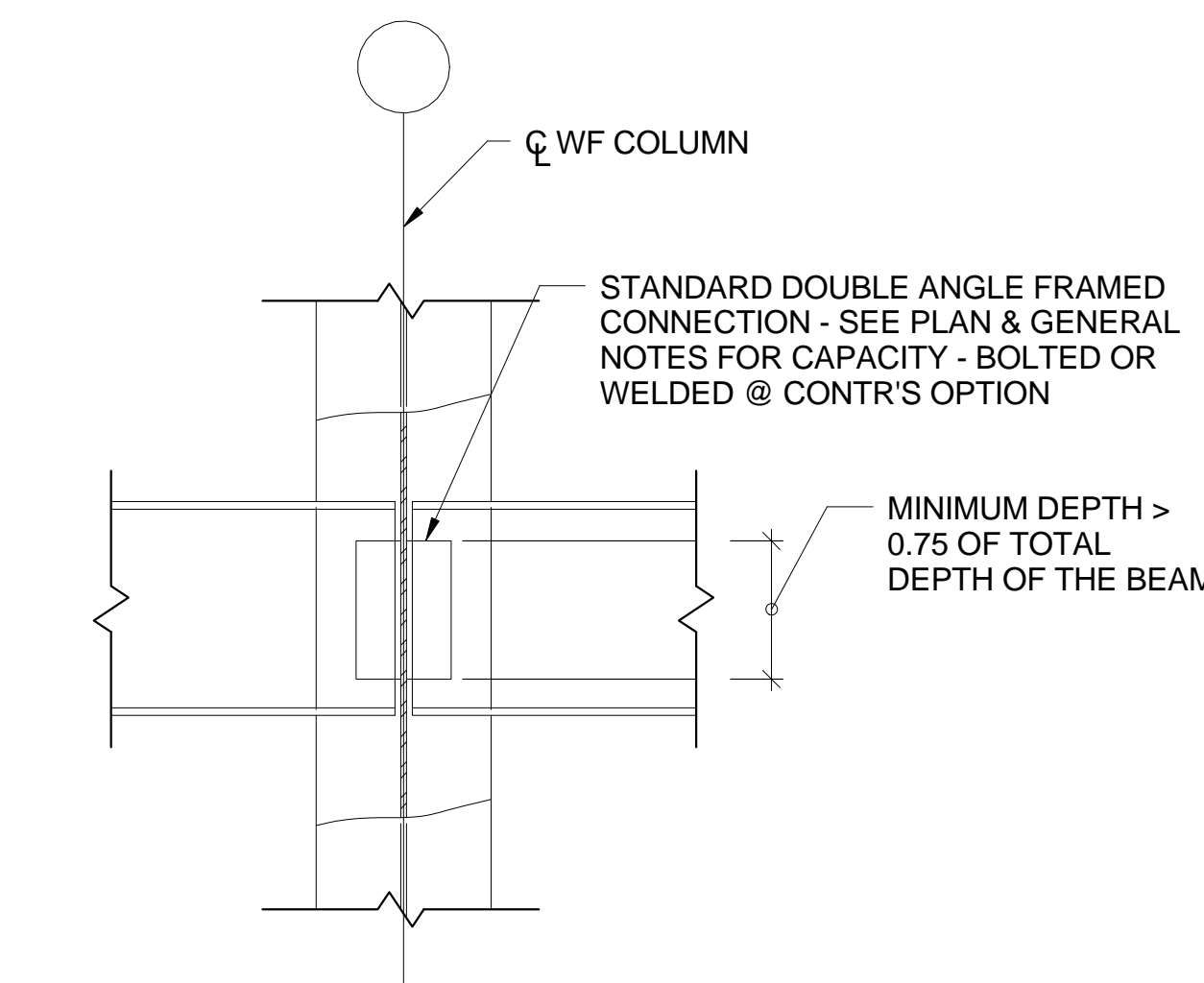
8 Typ. Detail - Metal Stud Wall Bracing @ Bottom of O.W.S.J.
3/4" = 1'-0"



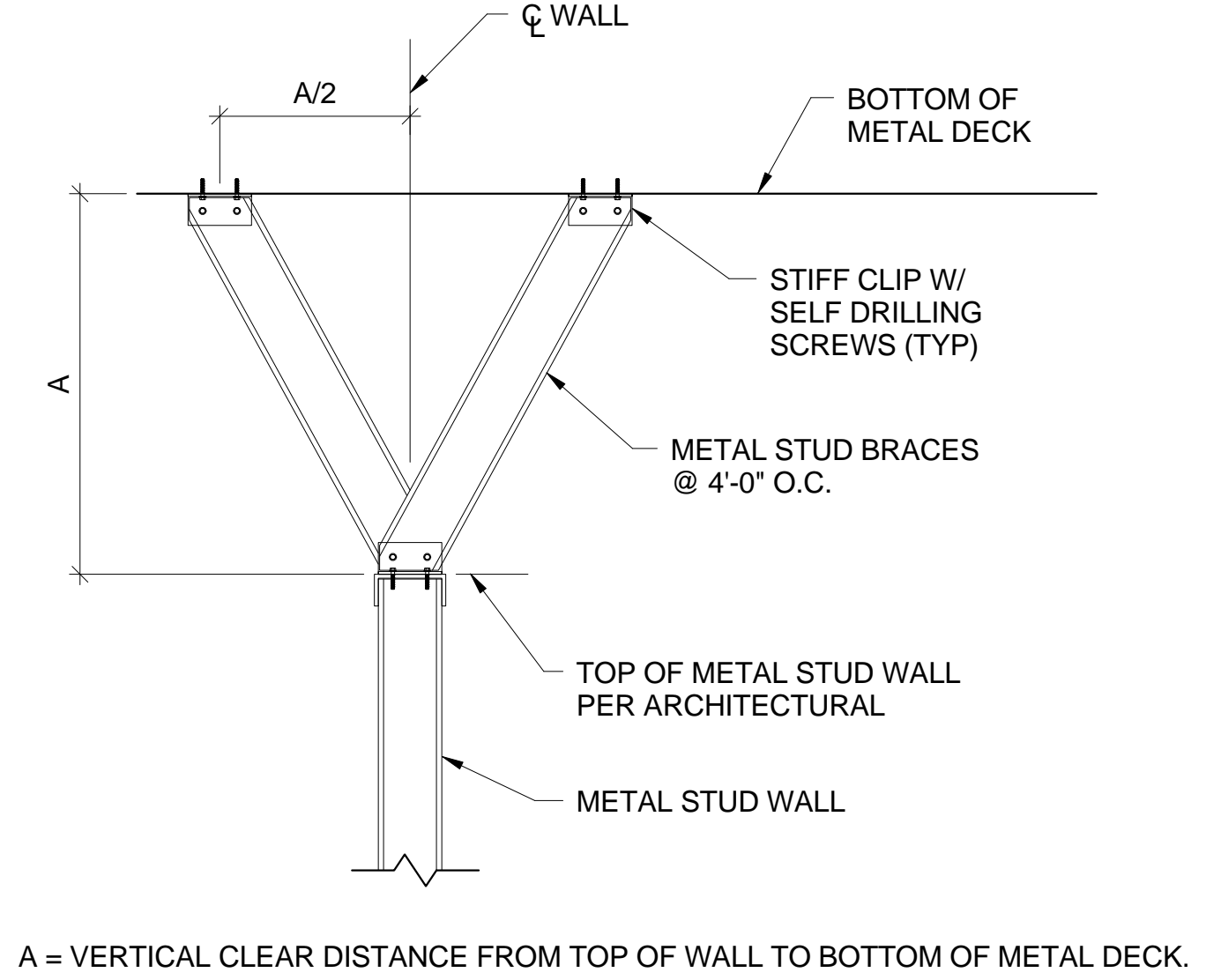
9 Typ. Joist Load Strut Detail
3/4" = 1'-0"



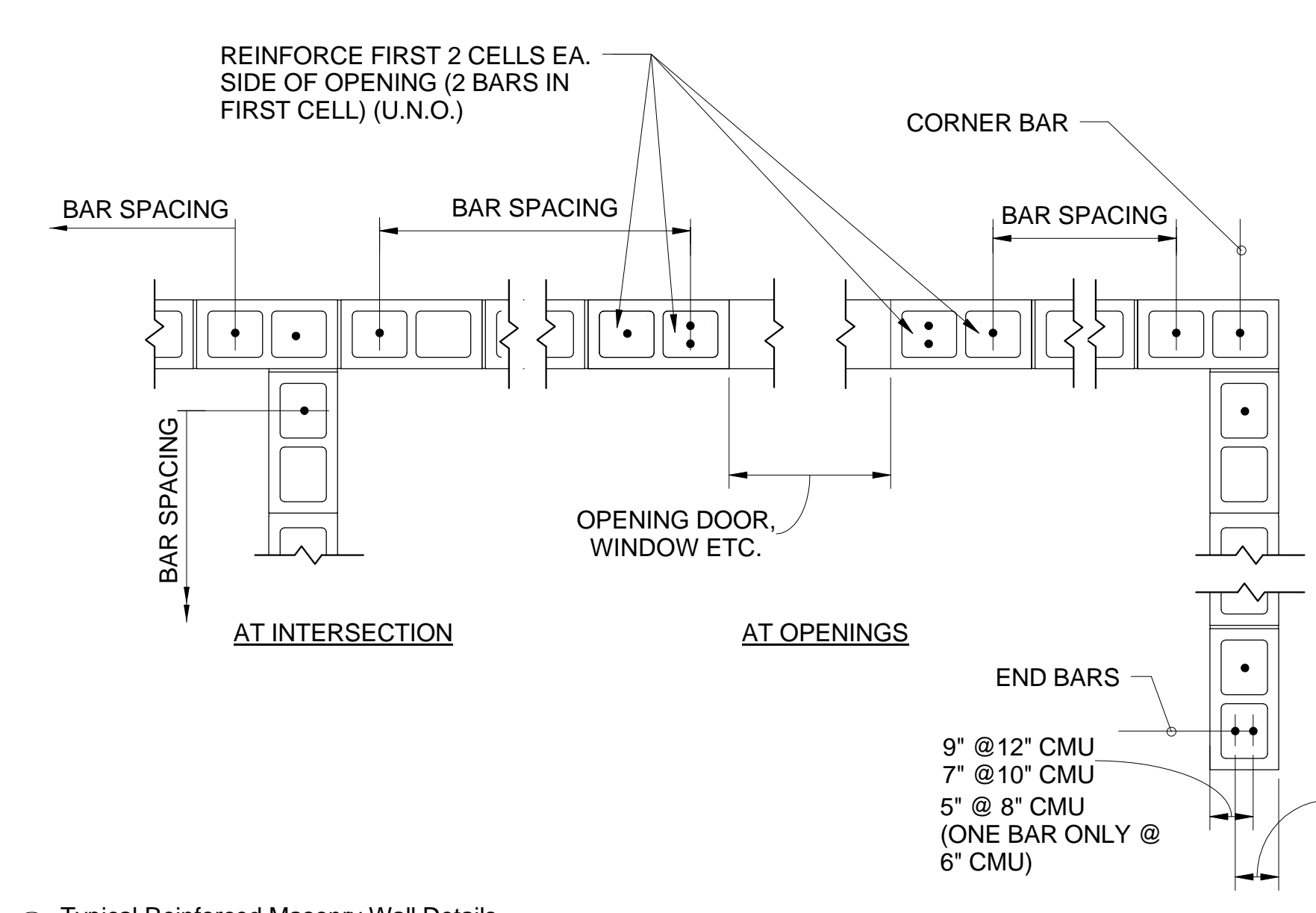
10 Typ. Beam to WF Column Shear Connection at Flange
3/4" = 1'-0"



11 Typ. Beam to WF Column Shear Connection at Web
3/4" = 1'-0"



12 Typ. Metal Stud Wall Bracing @ Top
3/4" = 1'-0"



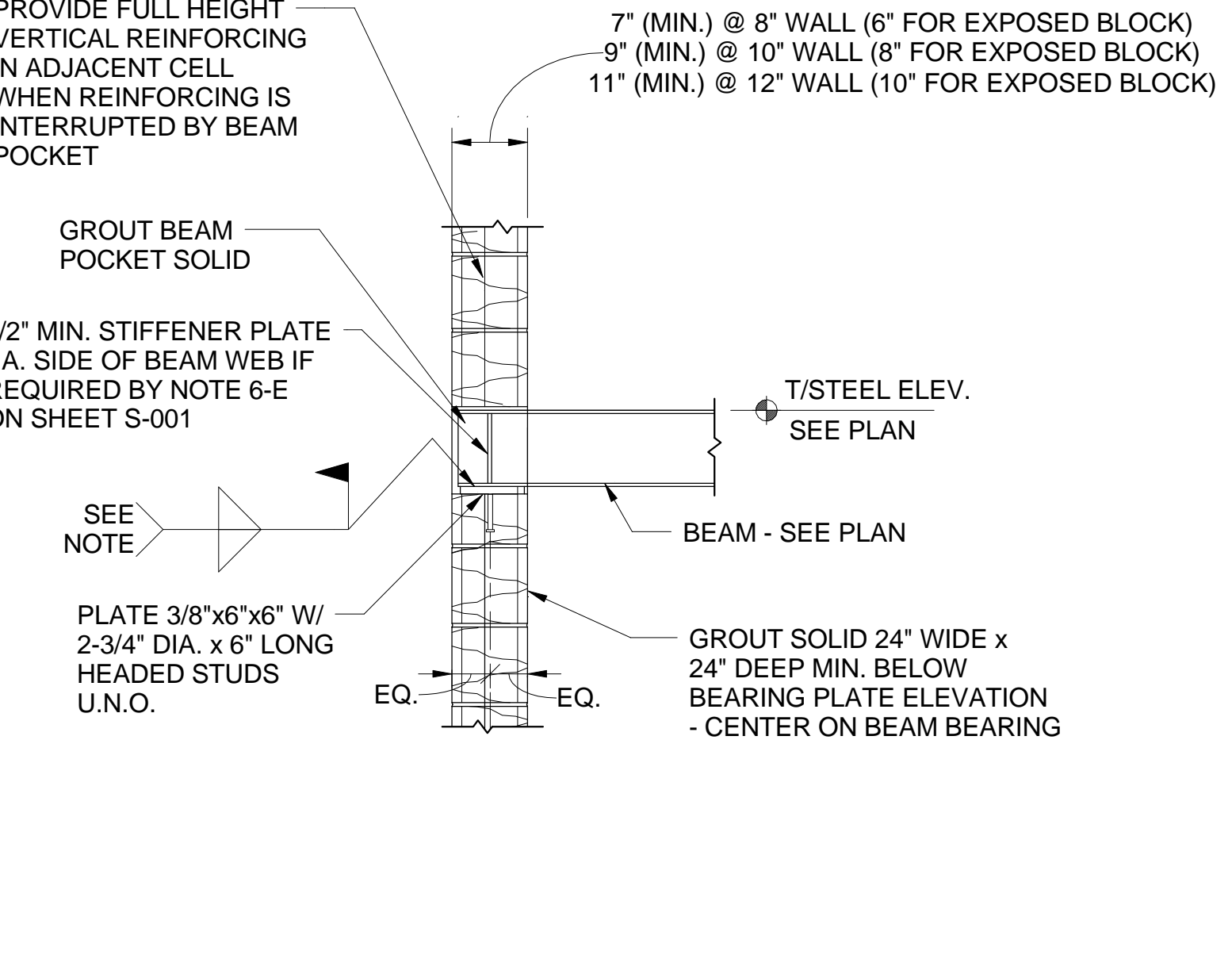
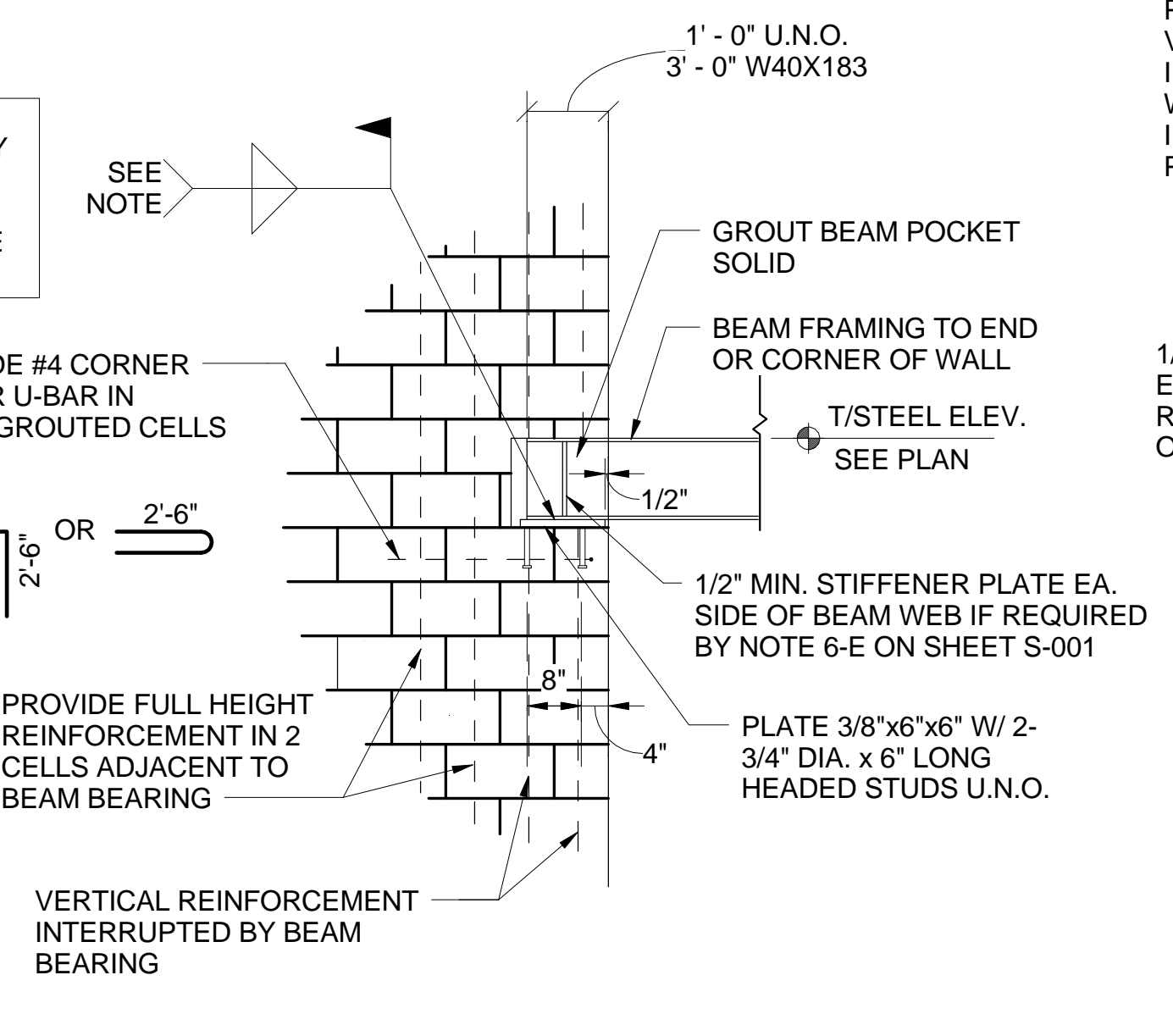
1 Typical Reinforced Masonry Wall Details
3/4" = 1'-0"

WF BEAM BEARING PLATE ON MASONRY

TYPE	WIDTH	LENGTH	# OF STUDS
W16X26	6"	8"	2
W18X35	8"	1'-0"	2
W40X183	10"	3'-0"	4
W10X33	8"	8"	2

NOTE: THIS TABLE SUPERSEDES BEARING PLATE DIMENSIONS ON DETAIL 2/S-401.

2 Typical WF Beam Bearing Detail
1/2" = 1'-0"



BOND BEAM REINFORCING SCHEDULE

LOCATION	REINFORCING
STANDARD	2 - #5 CONTINUOUS

NOTES TO BOND BEAM REINFORCING SCHEDULE:

- PROVIDE BOND BEAMS IN ALL CMU WALLS AT THE FOLLOWING LOCATIONS U.N.O.:
 - AT THE TOP OF ALL WALLS
 - AT ALL FLOOR AND ROOF ATTACHMENT LOCATIONS
 - AT THE BOTTOM OF ALL WALL OPENINGS, EXTENDING 2 FEET (MIN.) PAST OPENING EACH SIDE.
 - AT ADDITIONAL LOCATIONS SHOWN ON DRAWINGS.
 - AT BEARING LOCATIONS.

REINFORCED MASONRY WALL NOTES:

- ALL WORK SHALL CONFORM TO RECOMMENDATIONS OF ACI 530 AND NCMA.
- MASONRY DESIGN STRENGTH: NET AREA COMPRESSIVE STRENGTH: $f_m = 2000$ psi
GROUT STRENGTH: MORTAR TYPE: 2500 psi (MIN.) S
- REINFORCED MASONRY SHALL INCLUDE CMU.
- SUBMIT REINFORCED MASONRY SHOP DRAWINGS FOR APPROVAL FOR ALL REINFORCED MASONRY. INCLUDE PLAN VIEW, ELEVATION VIEW, AND SECTION VIEW OF ALL REINFORCED MASONRY WALLS THAT ARE SHOWN ON THE STRUCTURAL DRAWINGS. INCLUDE LAYOUT OF ALL REINFORCEMENT ON SECTIONS AND ELEVATIONS.
- TIE BARS FROM CAST-IN-PLACE CONCRETE SHALL MATCH THE VERTICAL REINFORCEMENT IN THE WALL ABOVE UNLESS NOTED OTHERWISE. SUCH TIE BARS SHALL BE FURNISHED BY THE CONCRETE CONTRACTOR.
- REINFORCING ENTIRELY WITHIN THE MASONRY SHALL BE FURNISHED BY THE MASONRY CONTRACTOR.
- WHEN A FOUNDATION TIE DOES NOT LINE UP WITH A VERTICAL BLOCK CORE, IT SHALL NOT BE SLOPED MORE THAN ONE HORIZONTAL IN 6 VERTICAL.
- REINFORCING BARS SHALL HAVE A MINIMUM CLEARANCE OF 3/4 OF AN INCH FROM THE MASONRY AND 1 INCH TO ADJACENT BARS AND NOT LESS THAN ONE BAR DIAMETER BETWEEN BARS NOT SPLICED. BARS WITHIN THE SAME CELL SHALL BE PLACED AS FAR APART AS PRACTICAL.
- REINFORCING STEEL SHALL BE SECURELY IN PLACE AND INSPECTED BEFORE GROUTING STARTS. REINFORCING BAR POSITIONERS SHALL BE USED AT THE TOP, BOTTOM, AND MIDDLE OF EACH WALL AS A MINIMUM.
- GROUTING SHALL BE STOPPED 1 1/2" BELOW THE TOP OF A COURSE AND 1/2" BELOW THE TOP OF A BOND BEAM, SO AS TO FORM A KEY AT THE POUR JOINT.
- GROUTING OF MASONRY BEAMS OVER OPENINGS SHALL BE DONE IN ONE CONTINUOUS OPERATION.
- REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS AND DETAILS OF DOOR AND WINDOW OPENINGS FOR SPECIAL COURSING AND OTHER MASONRY DETAILS. THE INFORMATION SHOWN ON THE STRUCTURAL DRAWINGS IS INTENDED TO DEFINE THE STRUCTURAL REQUIREMENTS ONLY.
- ALL MASONRY WALLS TO BE REINFORCED HORIZONTALLY AND VERTICALLY IN ACCORDANCE WITH DRAWINGS AND SCHEDULE.
- ALL REINFORCED CELLS SHALL BE GROUTED FULL.
- ALL HORIZONTAL REINFORCEMENT SHALL BE CONTINUOUS AT CORNERS OF WALL AND AT ALL WALL INTERSECTIONS. HORIZONTAL REINFORCEMENT SHALL END WITH STANDARD HOOK ANCHORED AROUND VERTICAL REINFORCING BARS.
- ALL WALL OPENINGS SHALL BE REINFORCED WITH A MINIMUM OF 2-#5 BAR ON ALL SIDES OF OPENING. REINFORCEMENT SHALL RUN CONTINUOUS FROM FLOOR TO TOP SUPPORT IN GROUTED CELLS.
- ALL INTERIOR WALLS TO BE BONDED BY TOOTHING OR KEYING INTERSECTING WALLS.
- MASONRY VENEER ANCHORING - ATTACH VENEER TO BACKUP MASONRY WITH "SEISMICLIP" INTERLOCK SYSTEM. PROVIDE A MINIMUM OF ONE ANCHOR FOR EVERY 1.77 SQ. FT. OF WALL WITH A 16" MAXIMUM VERTICAL AND HORIZONTAL SPACING DESIGNED TO RESIST THE LOAD OF 45 PSF.
- VERTICAL REINFORCEMENT TO BE CONTINUOUS THRU BOND BEAMS.

REINFORCED MASONRY LINTEL SCHEDULE

LINTEL	MAX. CLEAR SPAN	DEPTH	REINFORCING	SHEAR REINF.	MIN. BRG.	RE. DETAIL
L-1	4'-6"	16"	1 - #5 BAR	-	8"	3/S-401
L-2	12'-0"	24"	2 - #5 BAR	-	8"	3/S-401
L-3	-	40"	2 - #7 BAR	-	8"	3/S-401

NOTES TO REINFORCED MASONRY LINTEL SCHEDULE:

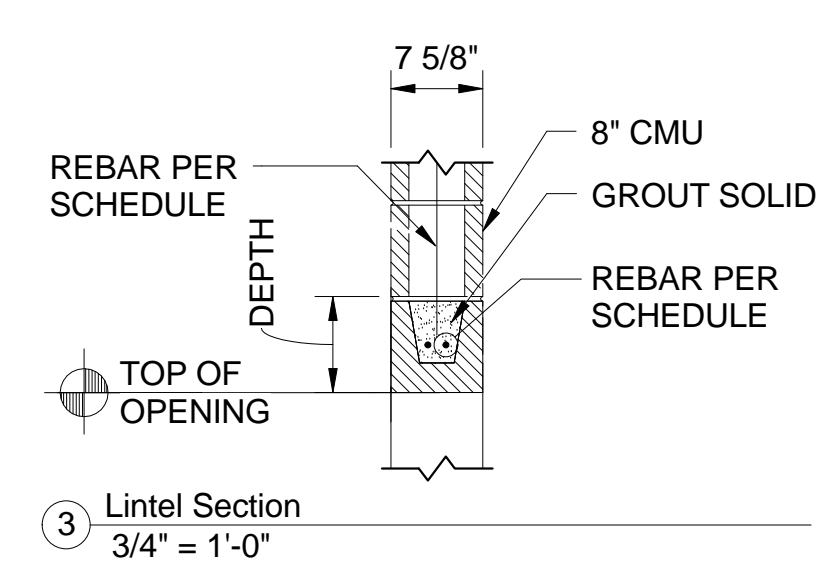
- EXTEND REINFORCING 24" EACH SIDE OF OPENING (MINIMUM). WHERE SPACE IS NOT AVAILABLE DUE TO CORNERS, ETC., PROVIDE 18" HOOK UPWARD INTO CELL ADJACENT TO OPENING.
- ALL CELLS SHALL BE FULLY GROUTED.
- CONTROL JOINTS ARE NOT NECESSARY. IF USED, HORIZONTAL REINFORCEMENT SHALL PASS THRU JOINT.
- WHERE LINTEL MARK IS NOT PROVIDED, PROVIDE LINTEL PER MAX. CLEAR SPAN LISTED IN TABLE.

STEEL LINTEL SCHEDULE

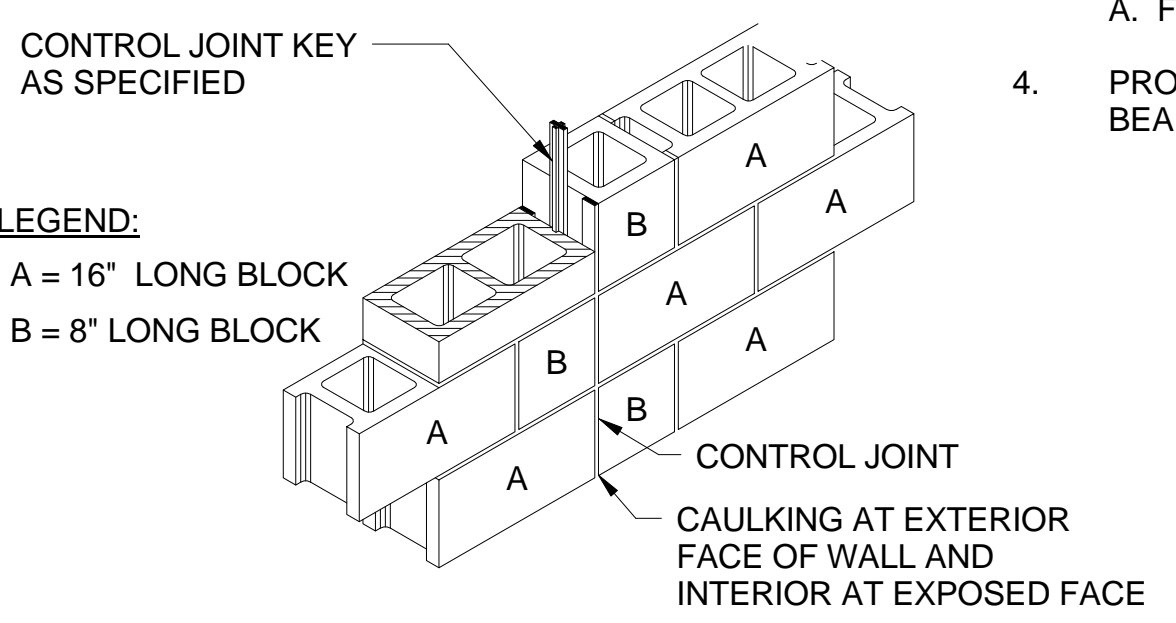
MAX. MASONRY OPENING	NOMINAL WALL THICKNESS		MIN. BRG.	REMARKS
	4" (BRICK)	8"		
5'-0"	1 - L4x4x5/16	2 - L4x3 1/2x1/4	8"	SEE NOTES 1-5 (TYP)
6'-0"	1 - L5x4x5/16	2 - L5x3 1/2x1/4	8"	
8'-0"	1 - L6x4x5/16	2 - L6x3 1/2x1/4	8"	
10'-0"	1 - L6x4x5/16	2 - L6x3 1/2x5/16	8"	
12'-0"	1 - L7x4x3/8	2 - L7x4x3/8	8"	

NOTES TO STEEL LINTEL SCHEDULE:

- THIS SCHEDULE APPLIES WHEN THE LINTEL OCCURS IN A NON-LOAD BEARING WALL OR WHEN DISTANCE TO STRUCTURAL BEARING ABOVE HEAD OF OPENING EXCEEDS 1/2 OF THE OPENING WIDTH.
- INSTALL LINTELS LONG LEG VERTICAL.
- DIMENSION OF VERTICAL LEG IS MIN., VERTICAL LEG MAY BE LONGER AT CONTRACTOR'S OPTION.
- WHERE LINTELS OCCUR IN EXTERIOR WALLS, MIN. THICKNESS SHALL BE 5/16" AND LINTEL SHALL BE HOT DIP GALVANIZED.
- USE LINTELS SPECIFIED UNDER 4" NOMINAL WALL THICKNESS FOR BRICK LINTELS UNLESS SPECIFIED OTHERWISE ON THESE PLANS.



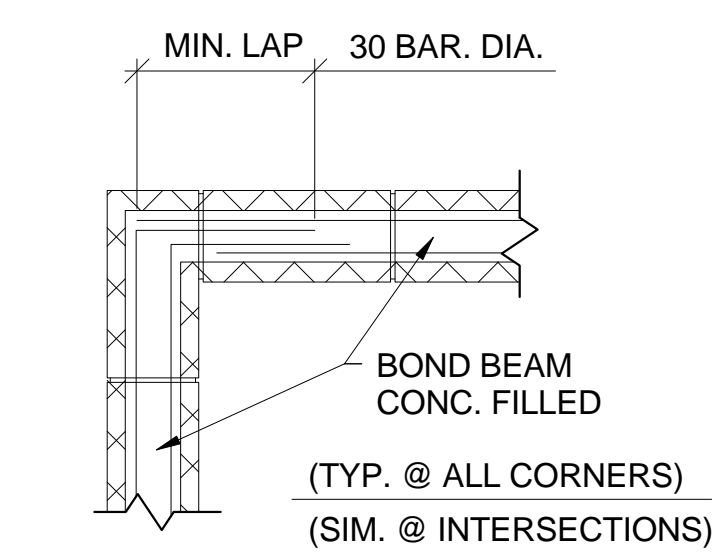
3 Lintel Section
3/4" = 1'-0"



- NOTES:**
- FIGURE ABOVE IS APPLICABLE TO THE CONSTRUCTION OF ALL CONTROL JOINTS IN 8" & 10" HOLLOW BLOCK WALLS.
 - MAXIMUM CONTROL JOINT SPACING = 50'.

4 Basic Control Jt. for Single WYTHE CMU Masonry Walls
1/2" = 1'-0"

- 6" CMU-----#4 @48"
8" CMU-----#4 @48"
10" CMU-----#5 @48"
12" CMU-----#5 @48"



5 Typical Bond Beam Corner Detail
3/4" = 1'-0"

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ENGINEERS INC.
PROFESSIONAL DESIGN NUMBER: 184-00028

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CHASE & CONNOR
081-00700
HILLSBORO ILLINOIS

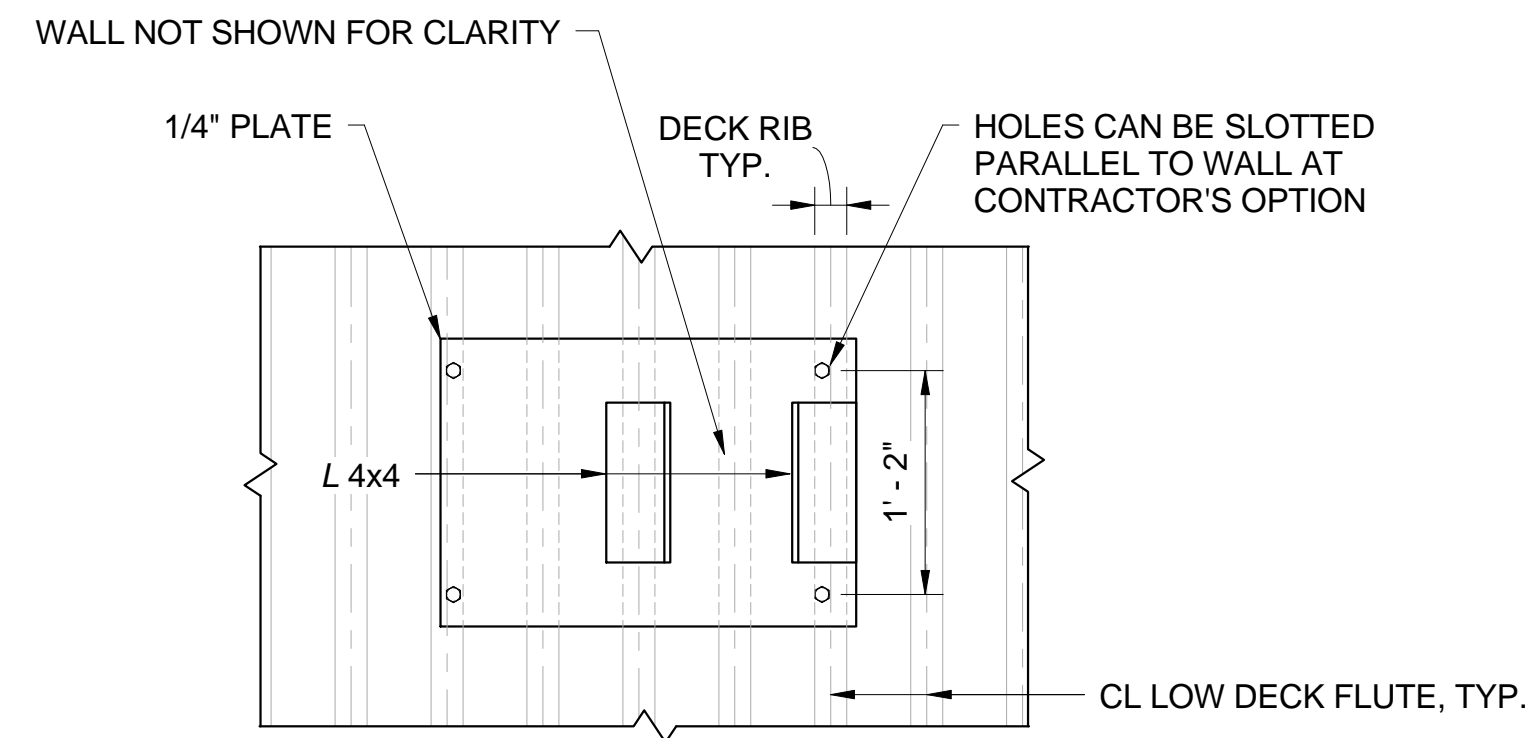
Chase Connor
SIGNATURE
DATE: 03/19/15
11-30-2016
LICENSE EXPIRES

PHASE 6 - FINE ARTS BUILDING ADDITION
NEW MARION HIGH SCHOOL
MARION C.U.S.D. #2
MARION, WILLIAMSON COUNTY, ILLINOIS

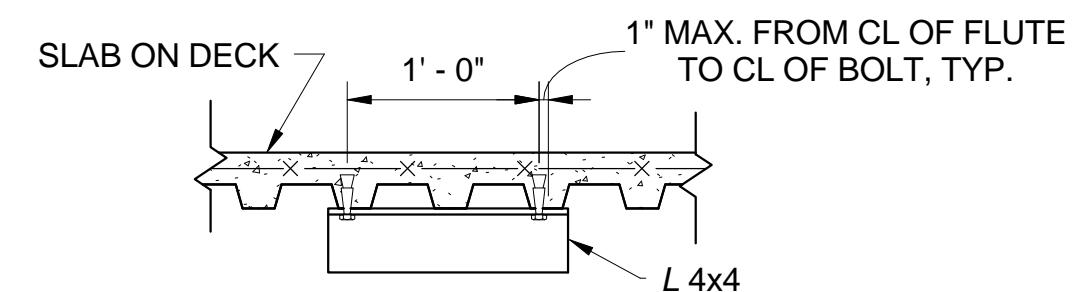
Mark	Date	Description

DATE: 03/19/15
PROJECT NO: 360-2632
DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

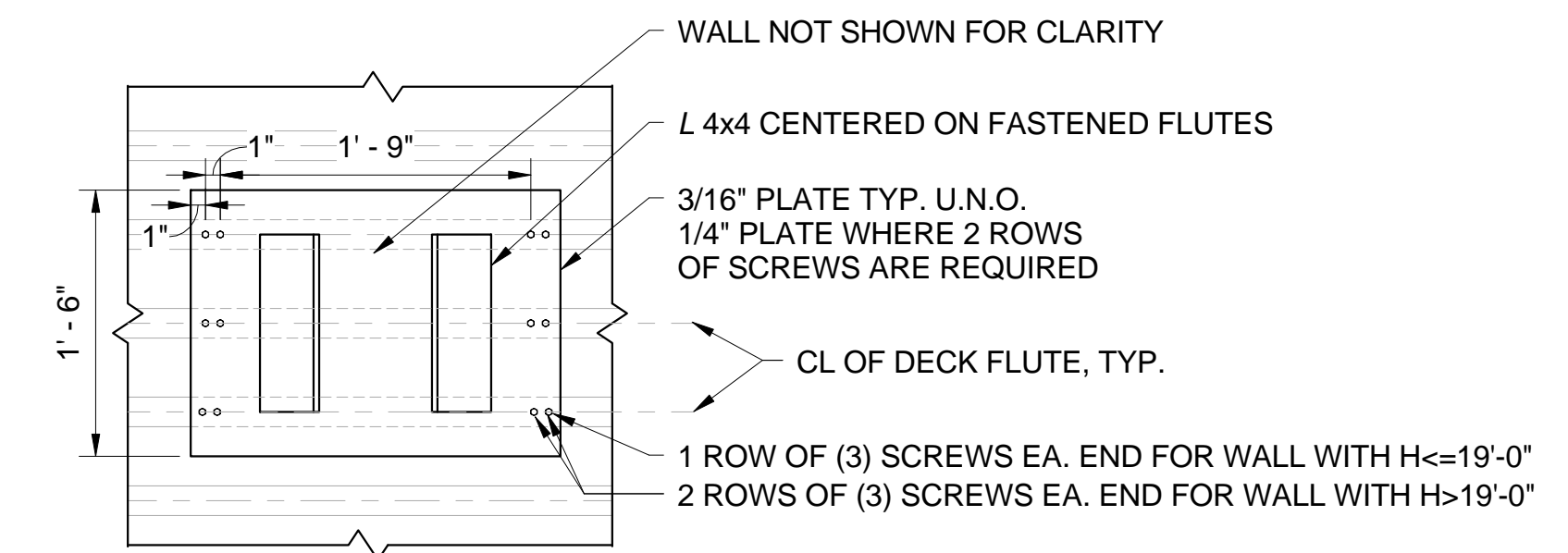
MASONRY DETAILS



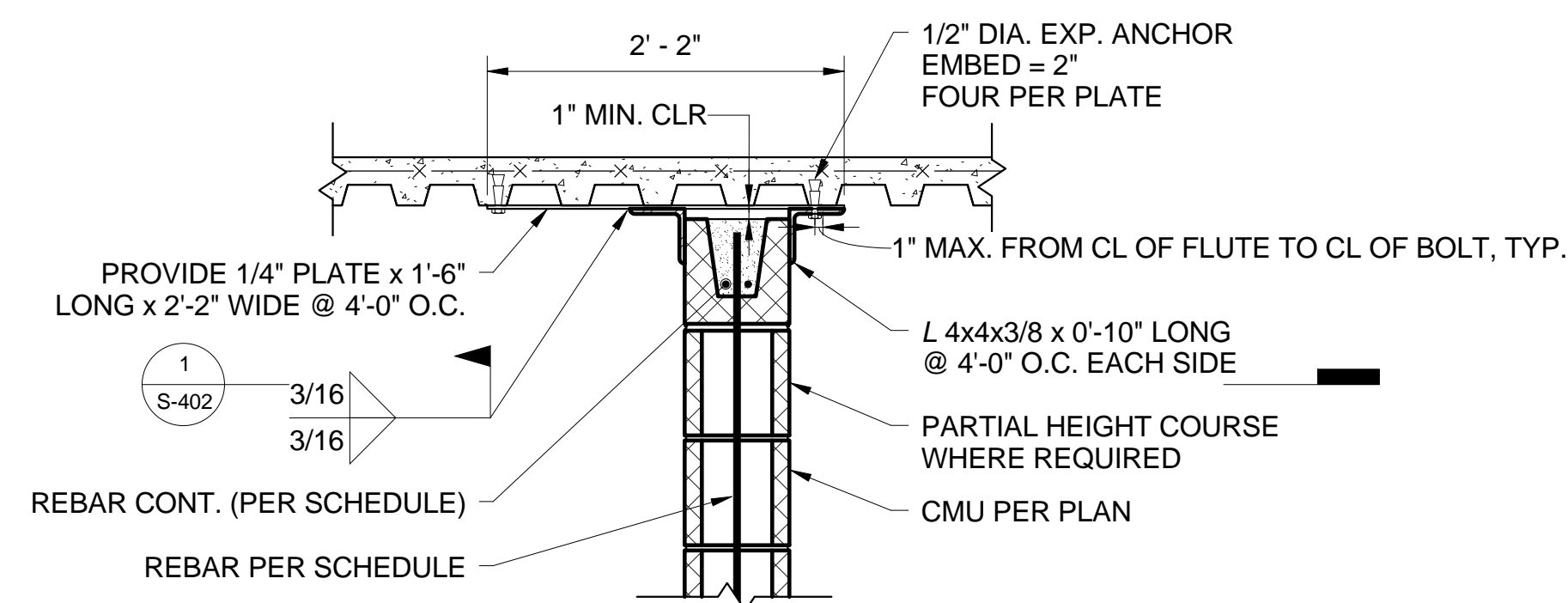
1 Typical Detail
1" = 1'-0"



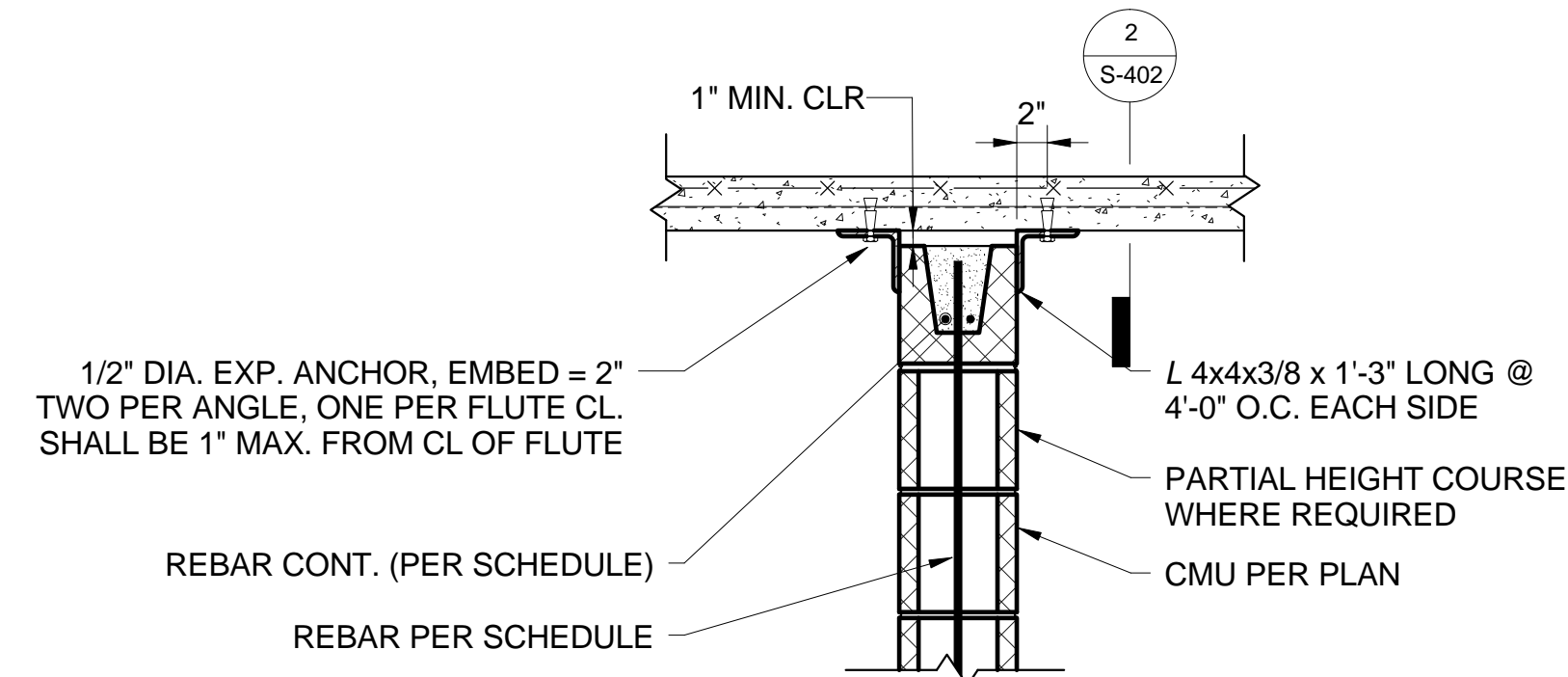
2 Typical Detail
1" = 1'-0"



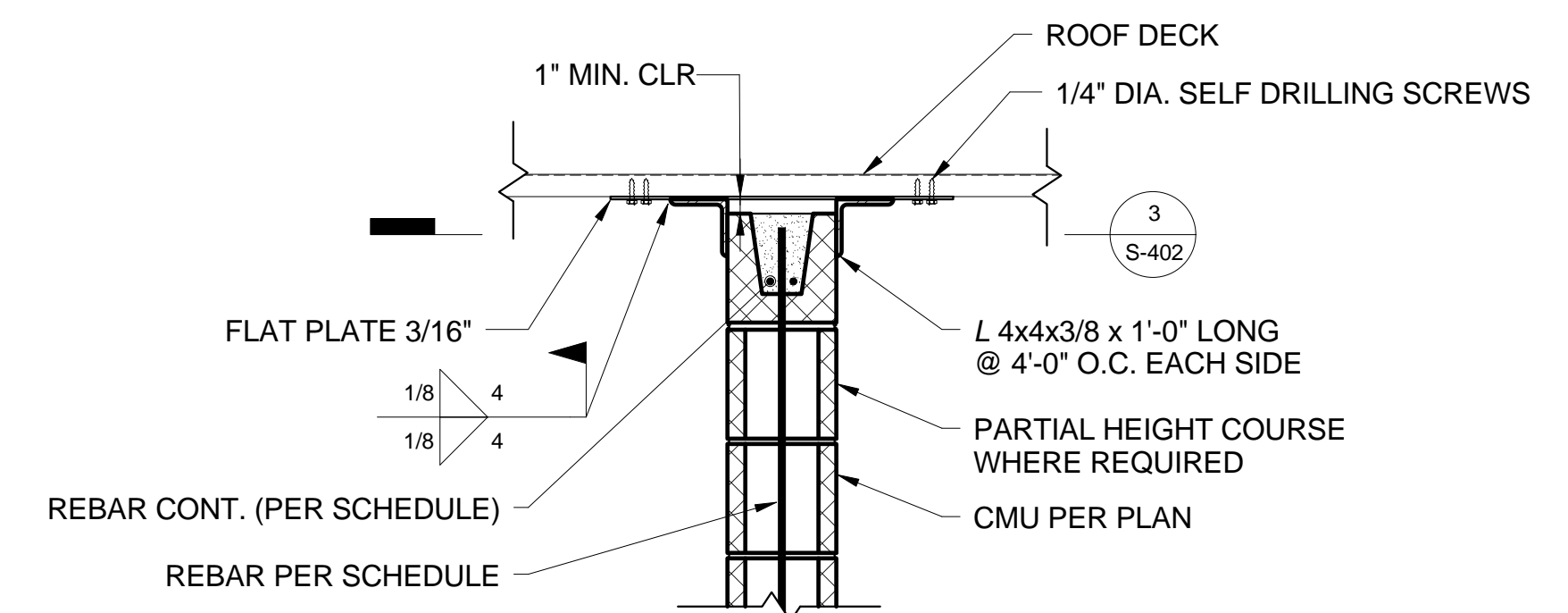
3 Typical Detail
1" = 1'-0"



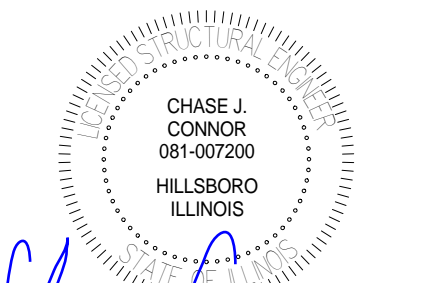
4 Typ. Detail - CMU Wall Bracing to Underside of Slab on Deck - Flutes Parallel
1" = 1'-0"



5 Typ. Detail - CMU Wall Bracing to Underside of Slab on Deck - Flutes Perpendicular
1" = 1'-0"



6 Typ. Detail - CMU Wall Bracing to Underside of Roof Deck - Flutes Perpendicular (Parallel, Sim.)
1" = 1'-0"



Chase Connor
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03/19/15

DATE

11-30-2016

LICENSE EXPIRES

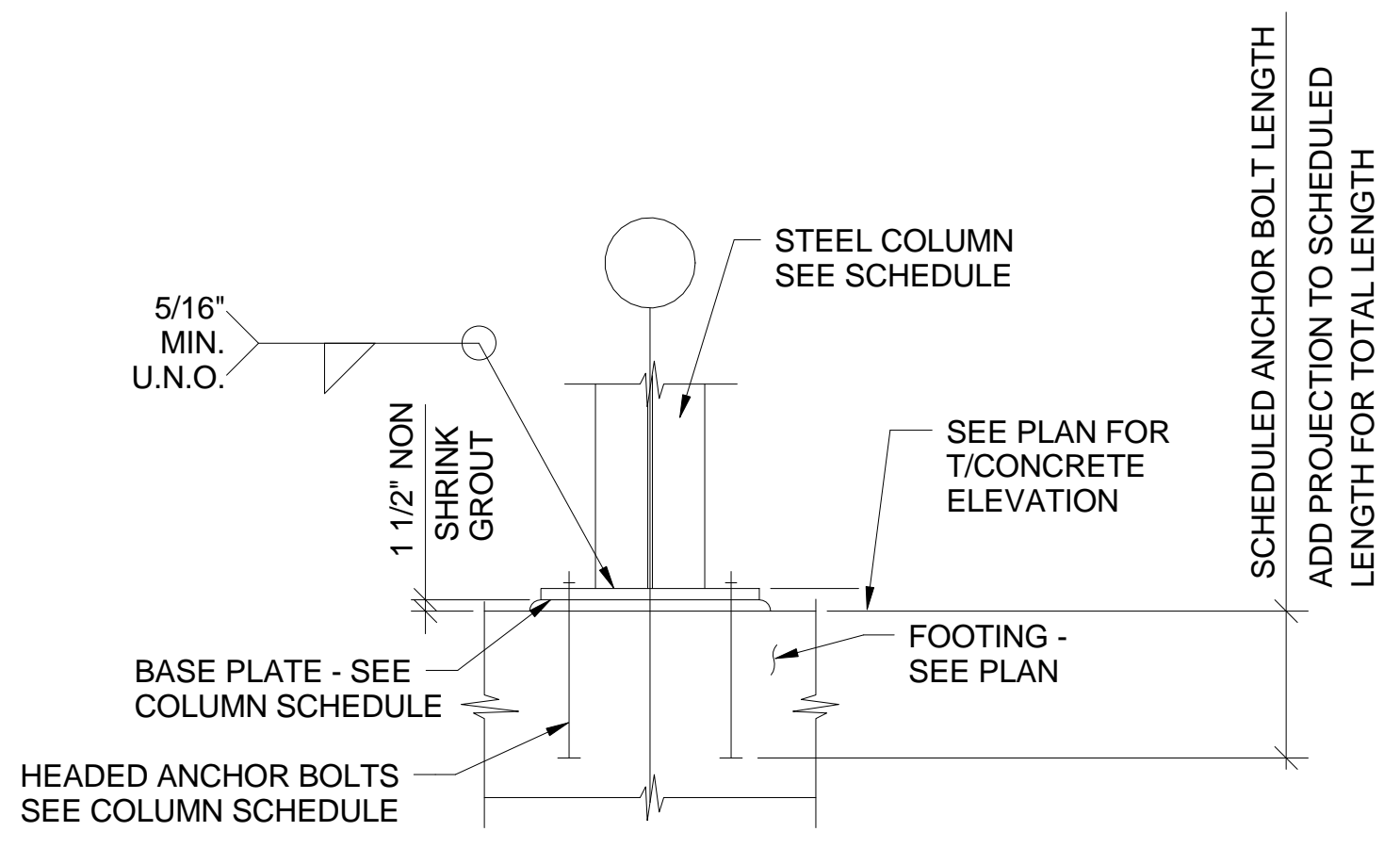
PHASE 6 - FINE ARTS BUILDING ADDITION
NEW MARION HIGH SCHOOL
MARION C.U.S.D. #2
MARION, WILLIAMSON COUNTY, ILLINOIS

Mark	Date	Description

DATE: 03/19/15
PROJECT NO: 360-2632
DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

MASONRY DETAILS

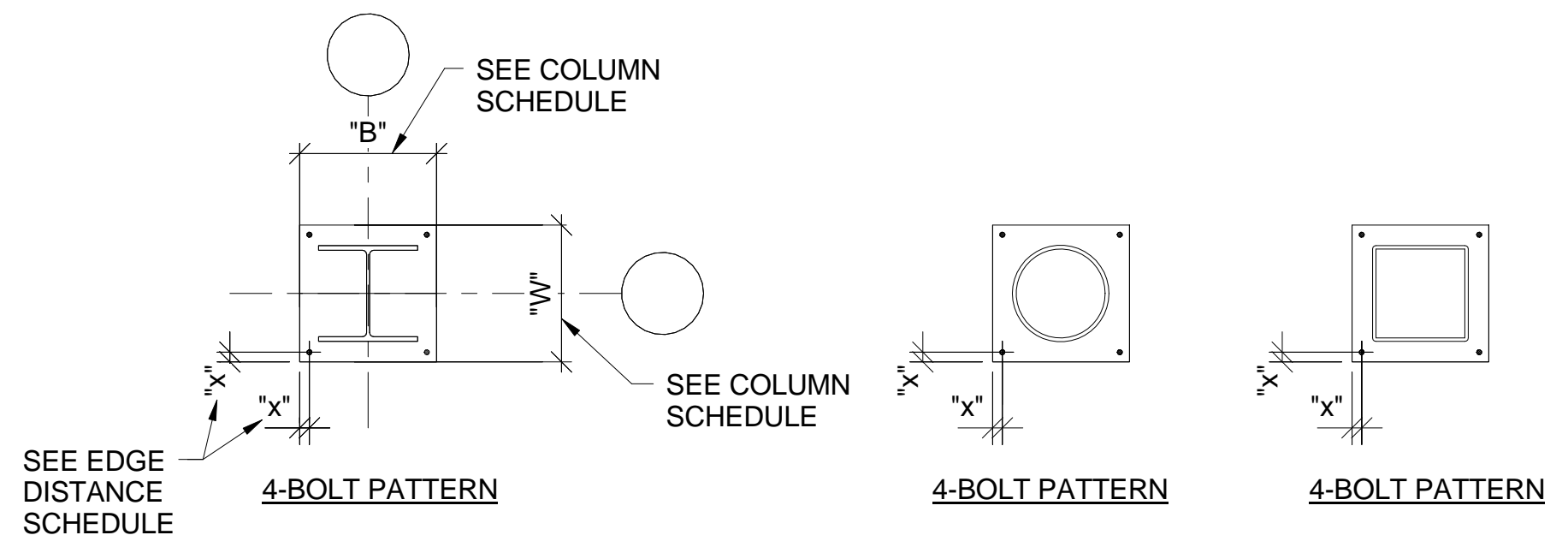
EDGE DISTANCE SCHEDULE	
BOLT DIAMETER	"X" (MIN.)
3/4"	1 3/4"



STRUCTURAL COLUMN WITHOUT PEDESTAL SCHEDULE		
Column Location Mark	Type	Base Plate
A-5	W8X28	Base Plate 4
B-6	W8X28	Base Plate 4
G-8	HSS5X5X3/8	Base Plate 1
C-7	HSS5X5X3/8	Base Plate 1
AA-DD	W10X39	Base Plate 4
A-9	W8X28	Base Plate 4
B-10	W8X28	Base Plate 4
H-13	W8X28	Base Plate 4
H-14	W8X28	Base Plate 4
FF-52	HSS4X4X3/8	Base Plate 1
A-11	W8X28	Base Plate 4
B-12	W8X28	Base Plate 4
HH-55	W8X28	Base Plate 4
HH-54	W8X28	Base Plate 4

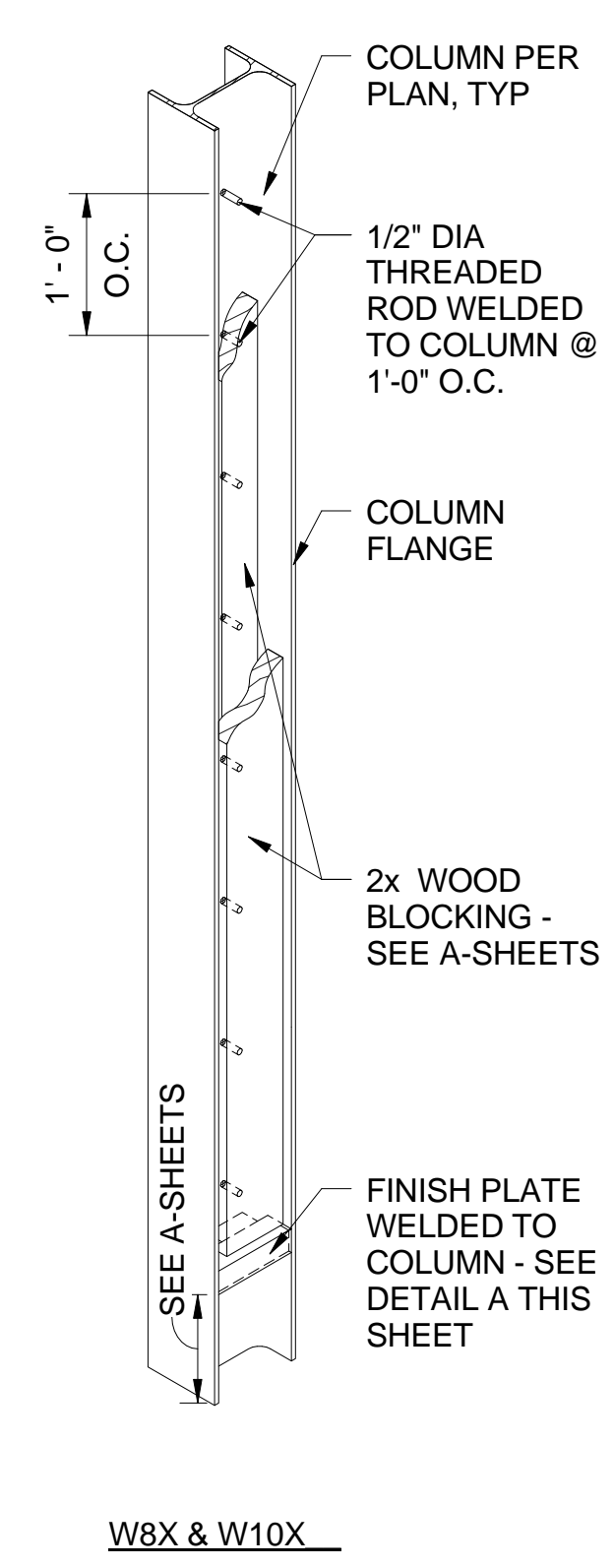
NOTE:
SEE SHEET S-502 FOR FULL COLUMN GRID MARKS.

STRUCTURAL COLUMN WITH PEDESTAL SCHEDULE						
Column Location Mark	Type	Base Plate	Pedestal Length	Pedestal Width	Pedestal Reinf.	Pedestal Ties
A-1	W8X28	Base Plate 4	1' - 6"	1' - 6"	4-#6	#4 @ 1'-0" O.C.
B-2	W8X28	Base Plate 4	1' - 6"	1' - 6"	4-#6	#4 @ 1'-0" O.C.
G-3	HSS4X4X3/8	Base Plate 1	1' - 4"	1' - 4"	4-#6	#4 @ 1'-0" O.C.
F-3	HSS4X4X3/8	Base Plate 1	1' - 4"	1' - 4"	4-#6	#4 @ 1'-0" O.C.
D-3	HSS4X4X3/8	Base Plate 1	1' - 4"	1' - 4"	4-#6	#4 @ 1'-0" O.C.
C-3	HSS4X4X3/8	Base Plate 1	1' - 4"	1' - 4"	4-#6	#4 @ 1'-0" O.C.
D-4	HSS4X4X3/8	Base Plate 1	1' - 4"	1' - 4"	4-#6	#4 @ 1'-0" O.C.
F-4	HSS4X4X3/8	Base Plate 1	1' - 4"	1' - 4"	4-#6	#4 @ 1'-0" O.C.
G-1	HSS5X0.250	Base Plate 3	1' - 4"	1' - 4"	4-#6	#4 @ 1'-0" O.C.
E-15	W8X28	Base Plate 4	1' - 6"	1' - 6"	4-#6	#4 @ 1'-0" O.C.
AA-53	HSS16X0.250	Base Plate 2	2' - 0"	2' - 0"	4-#6	#4 @ 1'-0" O.C.
AA-CC	W10X39	Base Plate 4	1' - 6"	1' - 6"	4-#6	#4 @ 1'-0" O.C.
D-1	HSS5X0.250	Base Plate 3	1' - 4"	1' - 4"	4-#6	#4 @ 1'-0" O.C.
H-15	W8X28	Base Plate 4	1' - 6"	1' - 6"	4-#6	#4 @ 1'-0" O.C.
GG-51	HSS4X4X3/8	Base Plate 1	1' - 4"	1' - 4"	4-#6	#4 @ 1'-0" O.C.
GG-52	HSS4X4X3/8	Base Plate 1	1' - 4"	1' - 4"	4-#6	#4 @ 1'-0" O.C.
FF-51	HSS4X4X3/8	Base Plate 1	1' - 4"	1' - 4"	4-#6	#4 @ 1'-0" O.C.
EE-52	HSS4X4X3/8	Base Plate 1	1' - 4"	1' - 4"	4-#6	#4 @ 1'-0" O.C.
EE-51	HSS4X4X3/8	Base Plate 1	1' - 4"	1' - 4"	4-#6	#4 @ 1'-0" O.C.
GG-50	HSS5X0.250	Base Plate 3	1' - 4"	1' - 4"	4-#6	#4 @ 1'-0" O.C.
DD-53	W10X39	Base Plate 4	1' - 6"	1' - 6"	4-#6	#4 @ 1'-0" O.C.
CC-53	W10X39	Base Plate 4	1' - 6"	1' - 6"	4-#6	#4 @ 1'-0" O.C.
AA-BB	HSS7.500X0.250	Base Plate 3	1' - 4"	1' - 4"	4-#6	#4 @ 1'-0" O.C.
BB-53	HSS7.500X0.250	Base Plate 3	1' - 4"	1' - 4"	4-#6	#4 @ 1'-0" O.C.

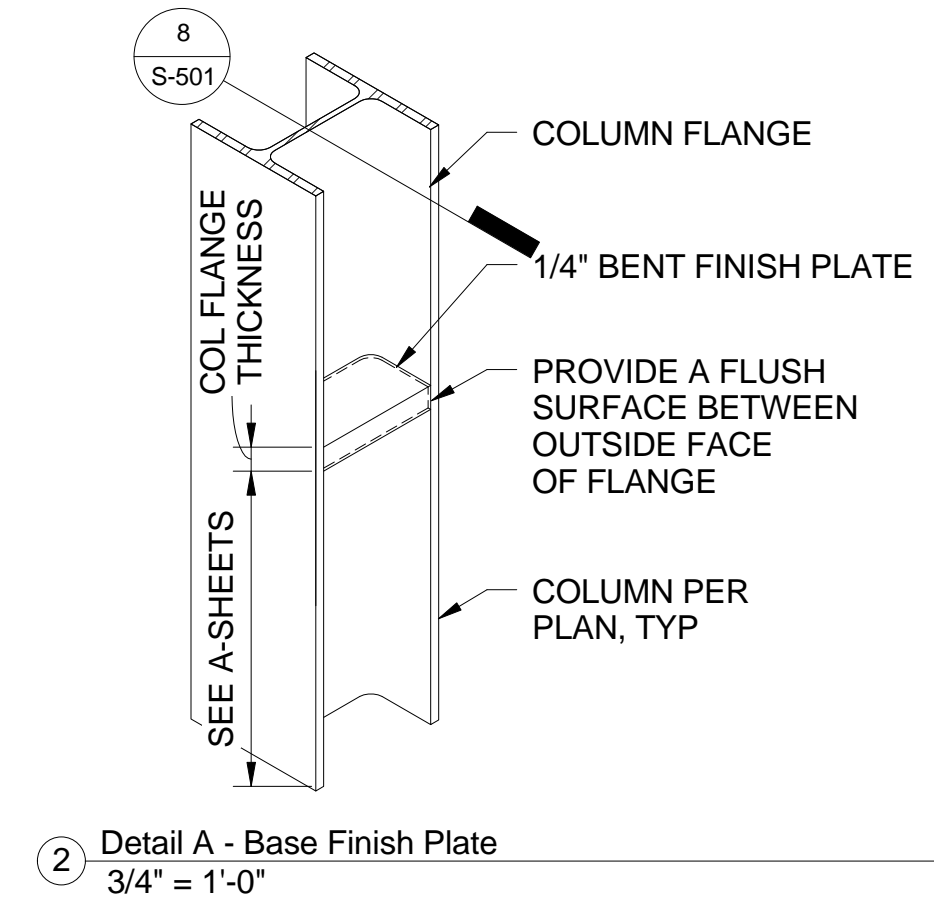


1 Typ. Steel Column Base Plate
1/8" = 1'-0"

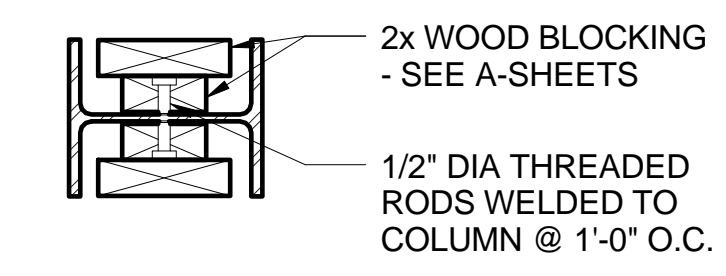
Base Plate Schedule								
Type	Base Plate Thickness	Base Plate Length	Base Plate Width	Base Plate Material	Number of Bolts	Anchor Bolt Dia.	Anchor Bolt Length	Anchor Material
Base Plate 1	7/8"	1' - 0"	1' - 0"	Fy = 36 ksi	4	3/4"	1' - 0"	F1554-36
Base Plate 2	1/2"	1' - 10"	1' - 10"	Fy = 36 ksi	4	3/4"	1' - 0"	F1554-36
Base Plate 3	3/4"	1' - 2"	1' - 2"	Fy = 36 ksi	4	3/4"	1' - 0"	F1554-36
Base Plate 4	1/2"	1' - 4"	1' - 4"	Fy = 36 ksi	4	3/4"	1' - 0"	F1554-36



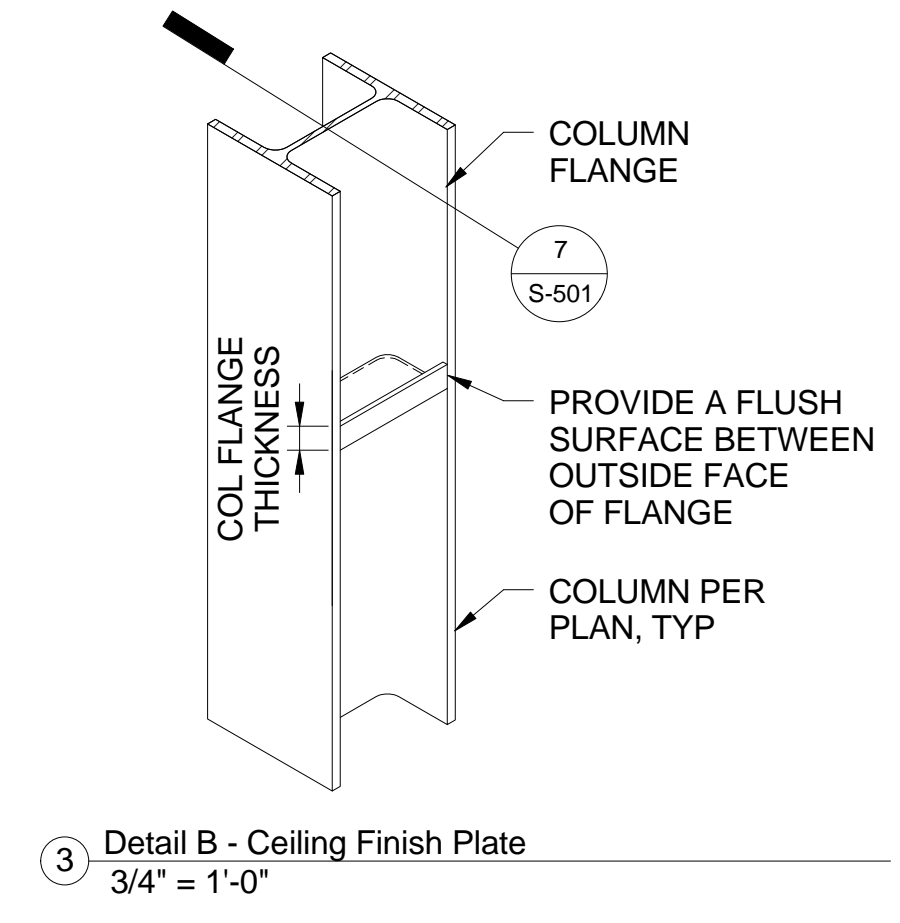
5 Special Column Finishing - Isometric View
3/4" = 1'-0"



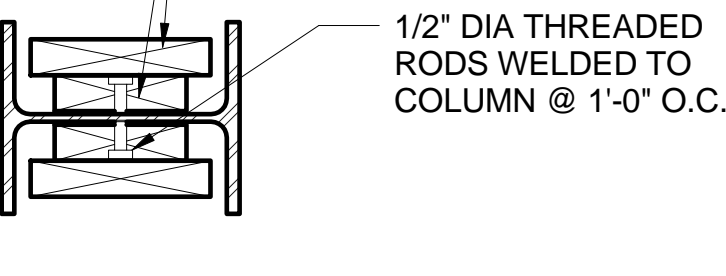
NOTE: SEE A-SHEETS FOR COLUMNS TO RECEIVE WOOD BLOCKING



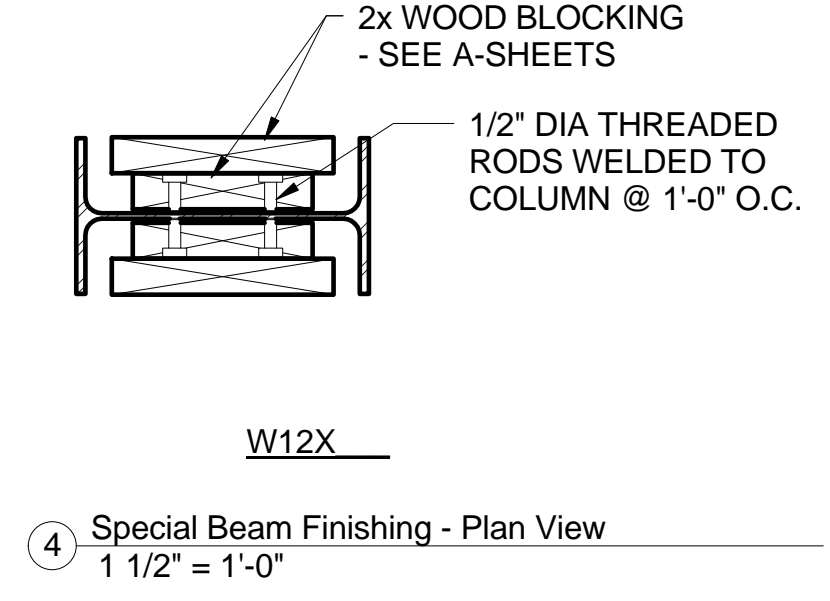
6 Special Column Finishing - Plan View
1 1/2" = 1'-0"



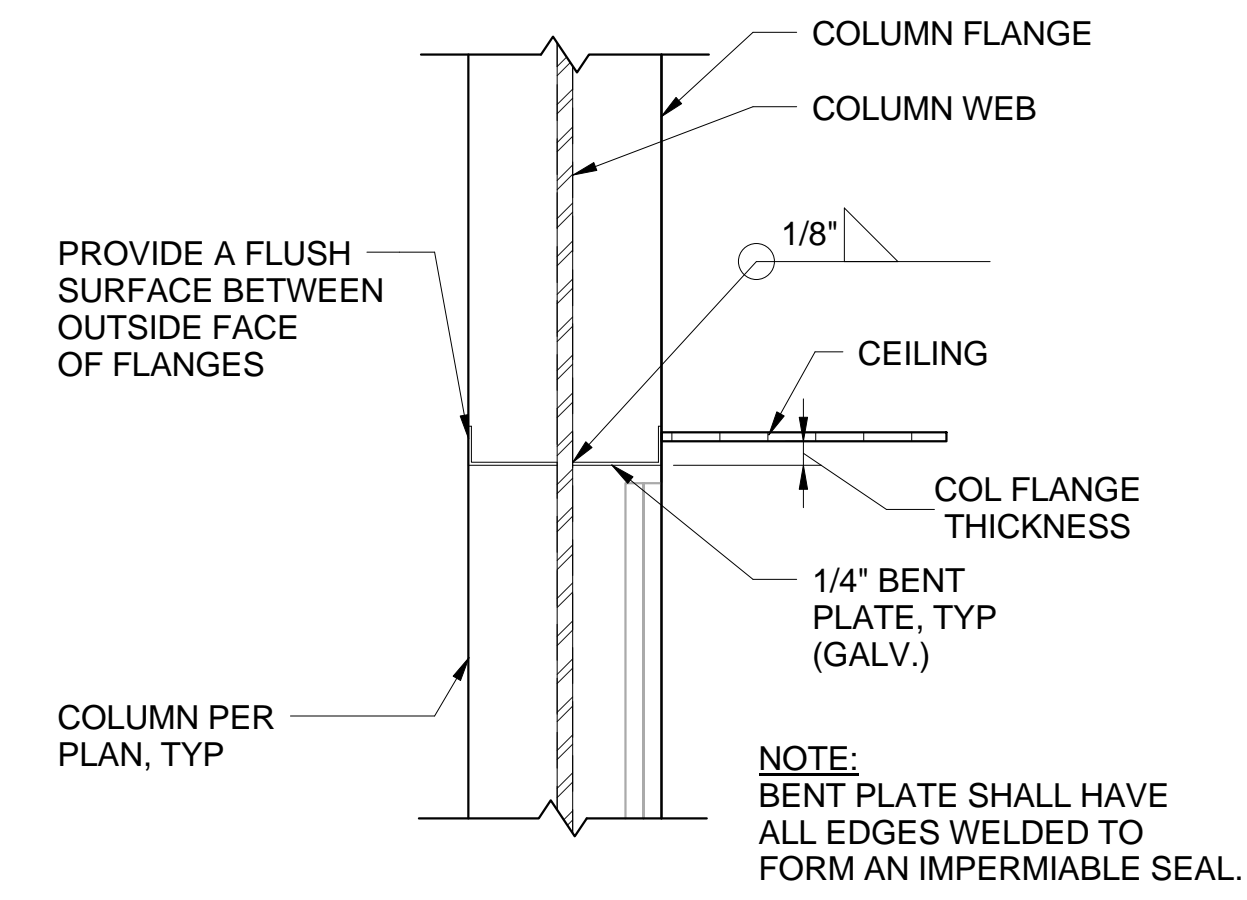
NOTE: SEE A-SHEETS FOR COLUMNS TO RECEIVE WOOD BLOCKING



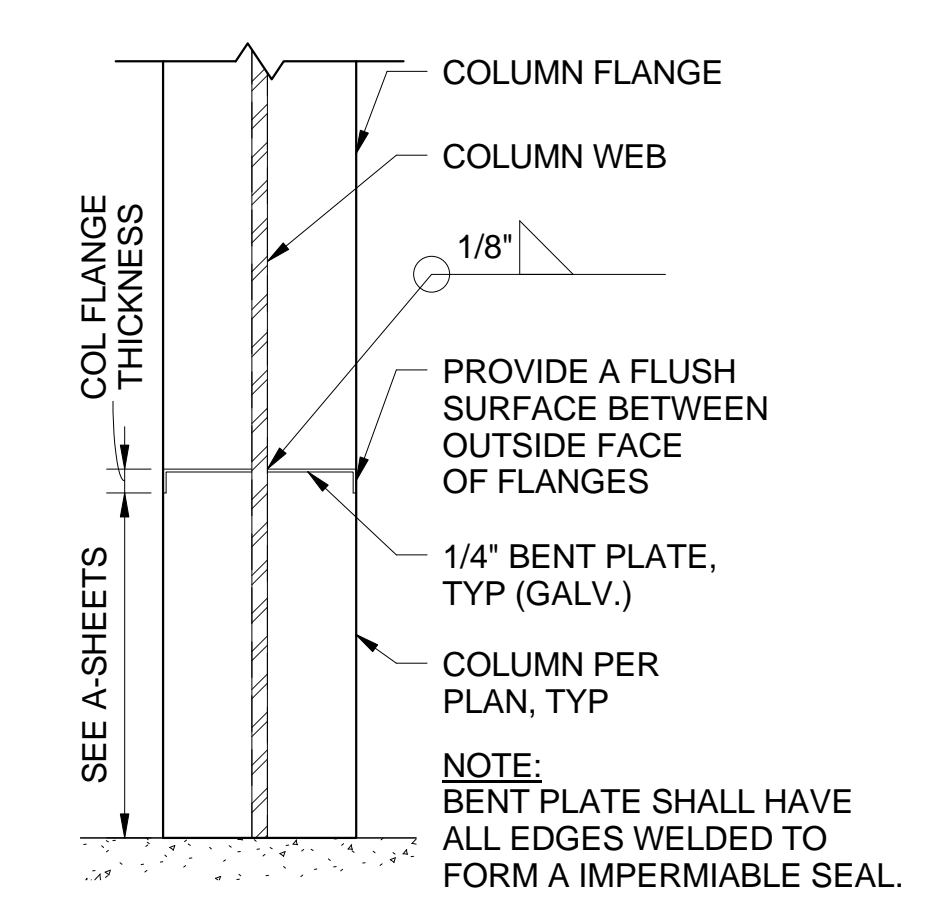
7 Ceiling Finish Plate - Section View
3/4" = 1'-0"



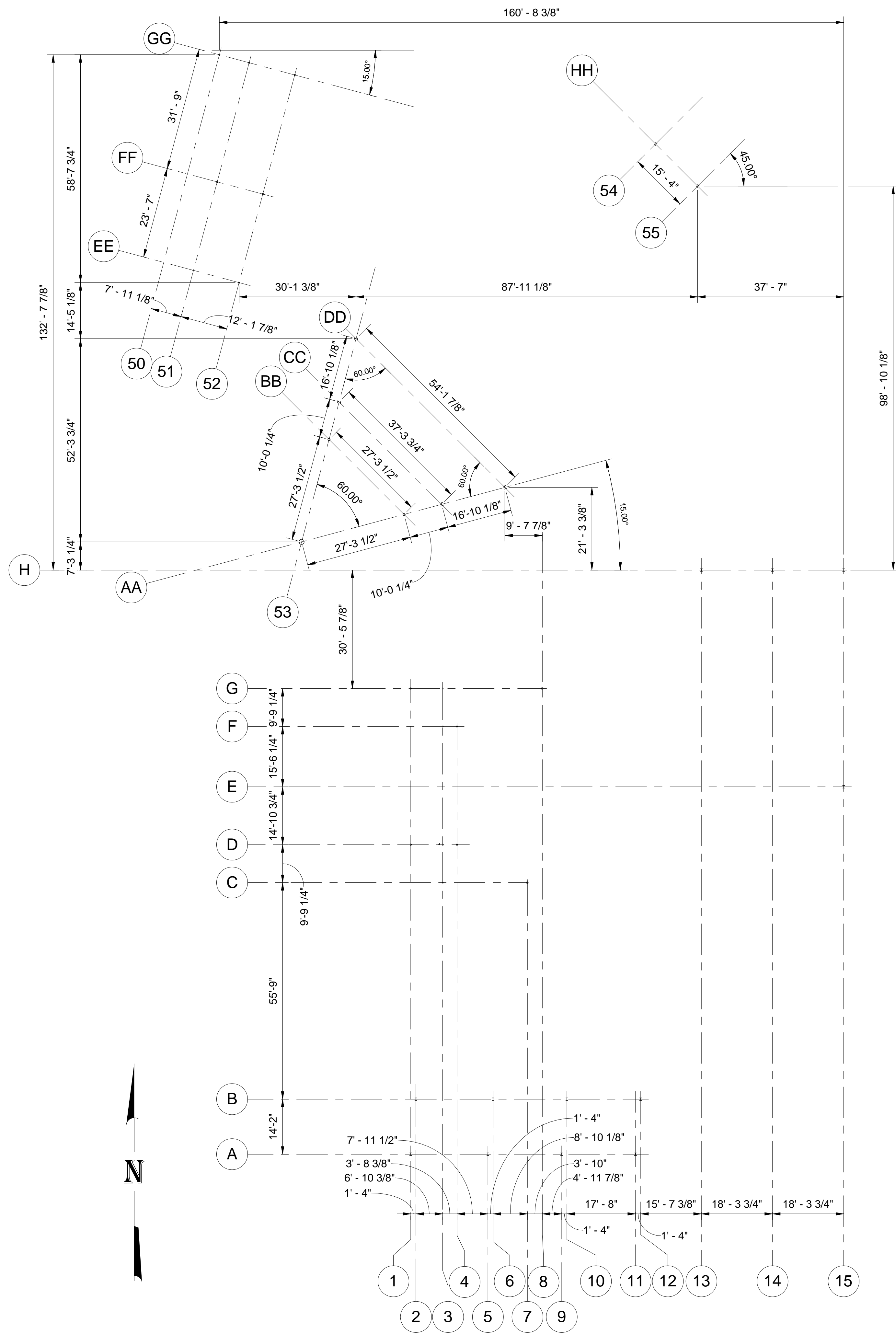
4 Special Beam Finishing - Plan View
1 1/2" = 1'-0"



7 Ceiling Finish Plate - Section View
3/4" = 1'-0"

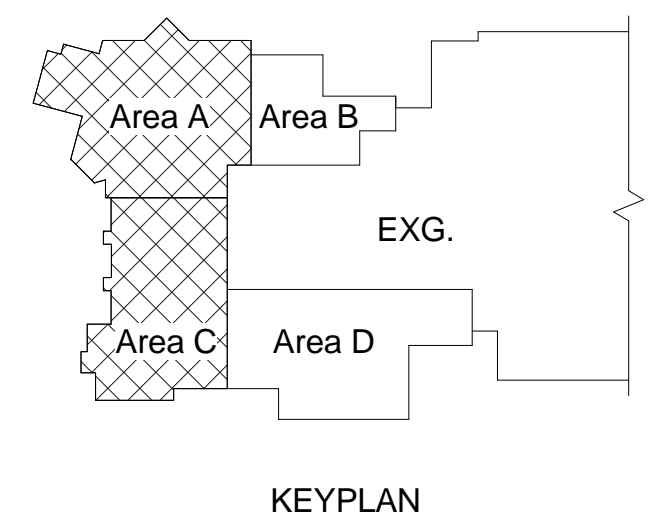


8 Base Finish Plate - Section View
3/4" = 1'-0"



T.O. Concrete									T.O. Concrete	472' - 0"
472' - 0"										472' - 0"
T.O. Upper Masonry									T.O. Upper Masonry	458' - 0"
458' - 0"										458' - 0"
Classroom Roof									Classroom Roof	448' - 0"
448' - 0"										448' - 0"
First Floor F.F.E.									First Floor F.F.E.	432' - 0"
432' - 0"										432' - 0"
Orchestra Pit F.F.E.									Orchestra Pit F.F.E.	423' - 4"
423' - 4"										423' - 4"
Column Locations	W8X28	W8X28	HSS4X4X3/8	HSS5X5X3/8	HSS5X0.250	W8X28	W8X28	HSS16X0.250		
	A-1, A-5, A-9, A-11	B-2, B-6, B-10, B-12	C-3, D-3, D-4, F-3, F-4, G-3	C-7, G-8	D-1, G-1, GG-50	E-15	H-13, H-14, H-15	AA-53		

T.O. Concrete								T.O. Concrete	472' - 0"
472' - 0"									472' - 0"
T.O. Upper Masonry								T.O. Upper Masonry	458' - 0"
458' - 0"									458' - 0"
Classroom Roof								Classroom Roof	448' - 0"
448' - 0"									448' - 0"
First Floor F.F.E.								First Floor F.F.E.	432' - 0"
432' - 0"									432' - 0"
Orchestra Pit F.F.E.								Orchestra Pit F.F.E.	423' - 4"
423' - 4"									423' - 4"
Column Locations	HSS7.500X0.250	W10X39	W10X39	HSS4X4X3/8	HSS4X4X3/8	W8X28			
	BB-53, AA-BB	CC-53, AA-CC	DD-53, AA-DD	EE-51, FF-51, GG-51	EE-52, FF-52, GG-52	HH-54, HH-55			



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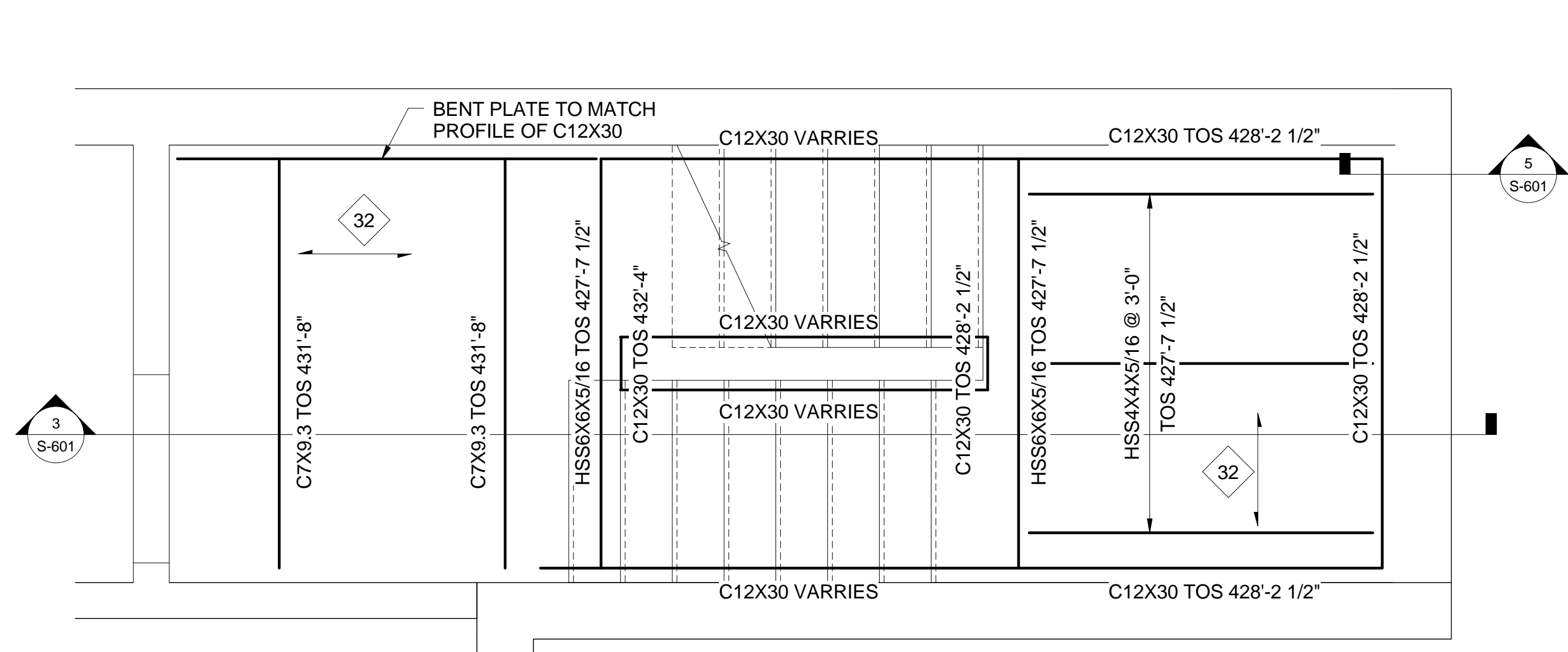
PHASE 6 - FINE ARTS BUILDING ADDITION
NEW MARION HIGH SCHOOL
MARION C.U.S.D. #2
MARION, WILLIAMSON COUNTY, ILLINOIS

Mark	Date	Description

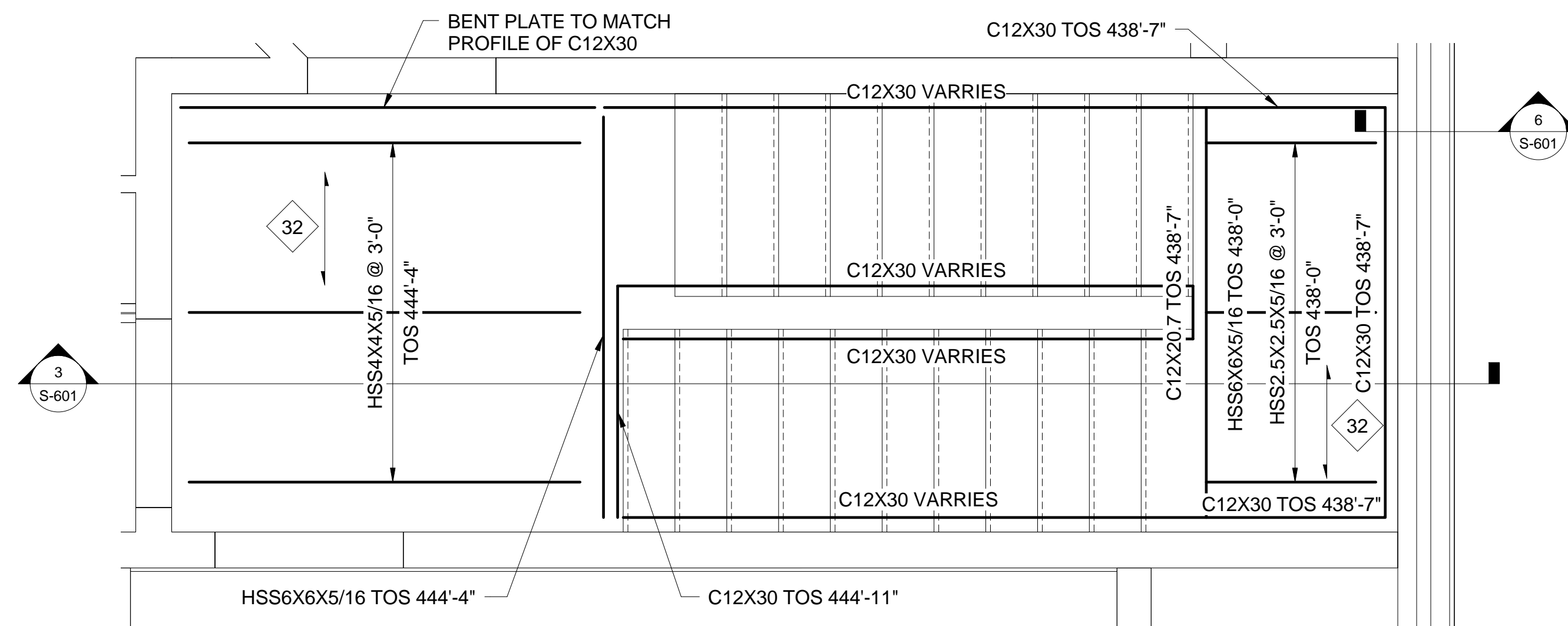
DATE: 03/19/15
PROJECT NO: 360-2632
DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

COLUMN GRID LINE
PLAN & GRAPHICAL
COLUMN SCHEDULE

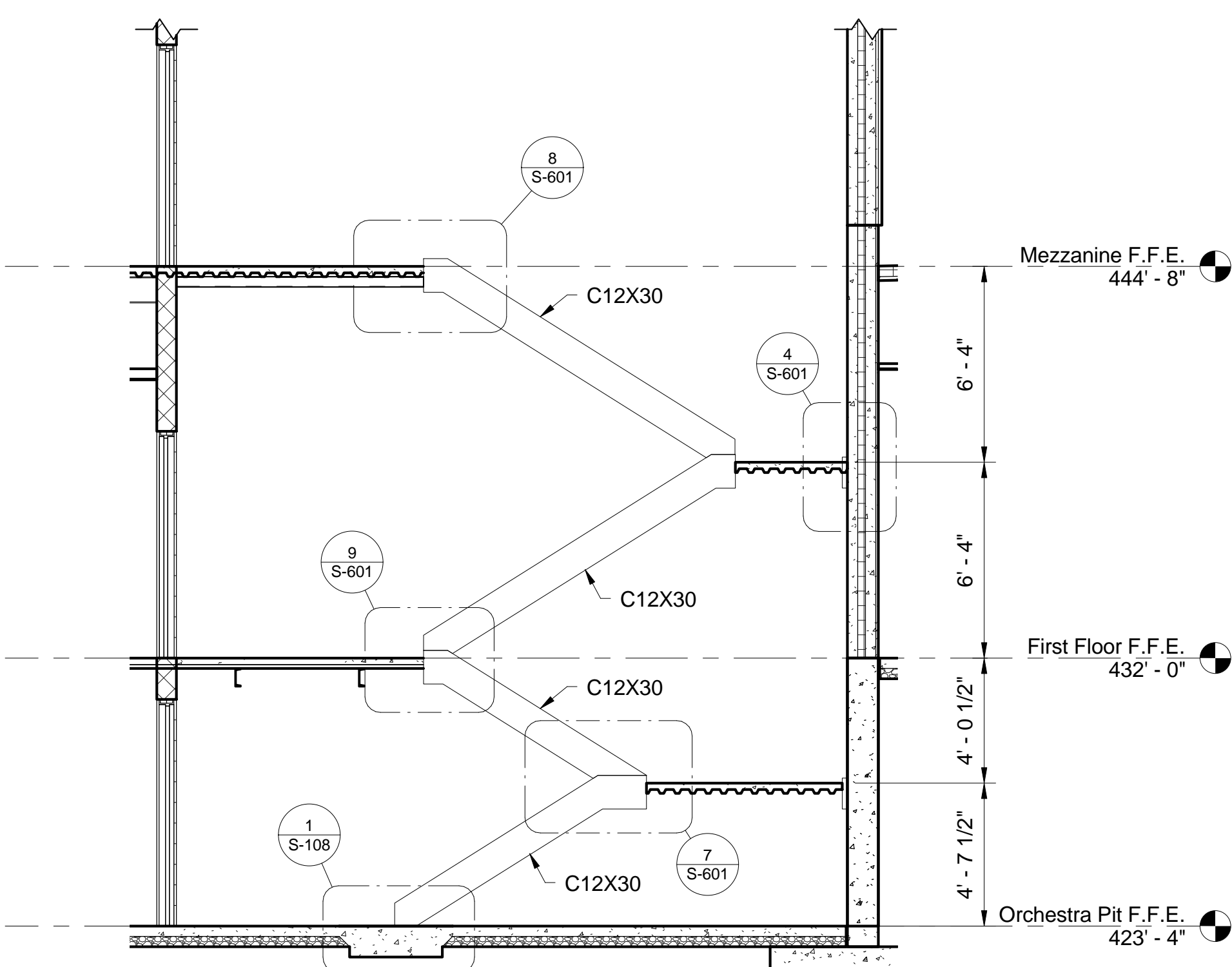
S-502



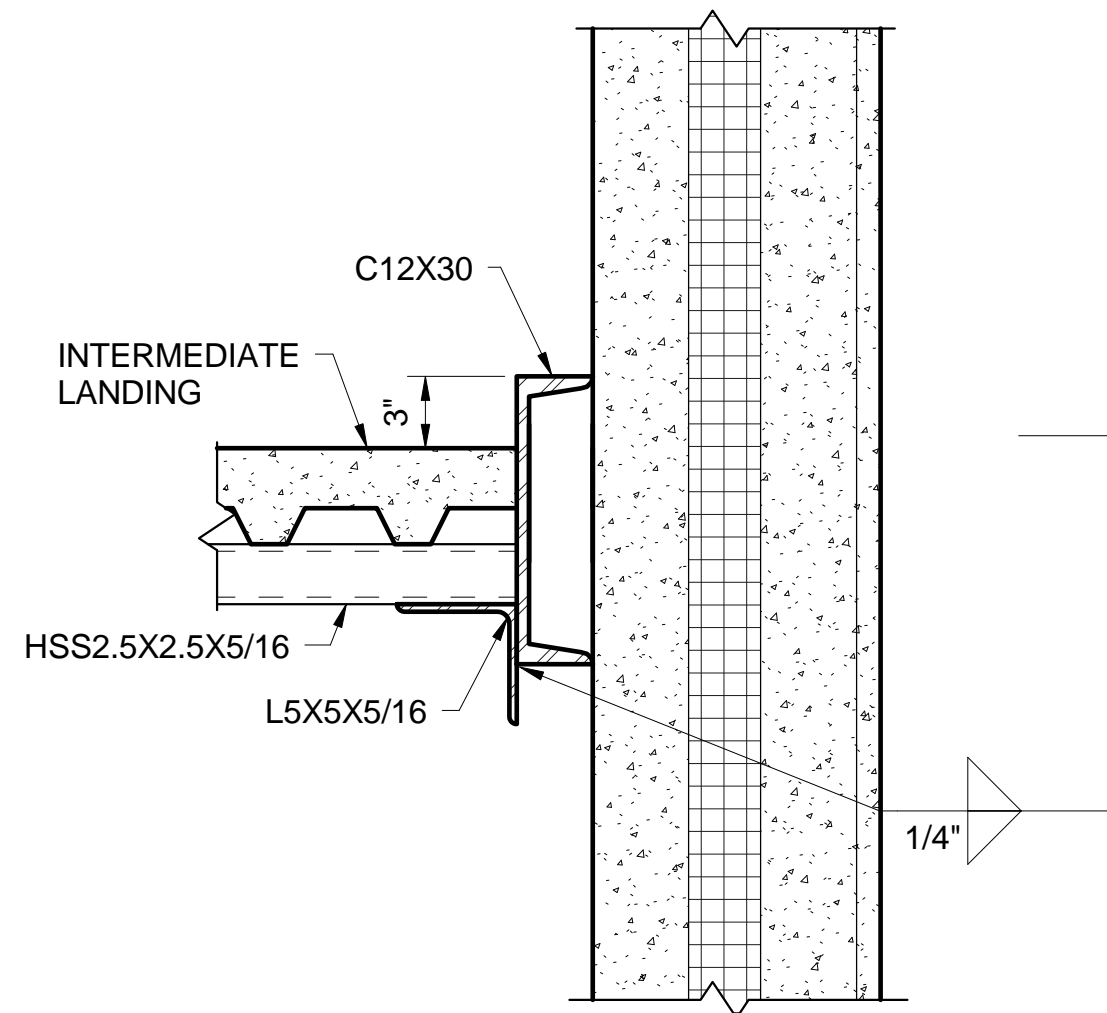
1 Enlarged Stair III - Basement to 1st Floor
1/2" = 1'-0"



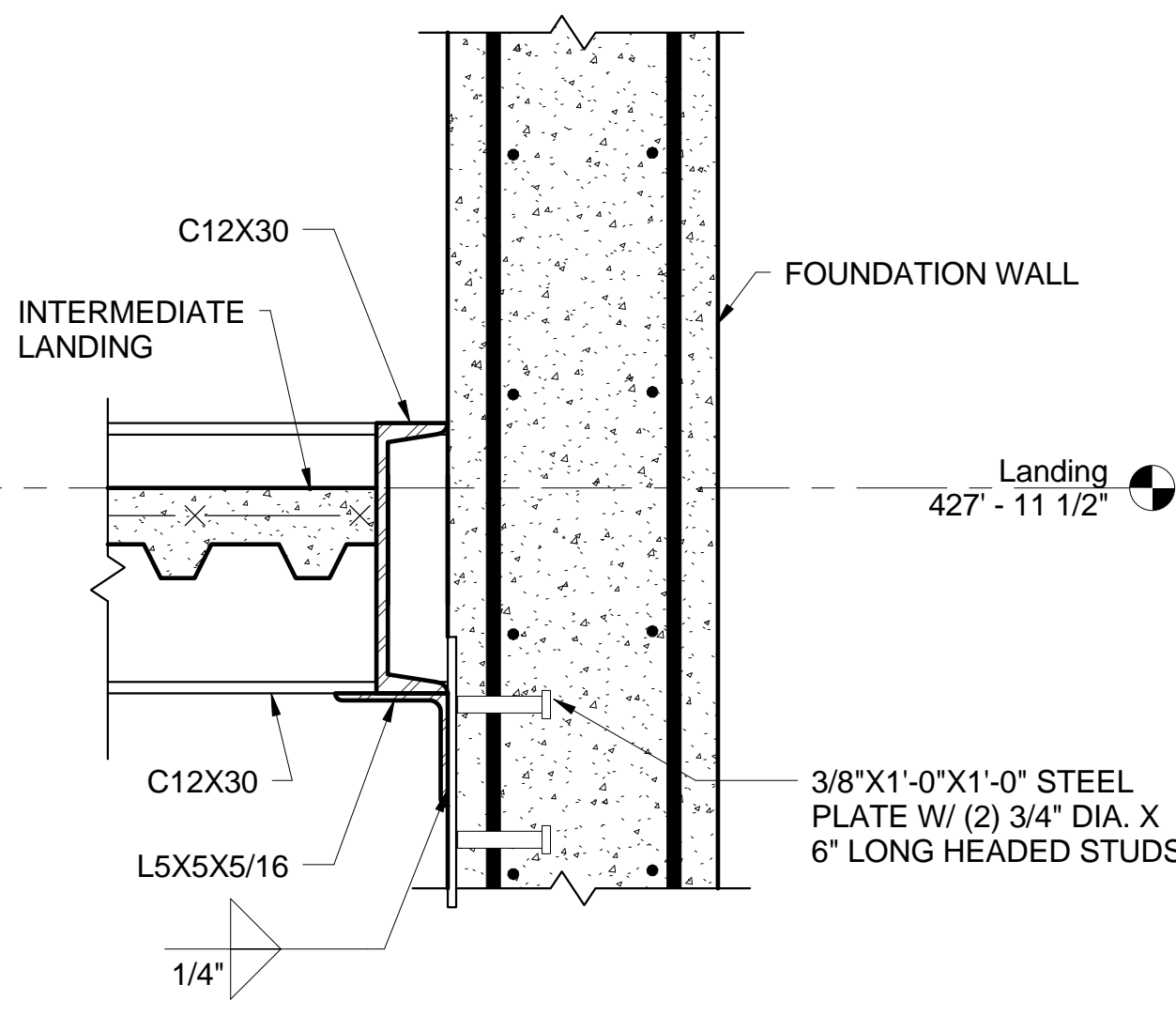
2 Enlarged Stair III - 1st Floor to Mezzanine
1/2" = 1'-0"



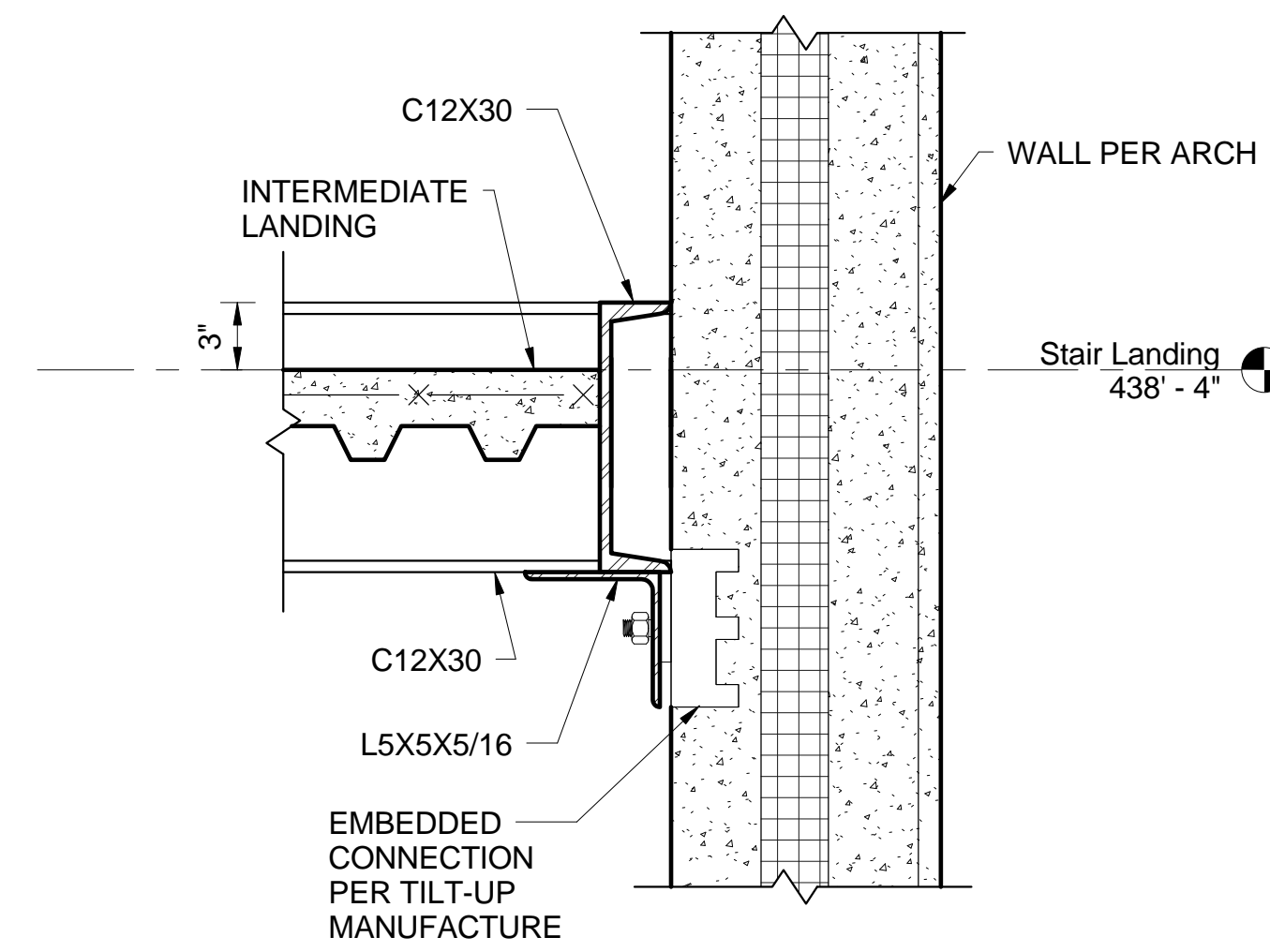
3 Stair Section
1/4" = 1'-0"



4 Stair Callout
1 1/2" = 1'-0"

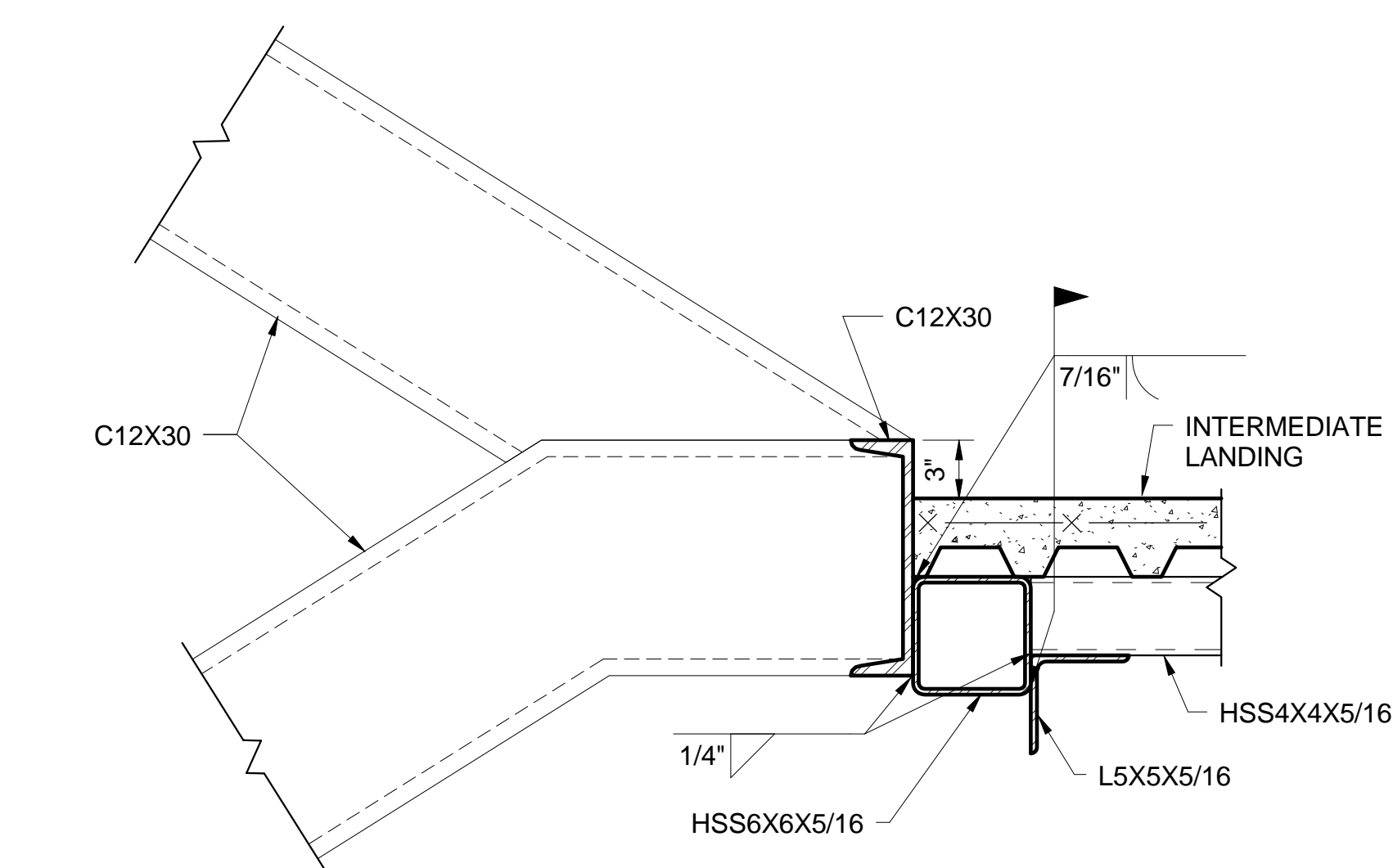


5 Stair Detail
1 1/2" = 1'-0"

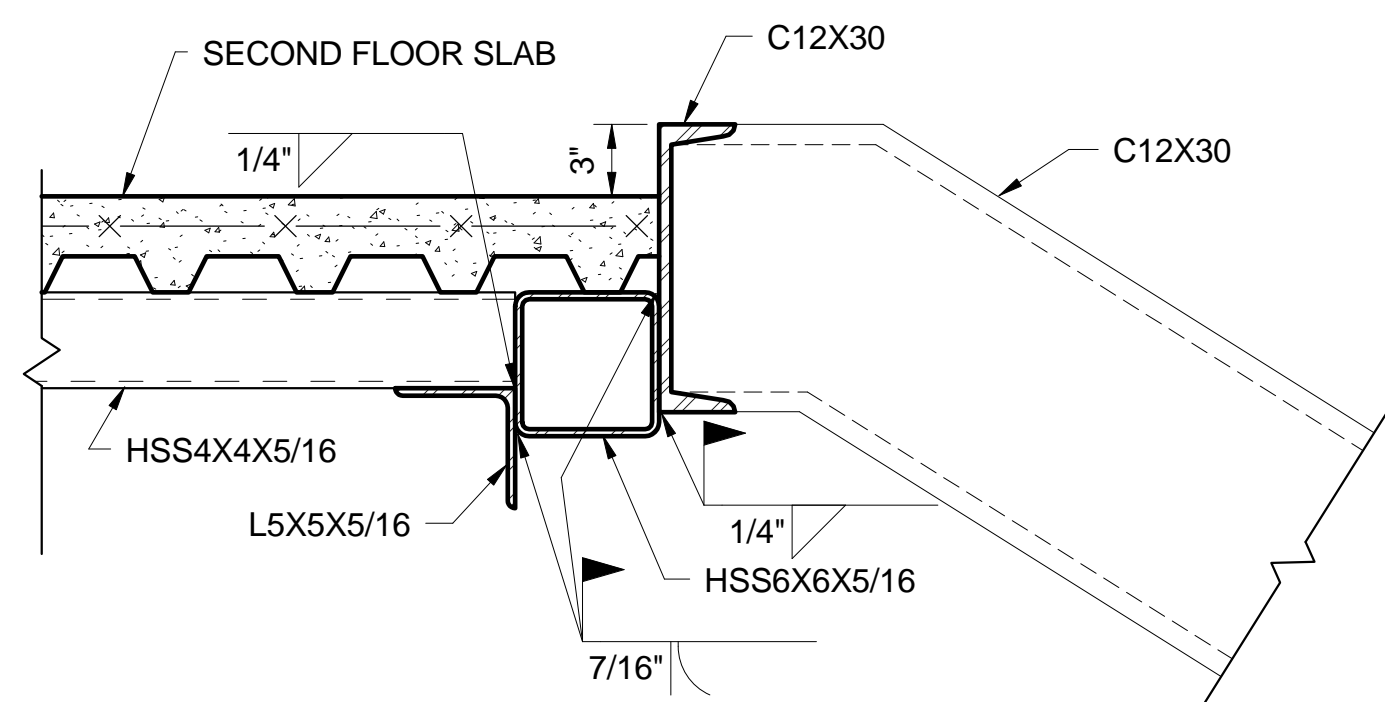


6 Stair Detail
1 1/2" = 1'-0"

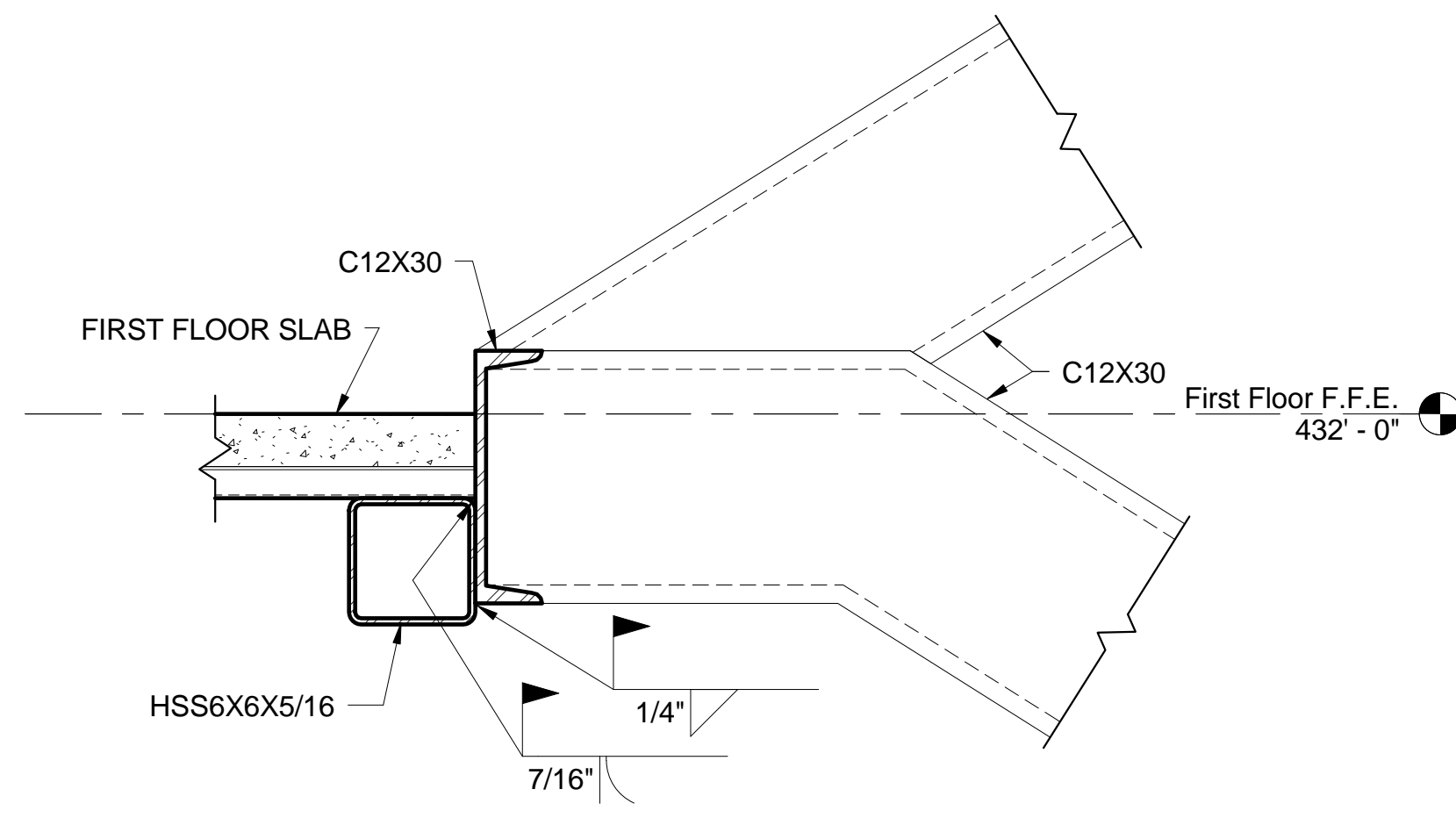
KEYNOTE LEGEND	
KEY VALUE	KEYNOTE TEXT
32	2 1/2" LIGHTWEIGHT CONCRETE OVER 1 1/2" - 18 GAUGE COMPOSITE METAL DECK (TOTAL THICKNESS = 4") - REINFORCED WITH 6X6 - W1.4X1.4



7 Stair Detail
1 1/2" = 1'-0"



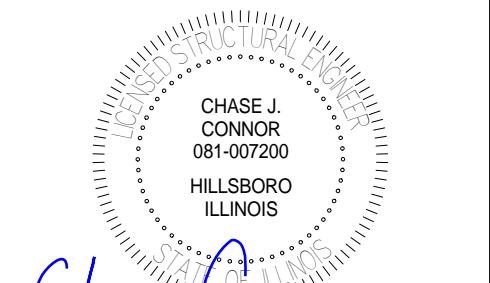
8 Stair Detail
1 1/2" = 1'-0"



9 Stair Section - Callout 1
1 1/2" = 1'-0"

GENERAL NOTES:

- REFER TO SHEET S-001 & S-002 FOR ADDITIONAL INFORMATION.
- SEE ARCH. SHEETS FOR DIMENSIONS OF STAIRS.
- WALL DETAILS SHOWN ARE FOR STRUCTURAL REPRESENTATION ONLY. SEE ARCH. SHEETS FOR SIZE, TEXTURE, COLOR, ETC.



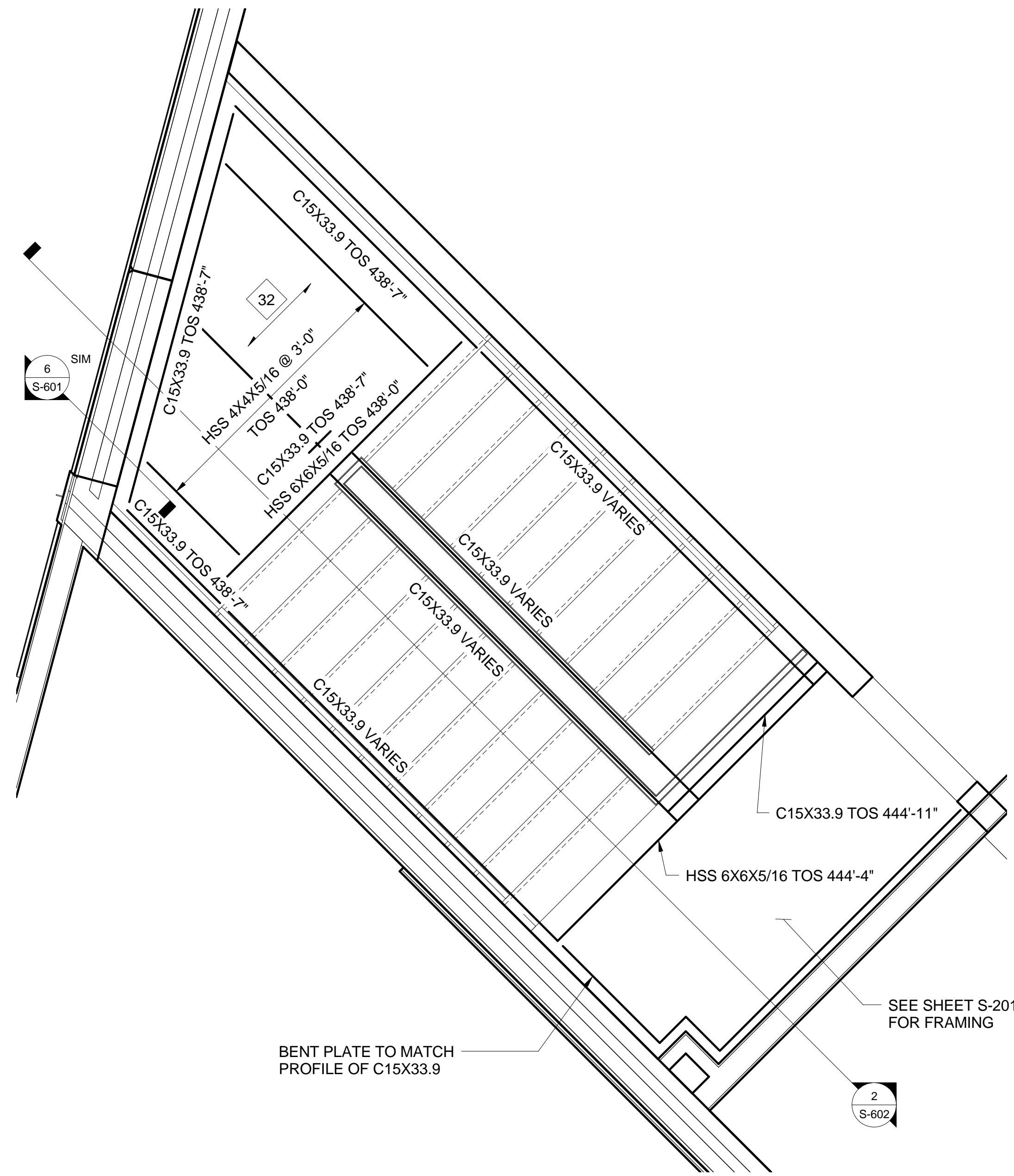
SIGNATURE: *Chase Connor*
DATE: 03/19/15
LICENSE EXPIRES: 11-30-2016

PHASE 6 - FINE ARTS BUILDING ADDITION
NEW MARION HIGH SCHOOL
MARION C.U.S.D. #2
MARION, WILLIAMSON COUNTY, ILLINOIS

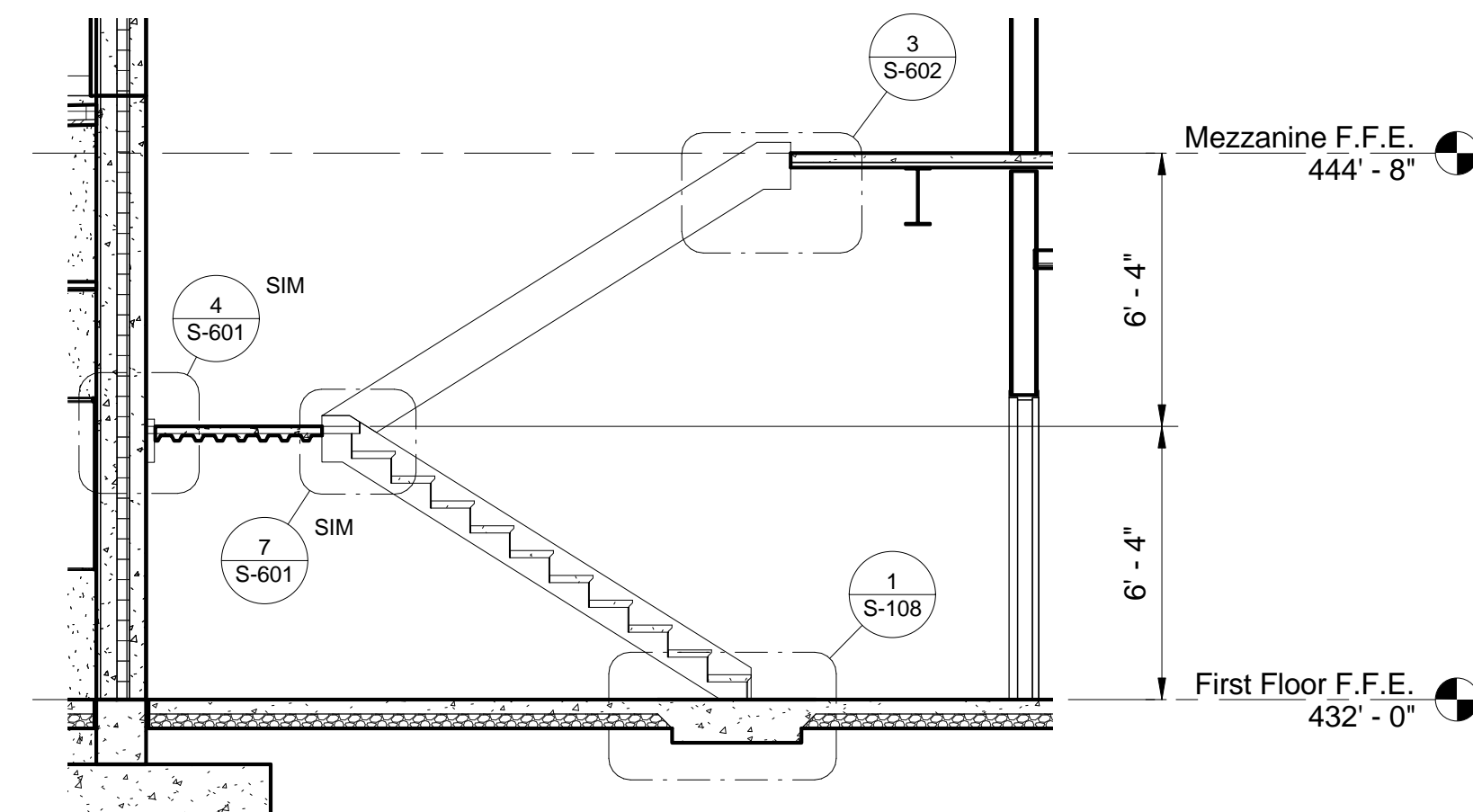
Mark	Date	Description

DATE: 03/19/15
PROJECT NO: 360-2632
DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

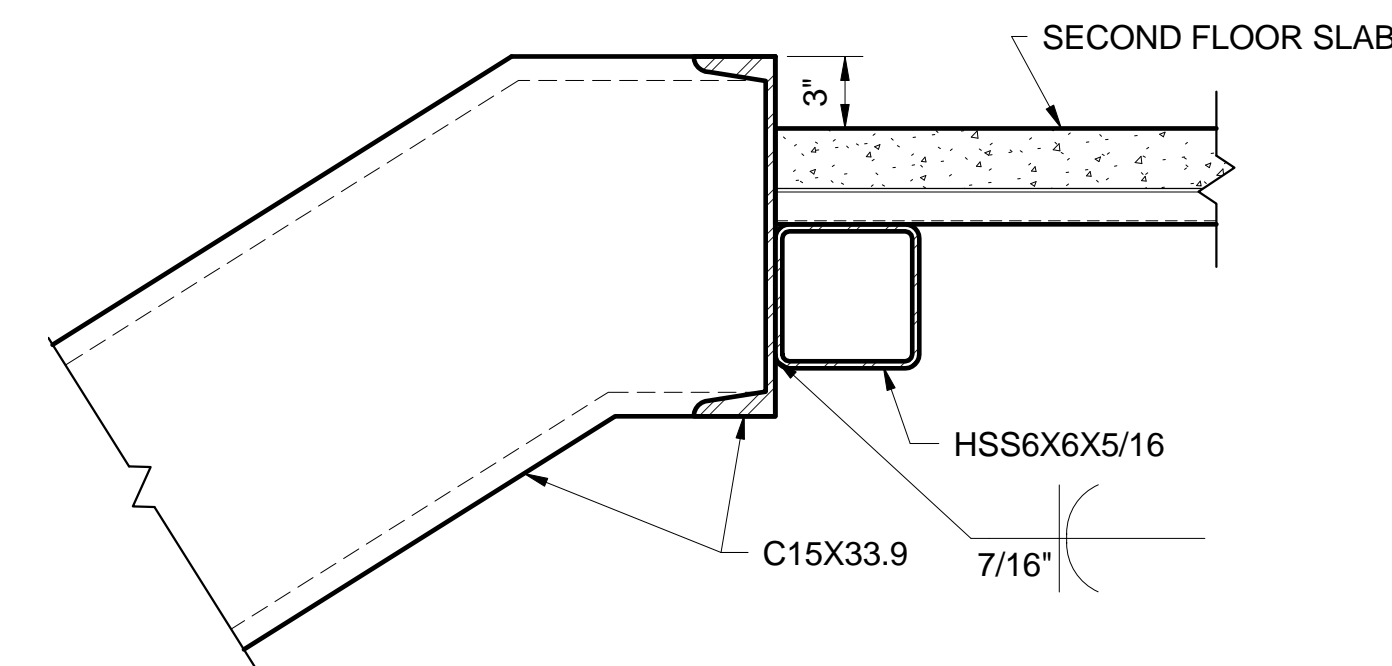
STAIR PLANS,
SECTIONS, AND
DETAILS



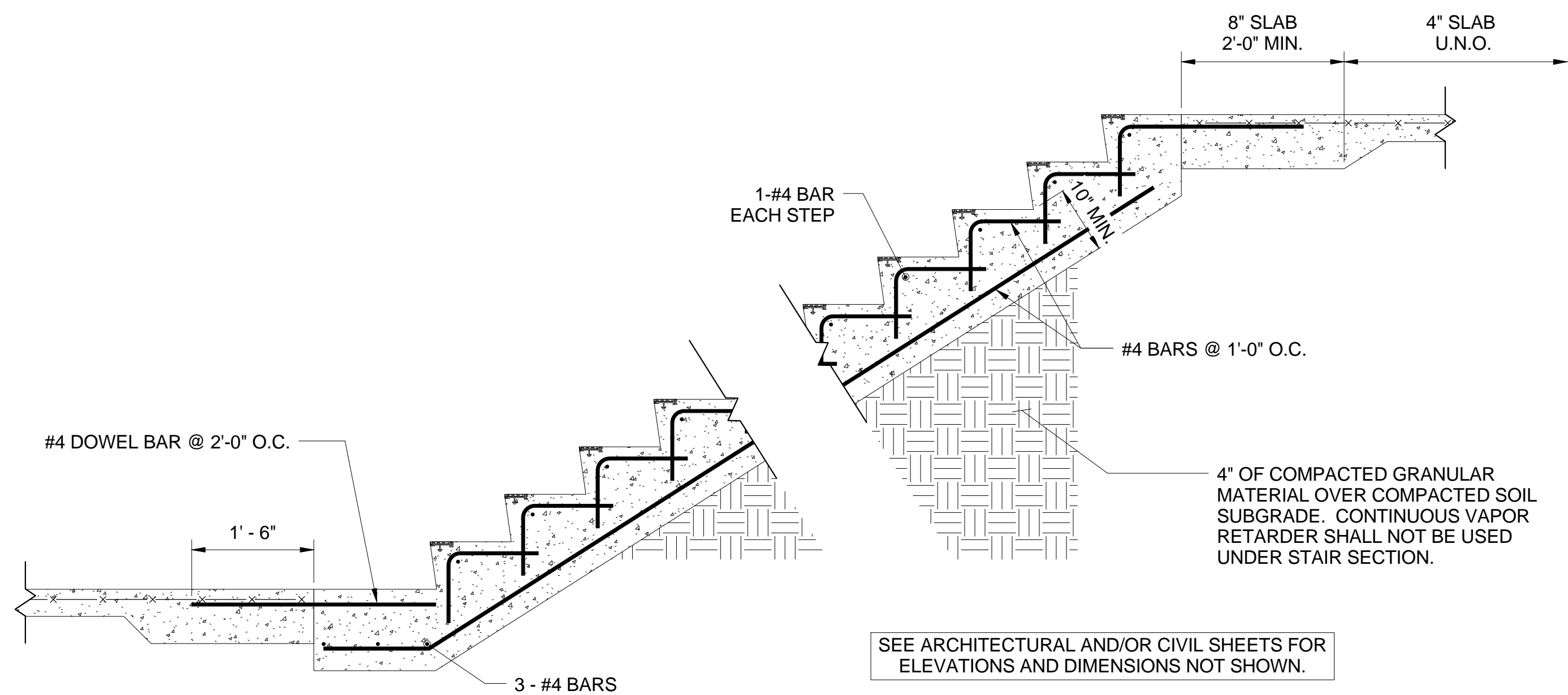
1 Enlarged Stair 118
1/2" = 1'-0"



2 Stair Section
1/4" = 1'-0"



3 Stair Detail
1 1/2" = 1'-0"



4 Typical Cast-In-Place Stair
3/4" = 1'-0"

KEYNOTE LEGEND	
KEY VALUE	KEYNOTE TEXT
32	2 1/2" LIGHTWEIGHT CONCRETE OVER 1 1/2" - 18 GAUGE COMPOSITE METAL DECK (TOTAL THICKNESS = 4") - REINFORCED WITH 6X6 - W1.4X1.4

GENERAL NOTES:

- REFER TO SHEET S-001 & S-002 FOR ADDITIONAL INFORMATION.
- SEE ARCH. SHEETS FOR DIMENSIONS OF STAIRS.
- WALL DETAILS SHOWN ARE FOR STRUCTURAL REPRESENTATION ONLY. SEE ARCH. SHEETS FOR SIZE, TEXTURE, COLOR, ETC.



Chase Connor
SIGNATURE

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PHASE 6 - FINE ARTS BUILDING ADDITION
NEW MARION HIGH SCHOOL
MARION C.U.S.D. #2
MARION, WILLIAMSON COUNTY, ILLINOIS

Mark	Date	Description

DATE: 03/19/15
PROJECT NO: 360-2632
DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

STAIR PLANS,
SECTIONS, AND
DETAILS

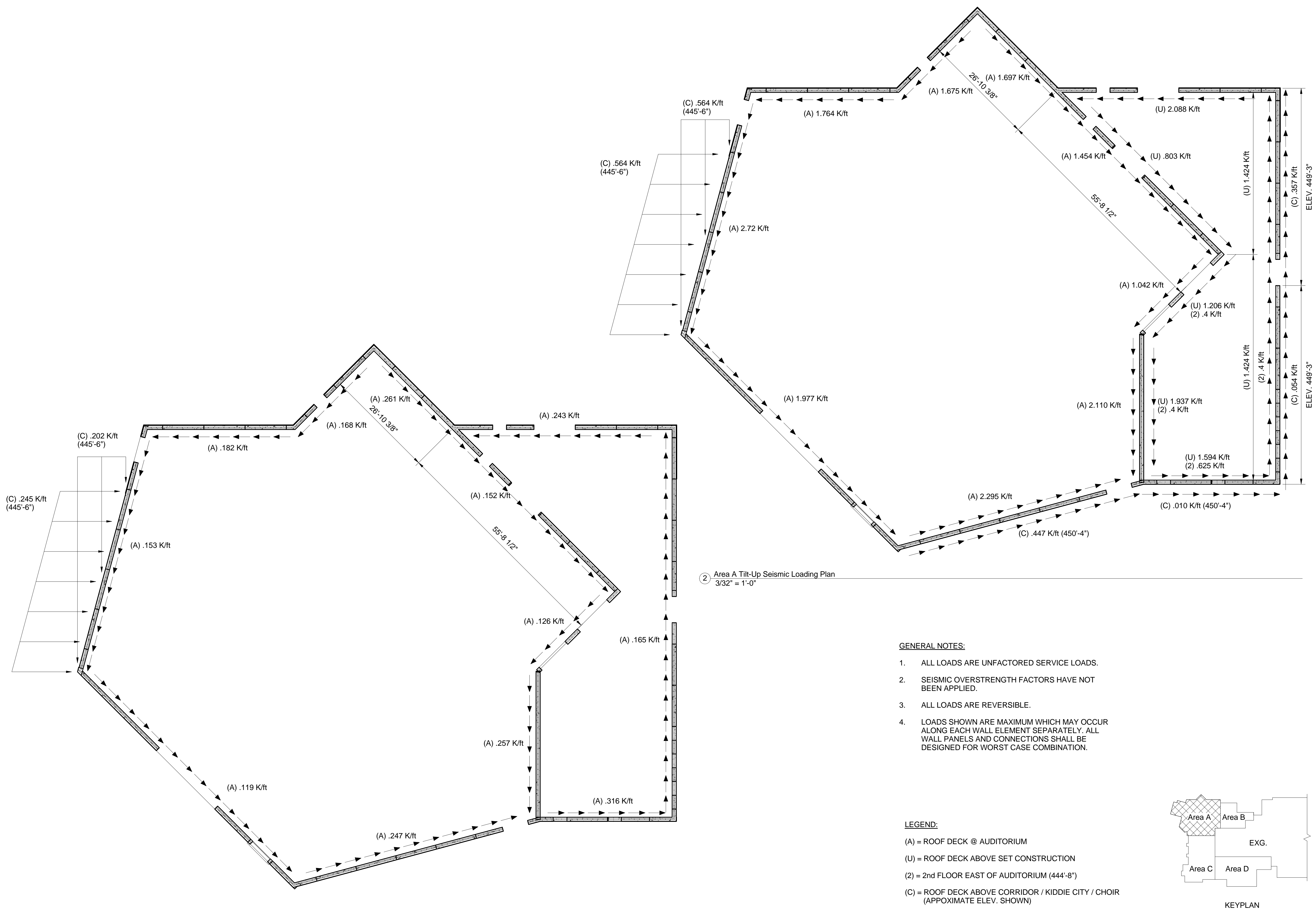
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DRAWN:	N.A.L.
CHECK:	R.V.B.

TILT-UP PANEL
LATERAL LOADING
PLAN

S-701

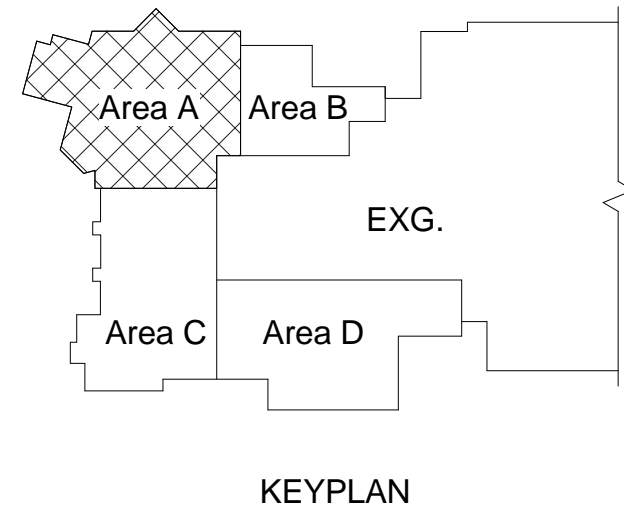


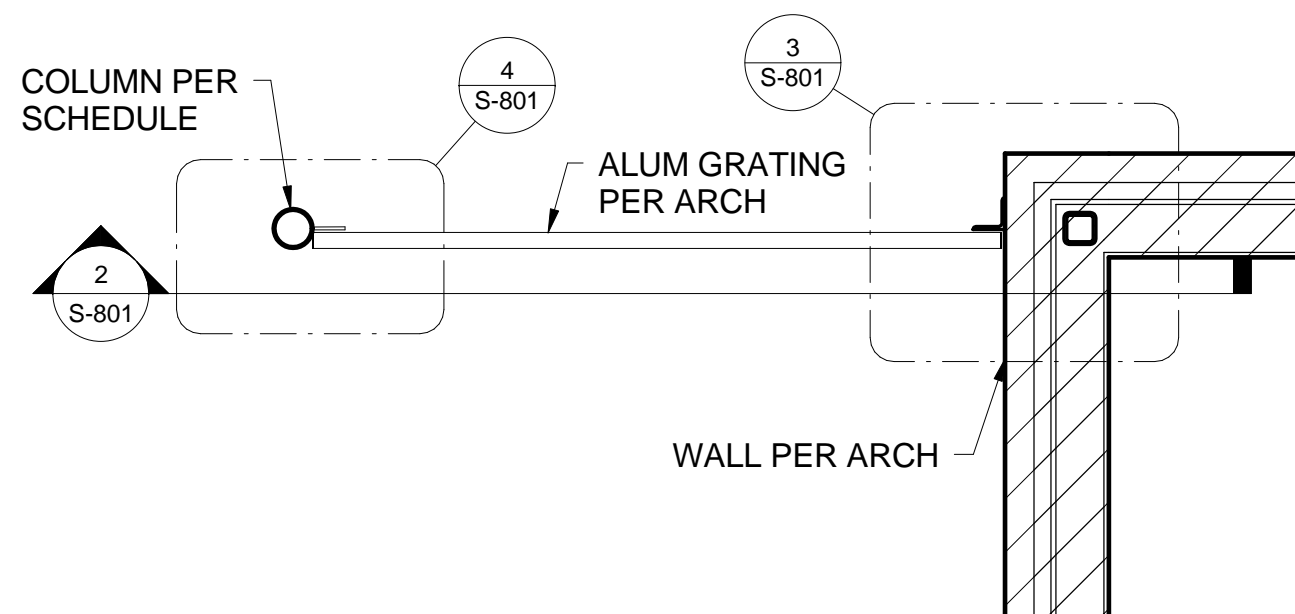
1 Area A Tilt-Up Wind Loading Plan
3/32" = 1'-0"

2 Area A Tilt-Up Seismic Loading Plan
3/32" = 1'-0"

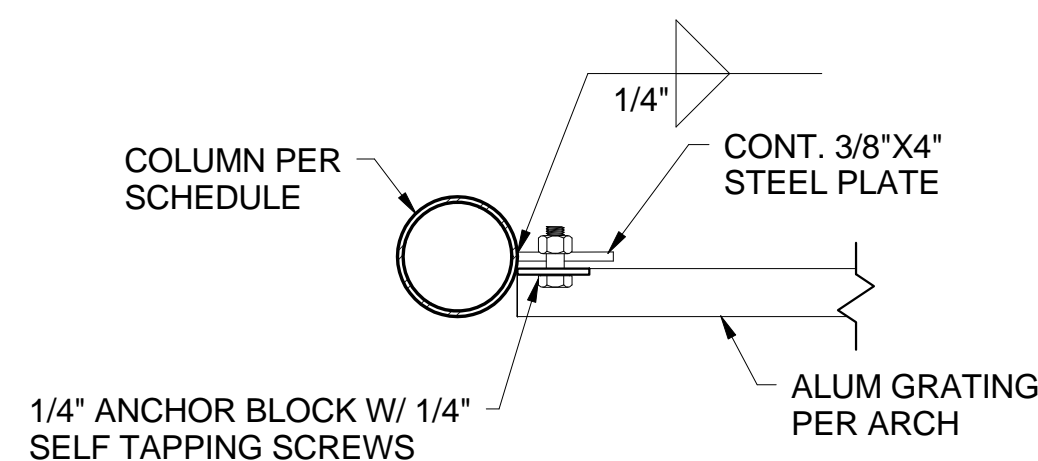
- GENERAL NOTES:**
- ALL LOADS ARE UNFACTORED SERVICE LOADS.
 - SEISMIC OVERSTRENGTH FACTORS HAVE NOT BEEN APPLIED.
 - ALL LOADS ARE REVERSIBLE.
 - LOADS SHOWN ARE MAXIMUM WHICH MAY OCCUR ALONG EACH WALL ELEMENT SEPARATELY. ALL WALL PANELS AND CONNECTIONS SHALL BE DESIGNED FOR WORST CASE COMBINATION.

- LEGEND:**
- (A) = ROOF DECK @ AUDITORIUM
 - (U) = ROOF DECK ABOVE SET CONSTRUCTION
 - (2) = 2nd FLOOR EAST OF AUDITORIUM (444'-8")
 - (C) = ROOF DECK ABOVE CORRIDOR / KIDDIE CITY / CHOIR (APPROXIMATE ELEV. SHOWN)

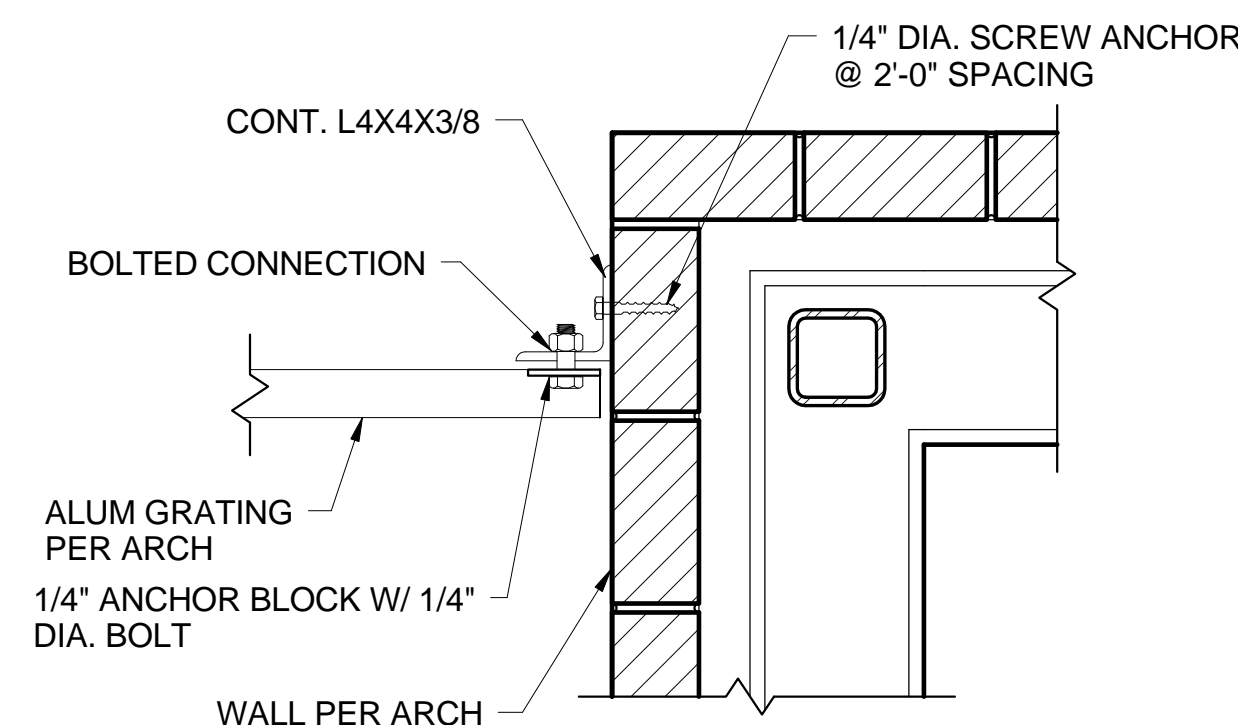




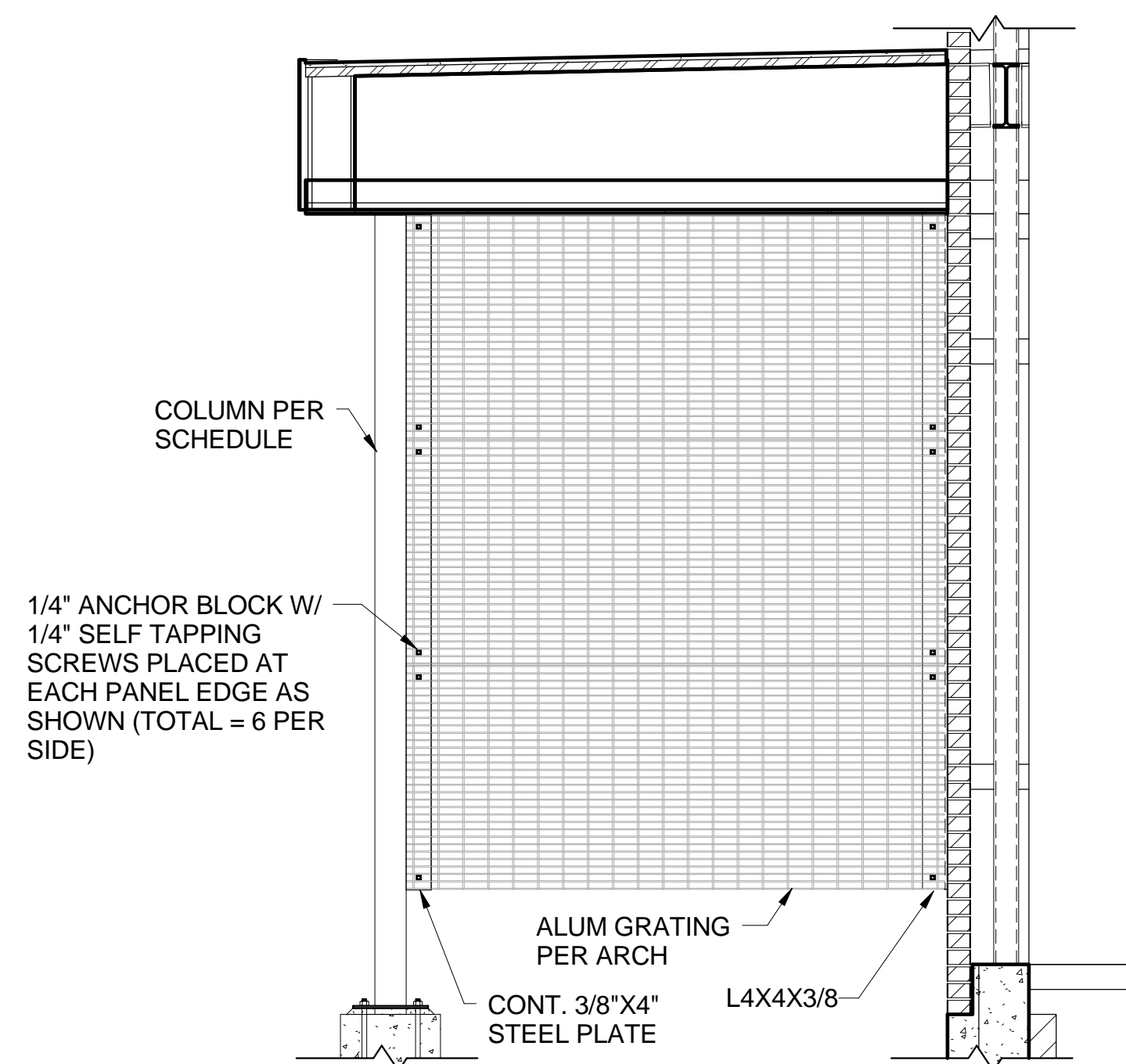
1 Sunshade Attachment Detail
1/2" = 1'-0"



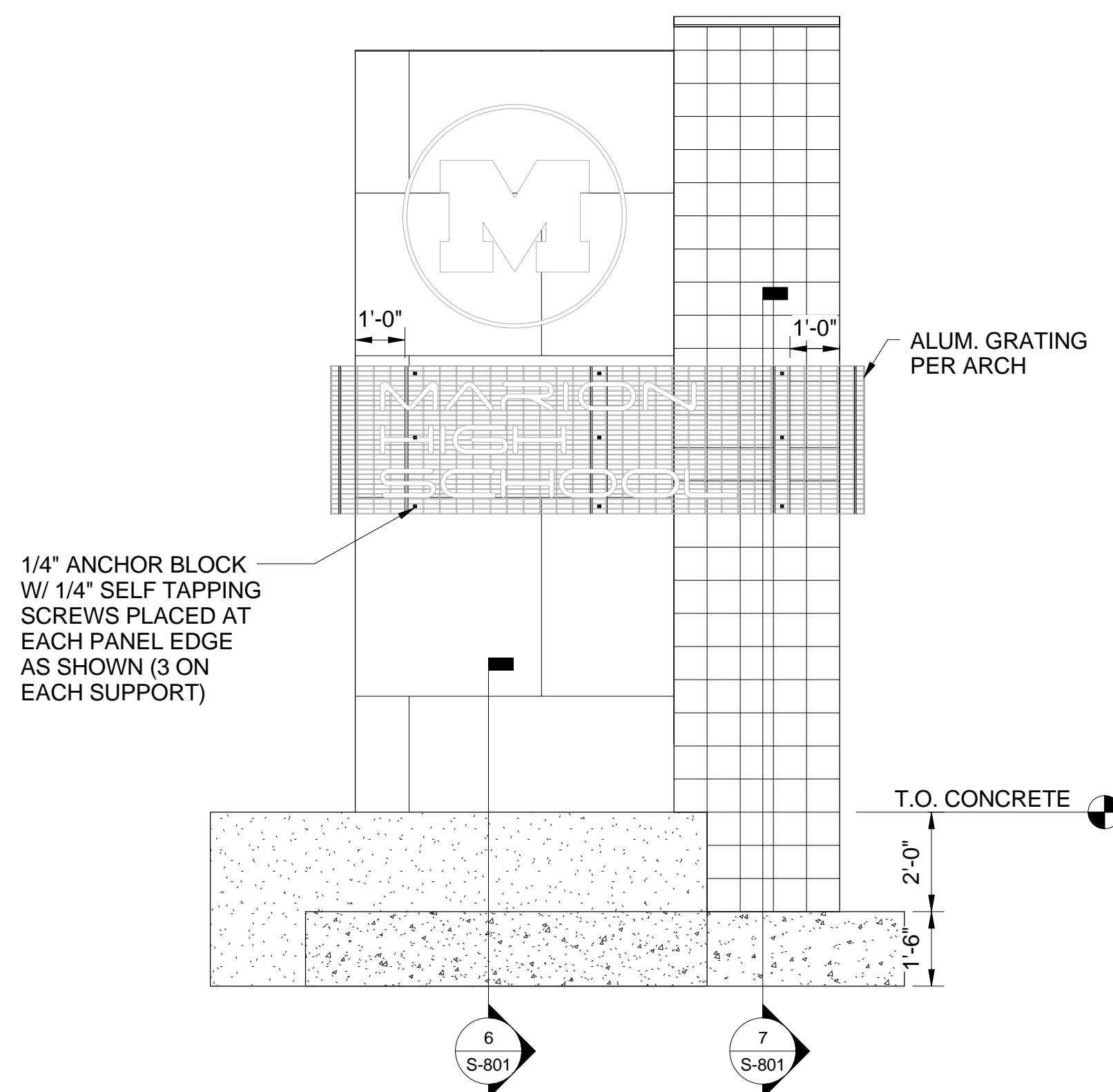
4 Sunshade Attachment Detail
1 1/2" = 1'-0"



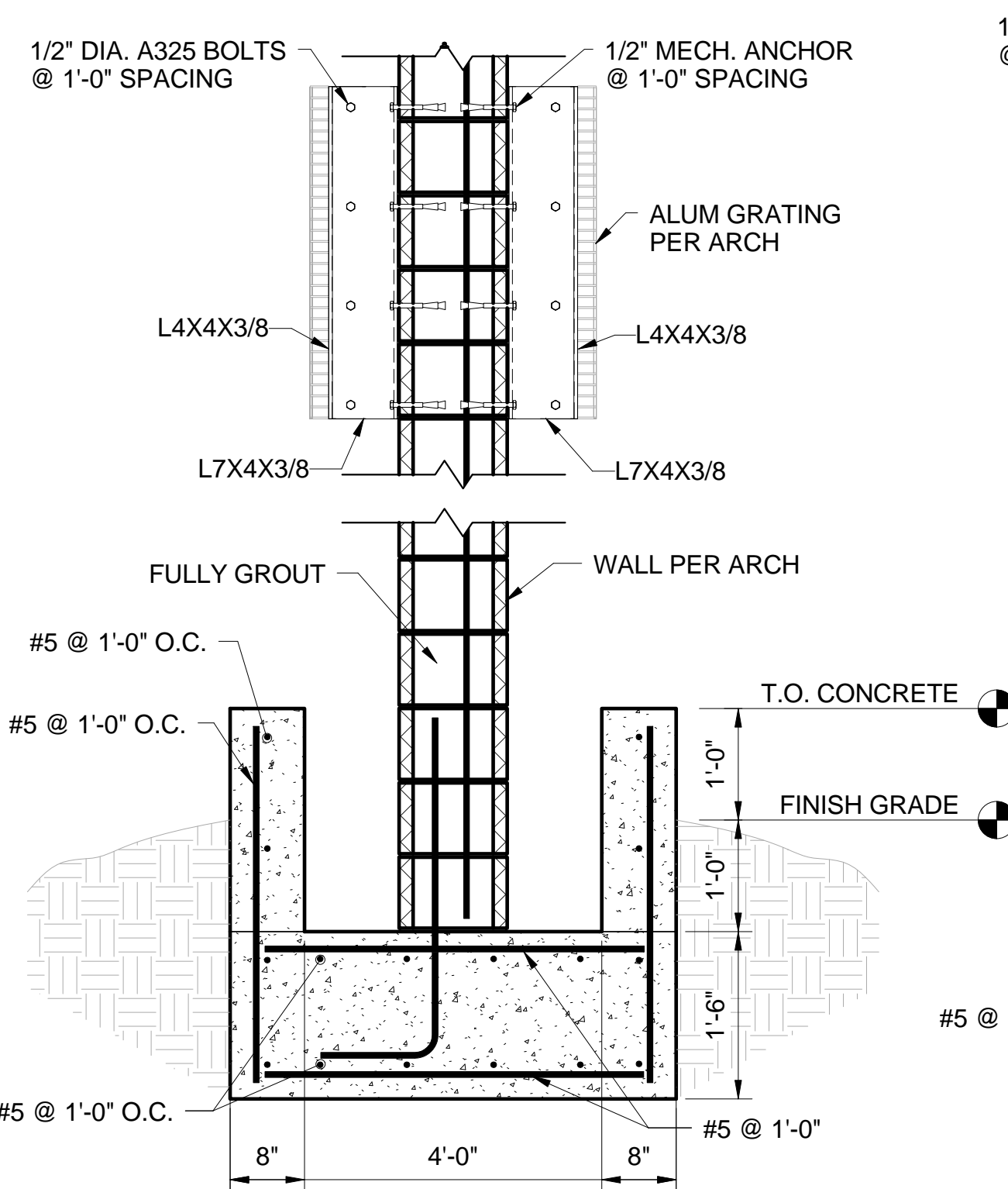
3 Sunshade Attachment Detail
1 1/2" = 1'-0"



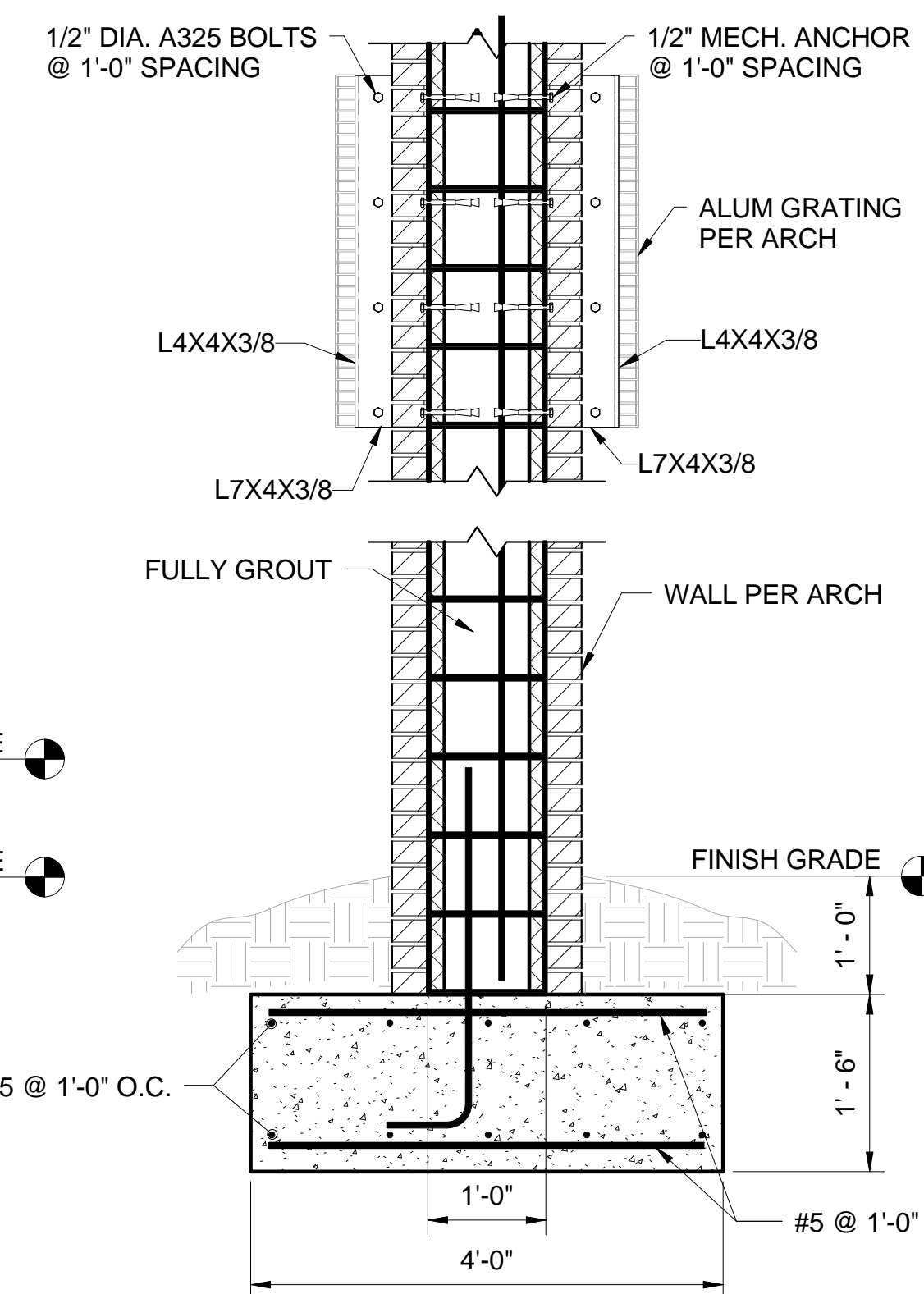
2 Typical Sunshade Elevation
1/2" = 1'-0"



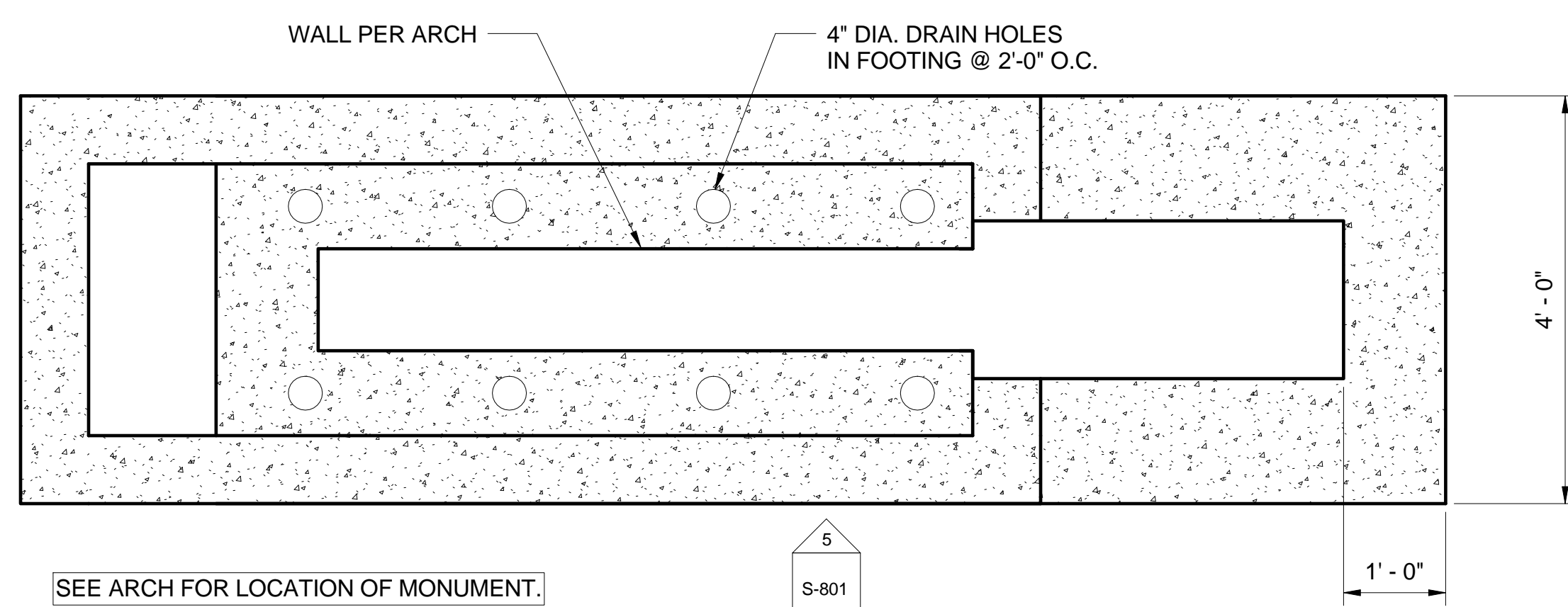
5 Monument Elevation
3/8" = 1'-0"



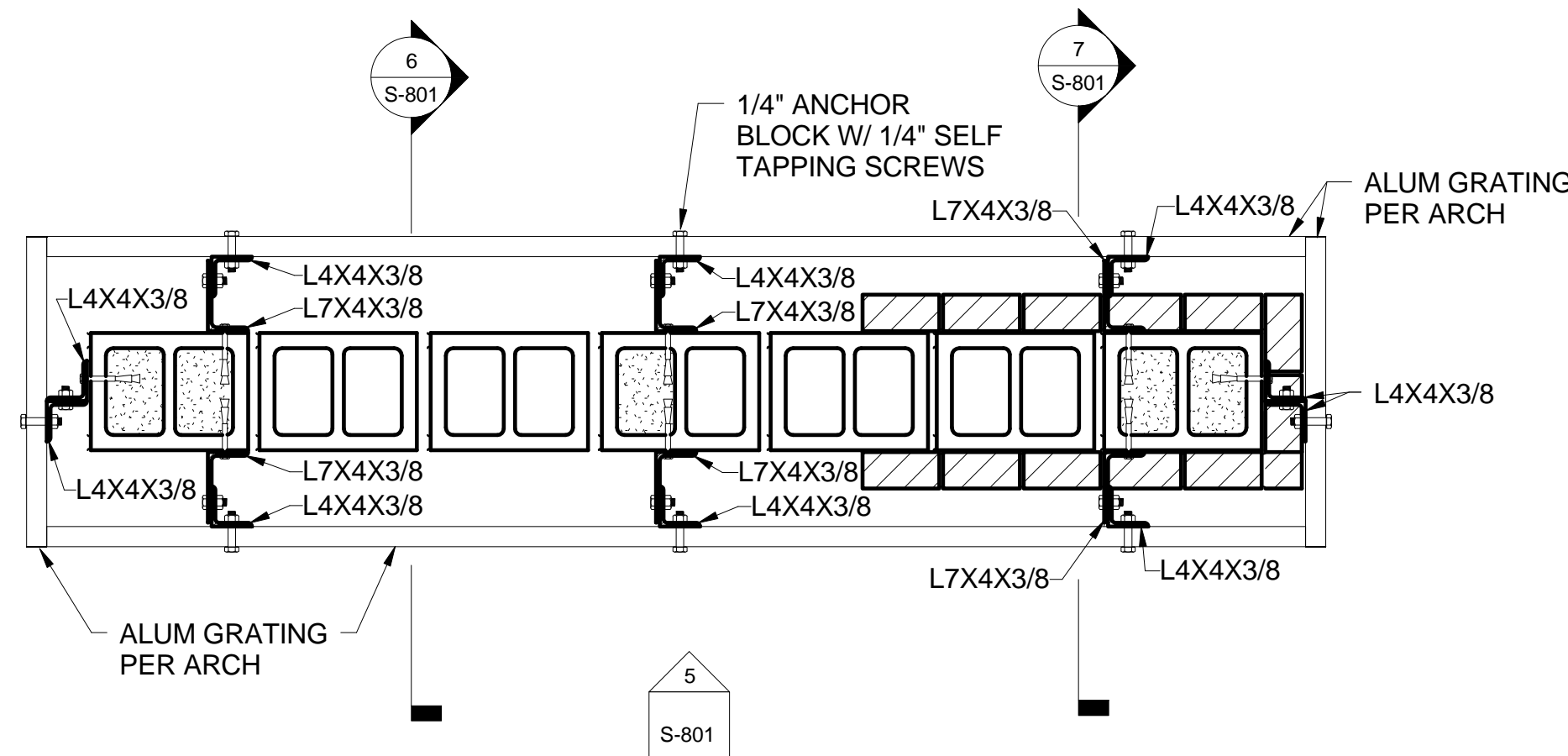
6 Monument Section
3/4" = 1'-0"



7 Monument Section
3/4" = 1'-0"



9 Monument Foundation Plan
3/4" = 1'-0"



8 Monument Plan
3/4" = 1'-0"

GENERAL NOTES:

- SEE ARCHITECTURAL SHEETS FOR DIMENSIONS, DETAILS, AND MISCELLANEOUS NOTES NOT SHOWN HERE.
- ALL EXPOSED STEEL FOR THE SUNSHADE AND SIGN FRAMING TO BE ELECTROSTATIC PAINTED. SEE ARCHITECTURAL.
- FOUNDATION REINFORCEMENT HOOKED BAR DEVELOPMENT LENGTH AS FOLLOWS U.N.O.:
#5 BAR.....1' - 0"
#7 BAR.....1' - 6"

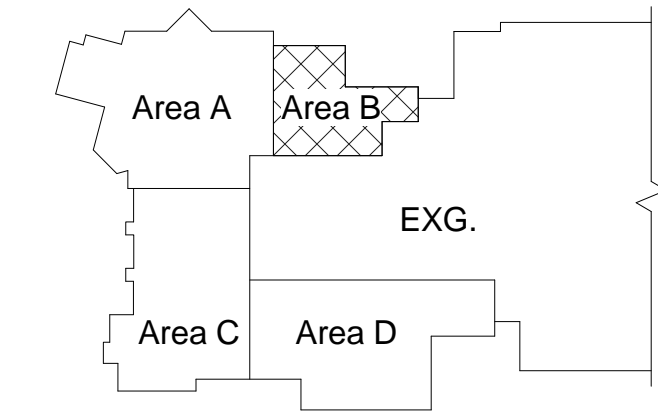
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MISC. DETAILS



1 Demo Plan - Area B
1/8" = 1'-0"



KEYPLAN

NOTES:

- 1. SEE A-SHEETS FOR SAWCUT AND DEMO LOCATIONS.

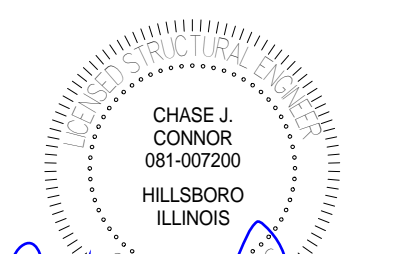
DAI
DESIGN ARCHITECTS, INC.
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200 N. MARKET STREET
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SPRINGFIELD, IL
ARNOLD, MO
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Chase Connor
SIGNATURE

03/19/15

DATE 11-30-2016

LICENSE EXPIRES

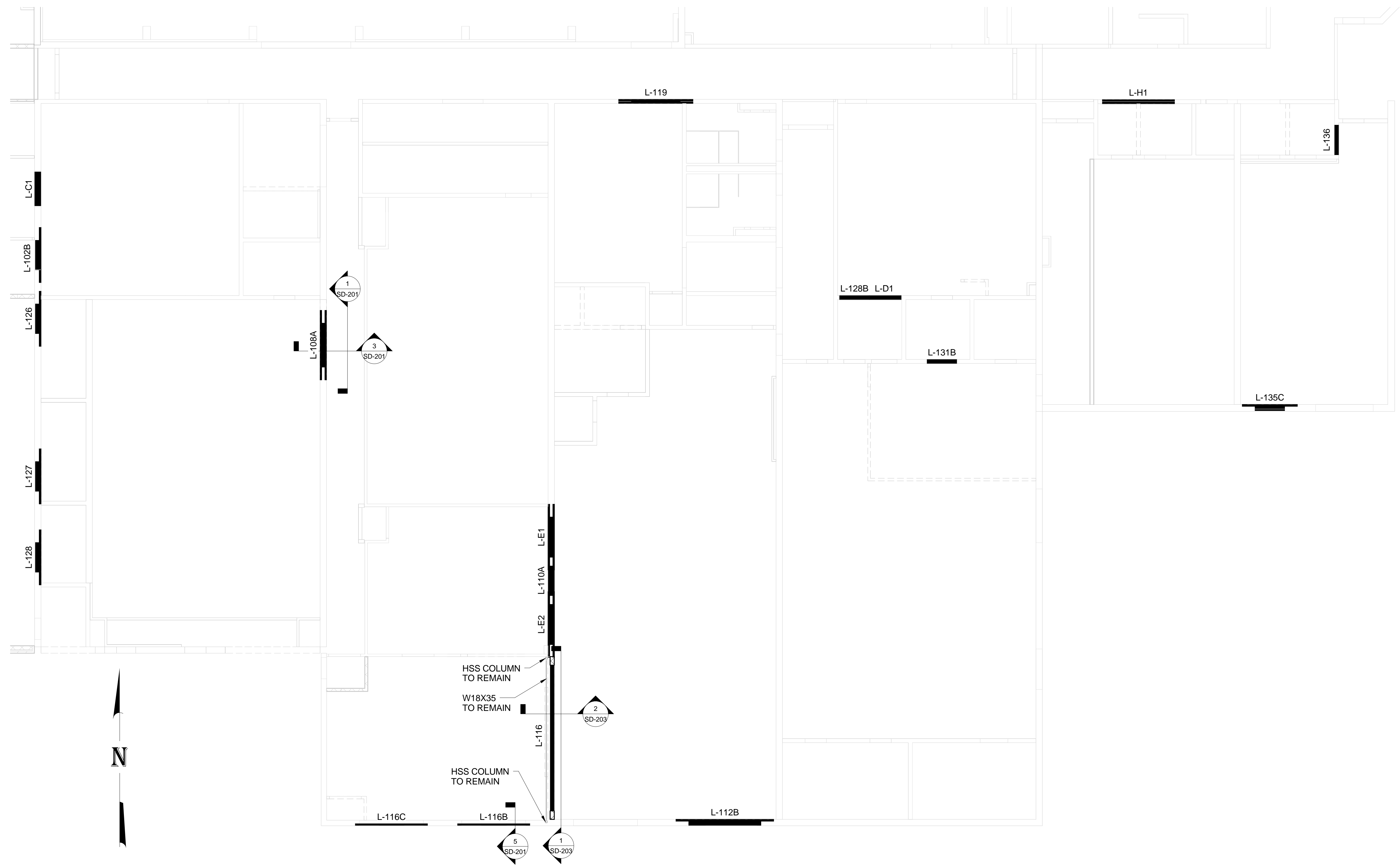
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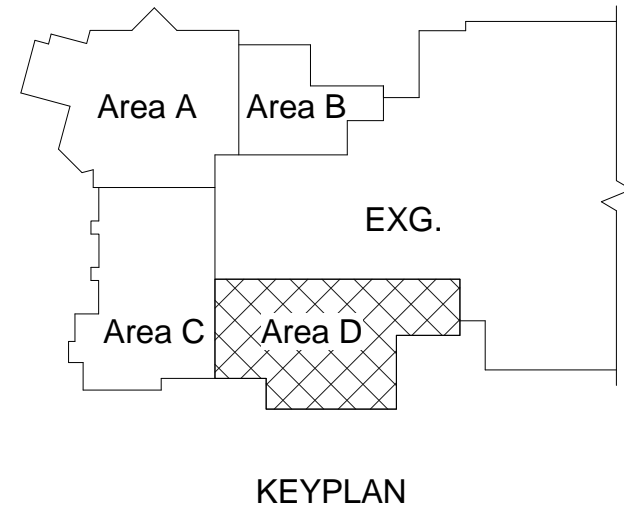
DATE: 03/19/15
PROJECT NO: 360-2632
DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

AREA B DEMO PLAN

SD-101



1 Demo Plan - Area D
1/8" = 1'-0"



- NOTES:
- SEE A-SHEETS FOR SAWCUT AND DEMO LOCATIONS.

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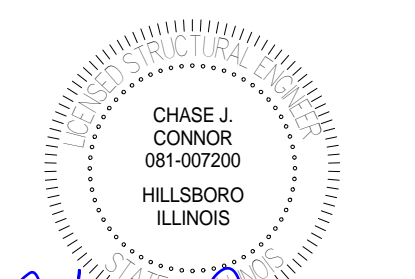
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03/19/15
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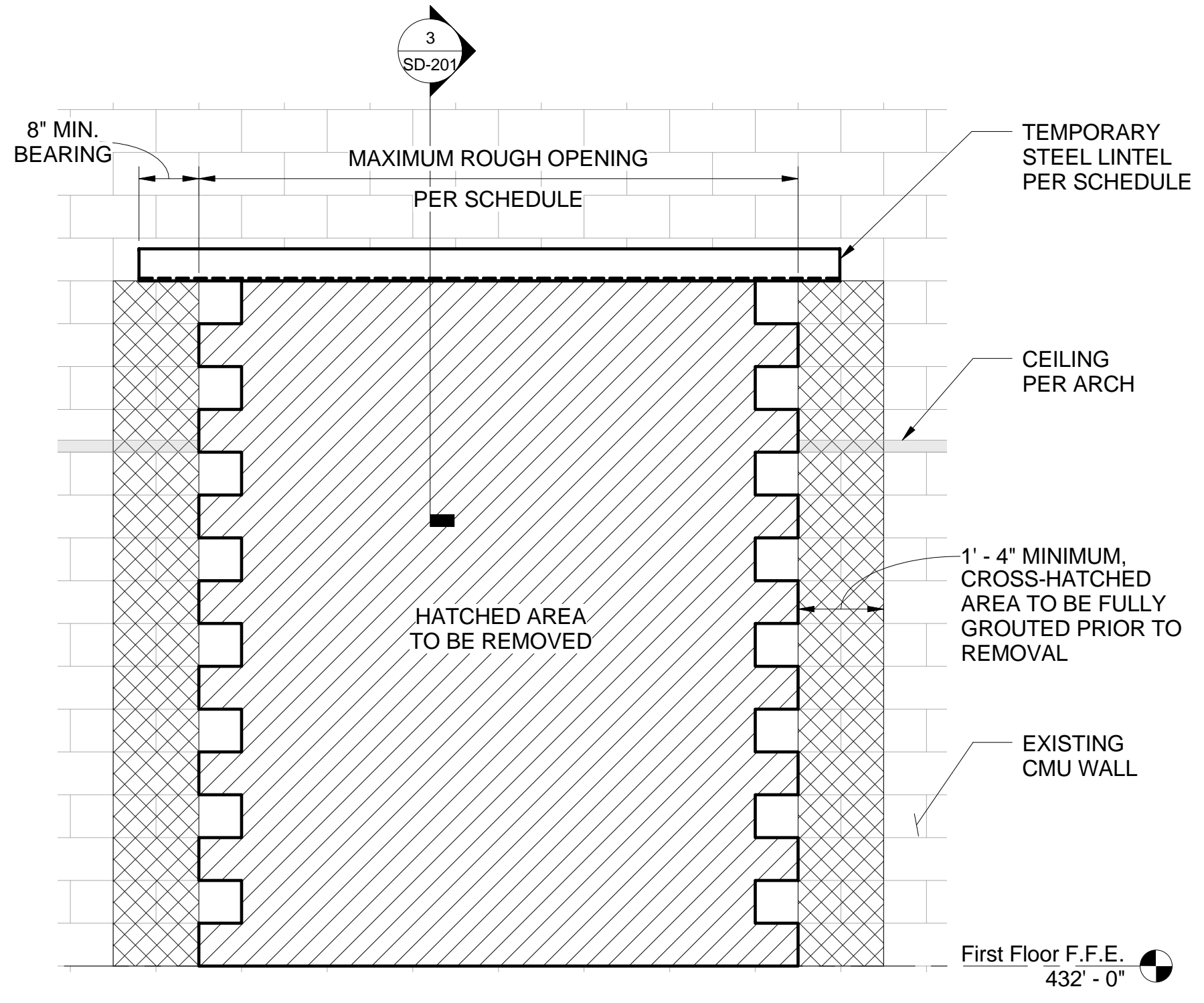
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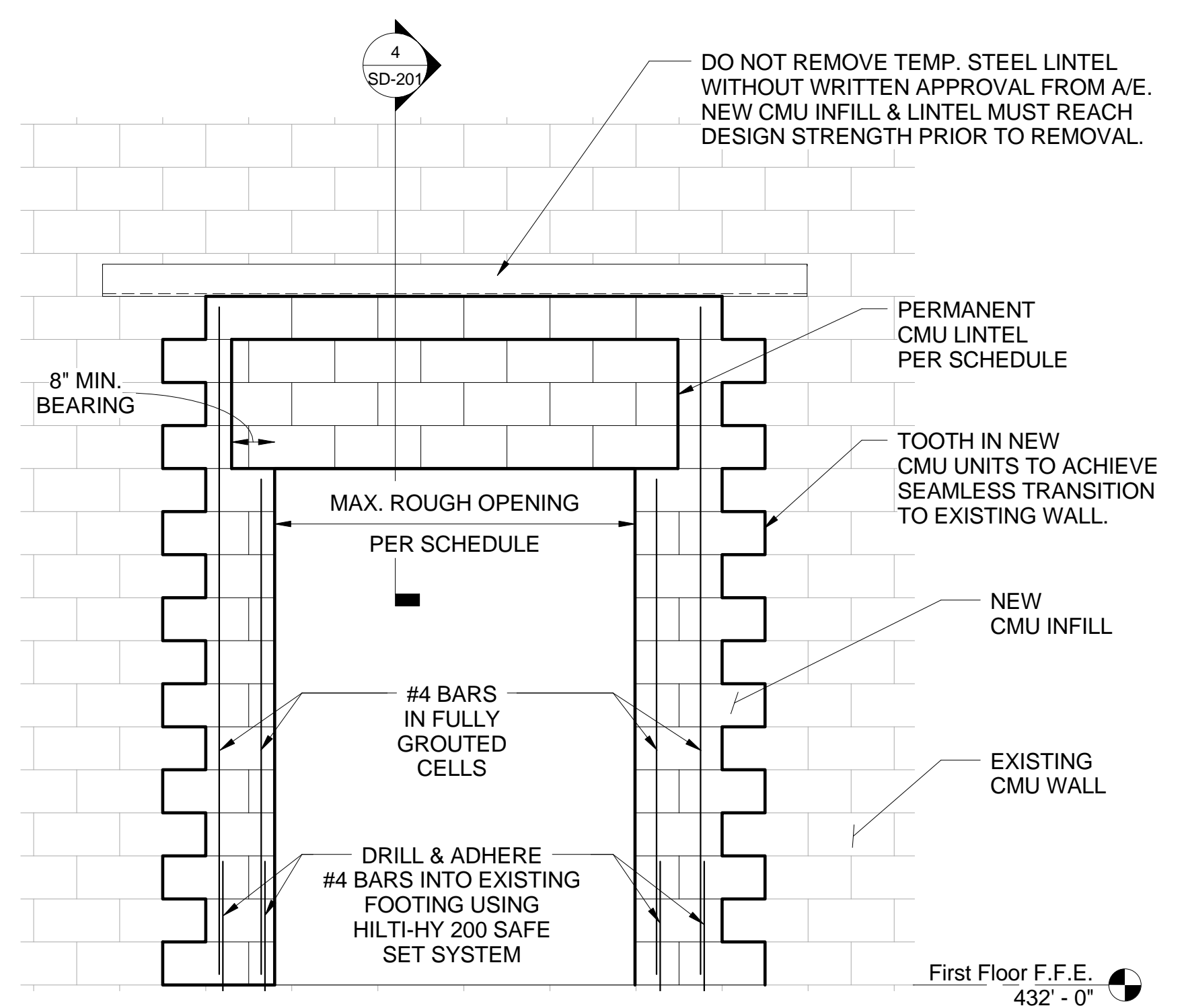
DATE: 03/19/15
PROJECT NO: 360-2632
DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

AREA D DEMO PLAN

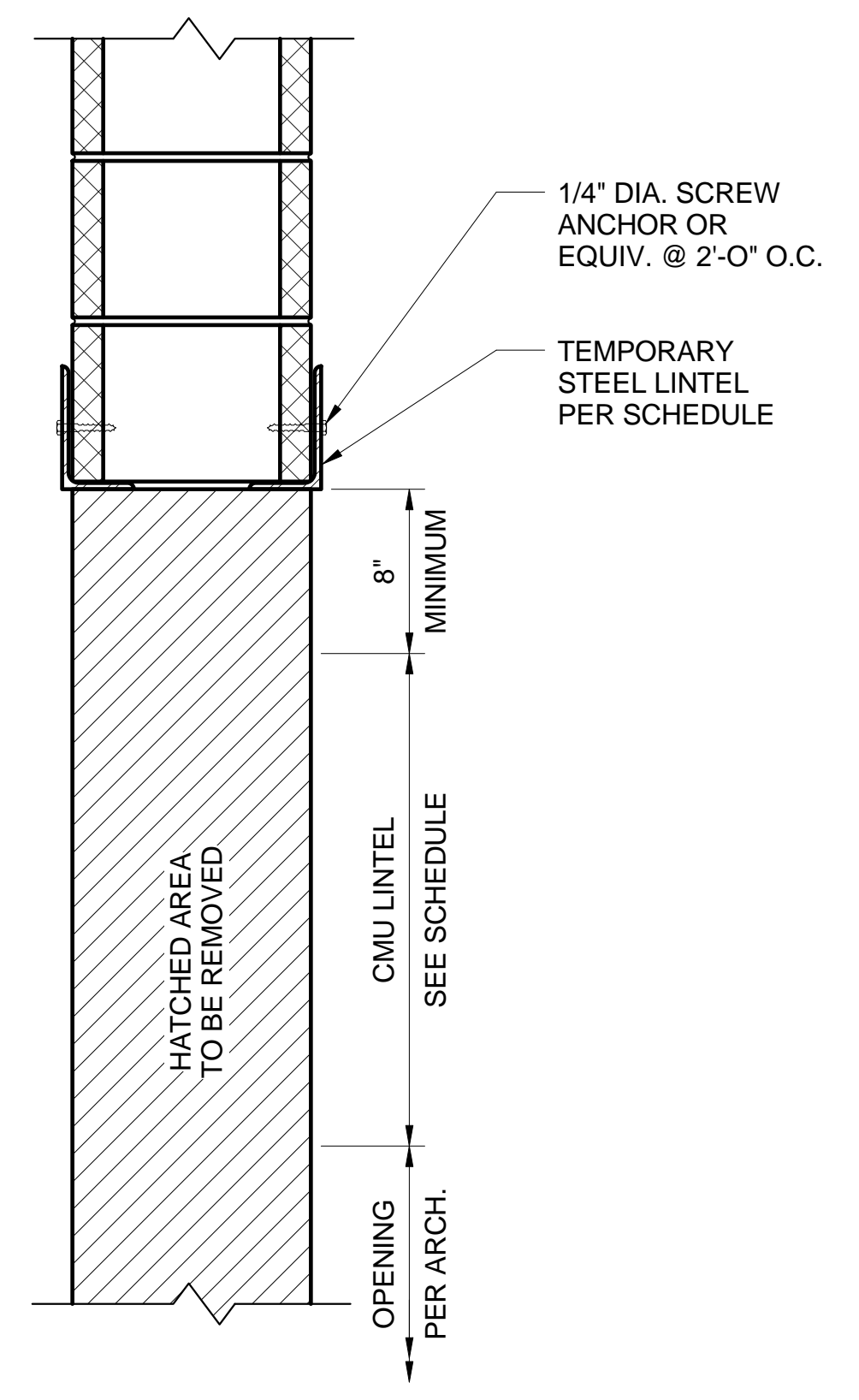
SD-102



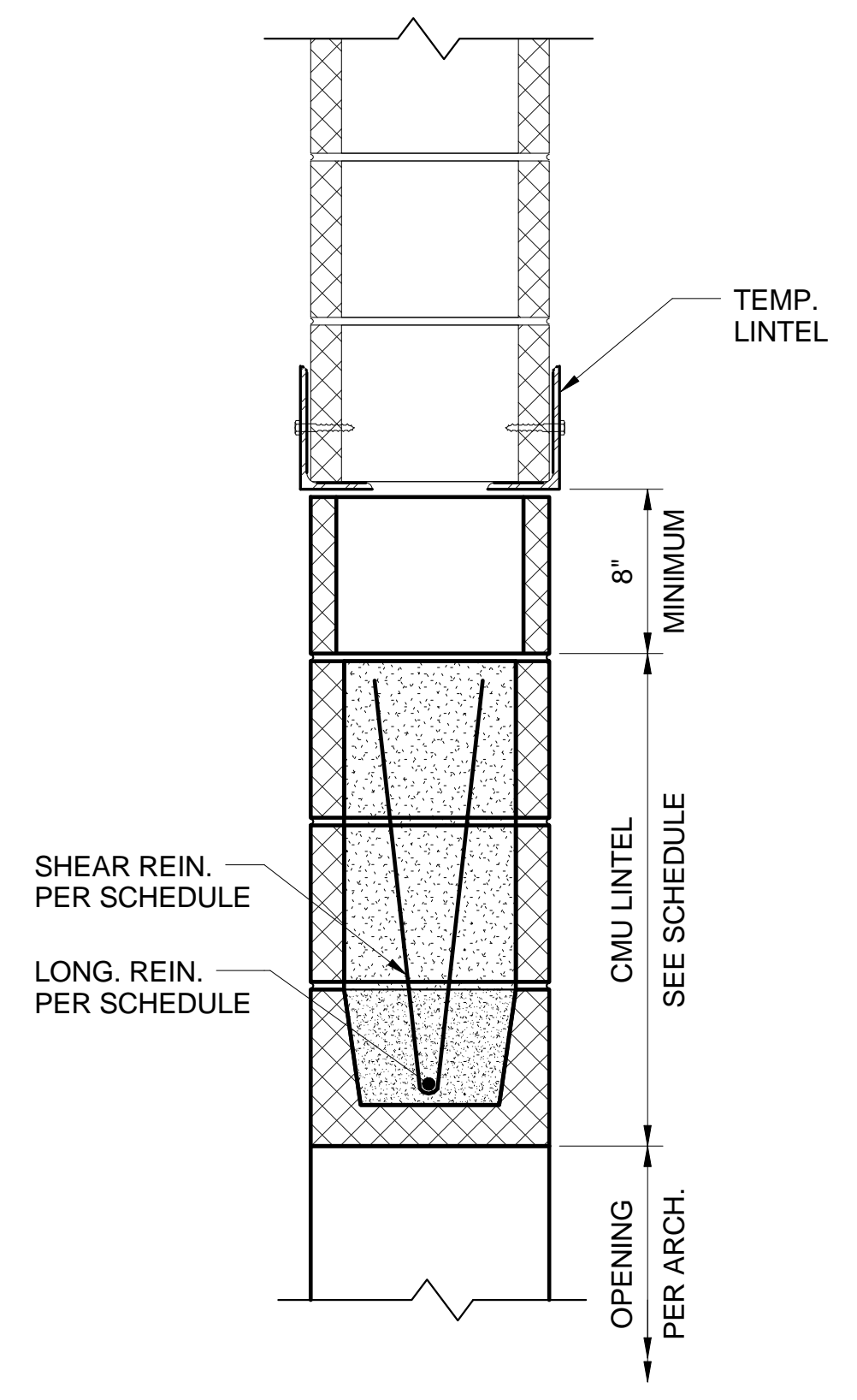
1 CMU Wall Demolition
1/2" = 1'-0"



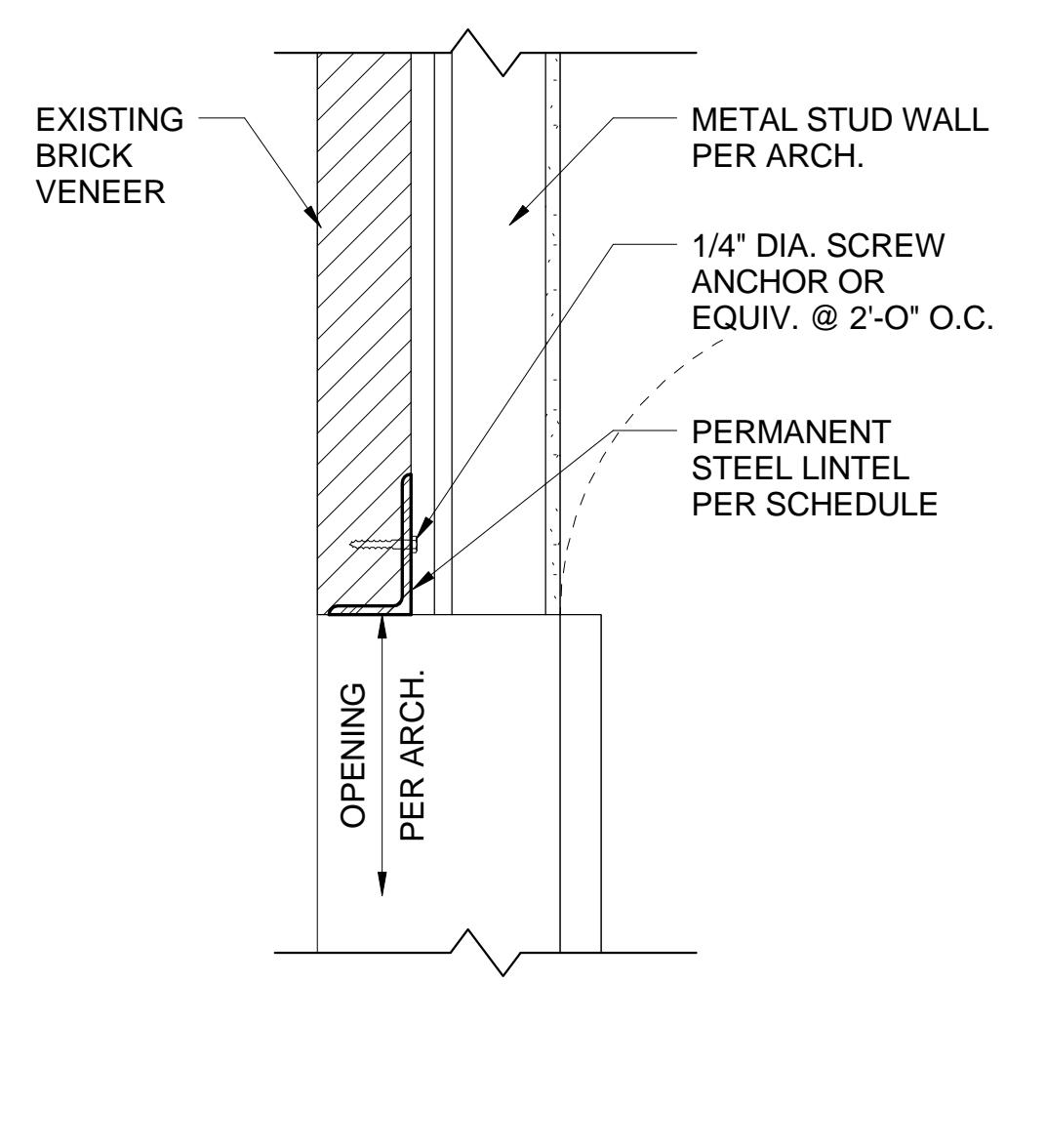
2 CMU Renovation
1/2" = 1'-0"



3 CMU Wall Demolition
1 1/2" = 1'-0"



4 CMU Renovation
1 1/2" = 1'-0"



5 Brick Lintel
1 1/2" = 1'-0"

SINGLE-WYTHE LINTEL SCHEDULE							
Reference Mark	Rough Opening	Wall Type	Quantity	Lintel	Temporary or Permanent	Longitudinal Reinforcement	Shear Reinforcement
L-105B	6' - 4"	8" CMU	1	8 x 16 CMU Lintel	Permanent	1 - #4	-
L-108A	9' - 7"	12" CMU	2	L6X3-1/2X5/16	Temporary		
L-108A	5' - 7"	12" CMU	1	12 x 24 CMU Lintel	Permanent	1 - #5	-
L-110A	3' - 4"	12" CMU	1	12 x 16 CMU Lintel	Permanent	1 - #5	-
L-116B	10' - 0"	4" Brick	1	L6X3-1/2X3/8	Permanent		
L-116C	10' - 0"	4" Brick	1	L6X3-1/2X3/8	Permanent		
L-119	10' - 4"	8" CMU	2	L6X3-1/2X5/16	Temporary		
L-119	6' - 4"	8" CMU	1	8 x 16 CMU Lintel	Permanent	1 - #4	-
L-128B	3' - 4"	8" CMU	1	8 x 16 CMU Lintel	Permanent	1 - #4	-
L-131B	3' - 4"	8" CMU	1	8 x 16 CMU Lintel	Permanent	1 - #4	-
L-136	3' - 4"	8" CMU	1	8 x 8 CMU Lintel	Permanent	1 - #4	-
L-C1	4' - 0"	12" CMU	1	12 x 16 CMU Lintel	Permanent	1 - #5	-
L-D1	4' - 0"	8" CMU	1	8 x 16 CMU Lintel	Permanent	1 - #4	-
L-E1	9' - 0"	12" CMU	2	L6X3-1/2X5/16	Temporary		
L-E1	5' - 0"	12" CMU	1	12 x 24 CMU Lintel	Permanent	1 - #5	-
L-E2	9' - 0"	12" CMU	2	L6X3-1/2X5/16	Temporary		
L-E2	5' - 0"	12" CMU	1	12 x 24 CMU Lintel	Permanent	1 - #5	-
L-H1	10' - 0"	8" CMU	2	L6X3-1/2X5/16	Temporary		
L-H1	6' - 0"	8" CMU	1	8 x 16 CMU Lintel	Permanent	1 - #4	-

- LINTEL SCHEDULE NOTES:
- ALL ANGLES SHALL BE PLACED LONG LEG VERTICAL.
 - ALL STEEL LINTELS LOCATED IN EXTERIOR WALLS SHALL BE HOT DIP GALVANIZED.
 - SEE SHEETS S-001 & S-002 FOR GENERAL STRUCTURAL NOTES.
 - SEE SHEETS S-401 & S-402 FOR ADDITIONAL MASONRY INFORMATION NOT SHOWN HERE.

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CONCOR
081-007200
HILLSBORO
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LICENSE EXPIRES

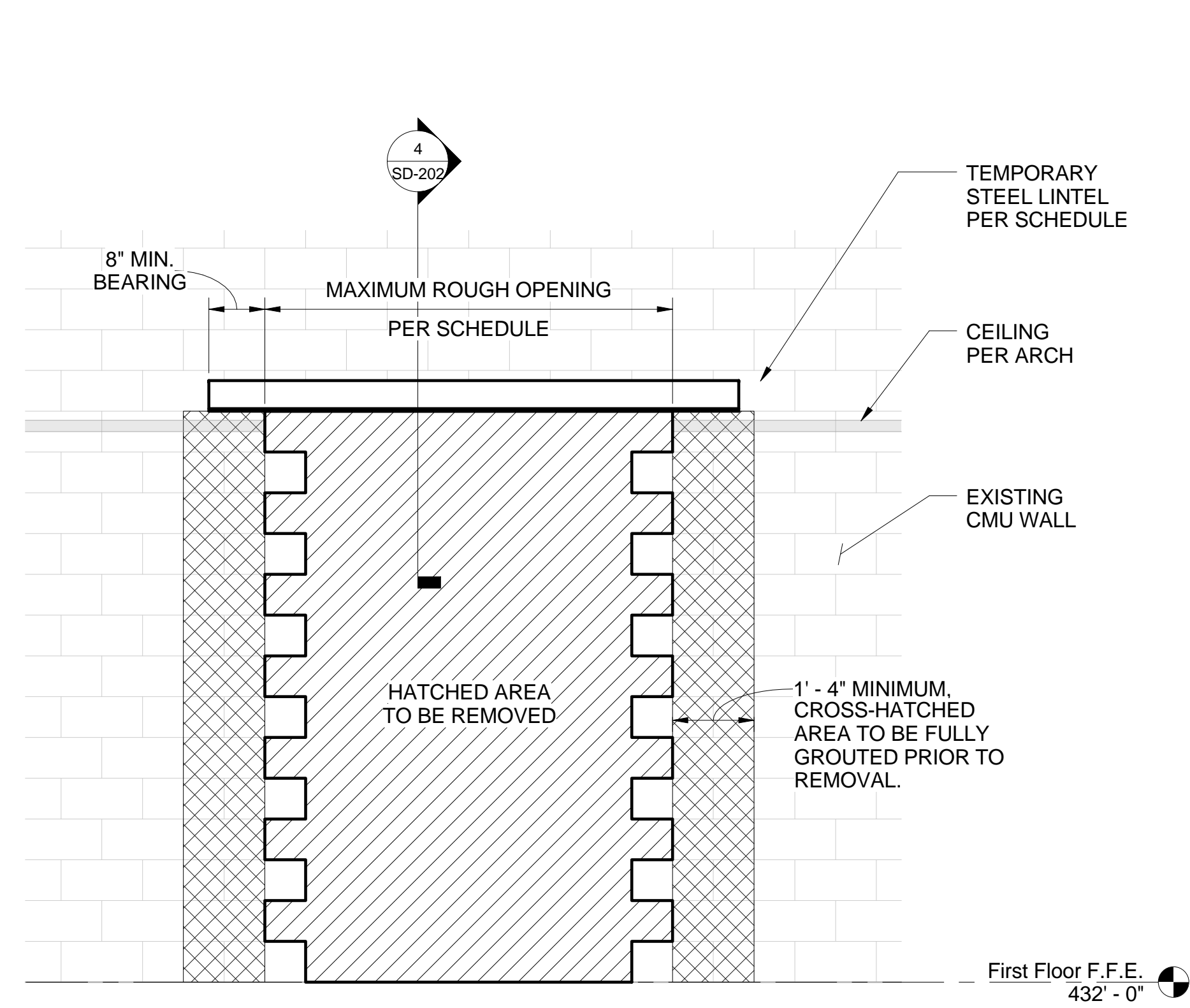
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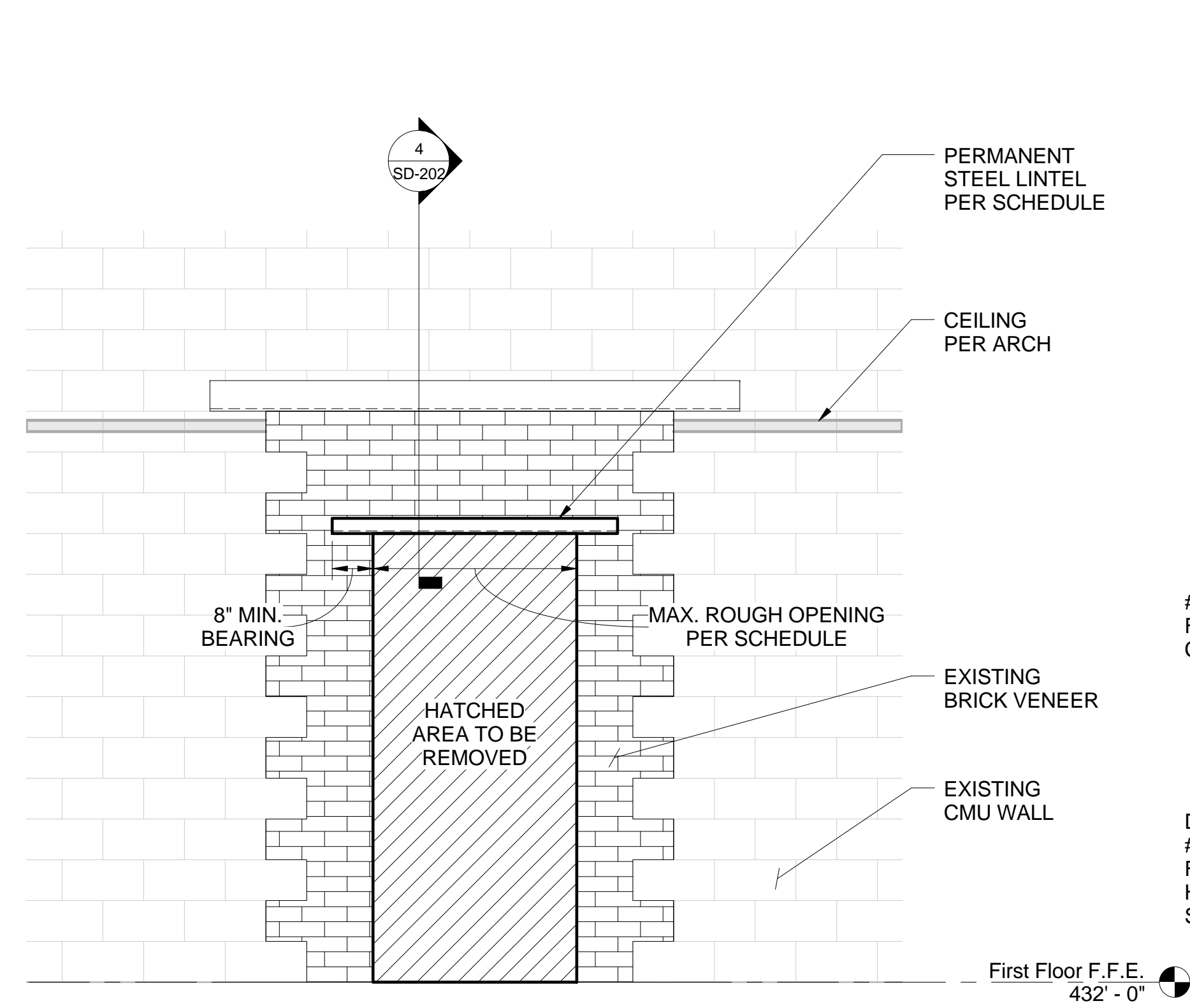
DATE: 03/19/15
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DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

WALL MODIFICATIONS
SINGLE-WYTHE
WALLS

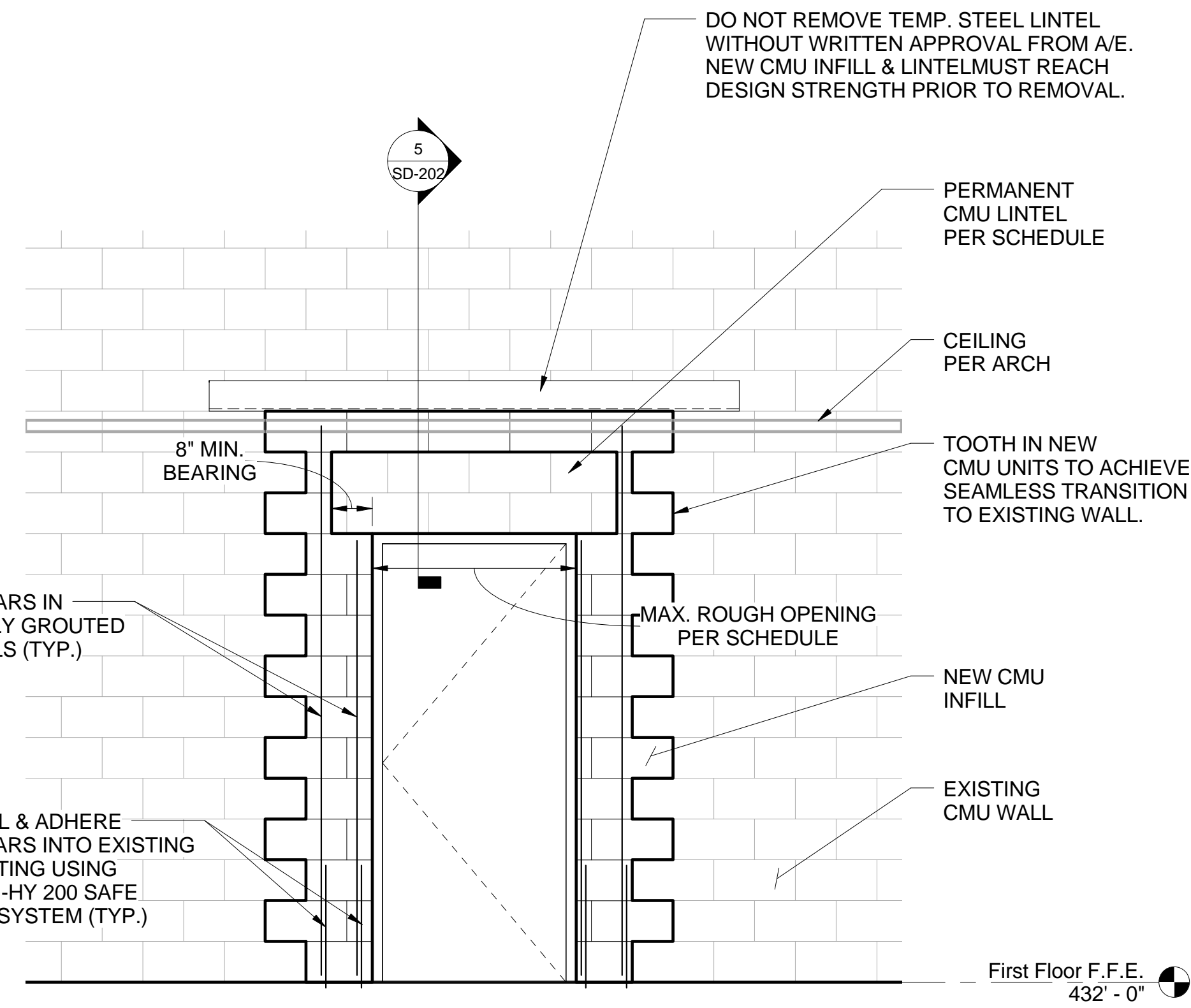
SD-201



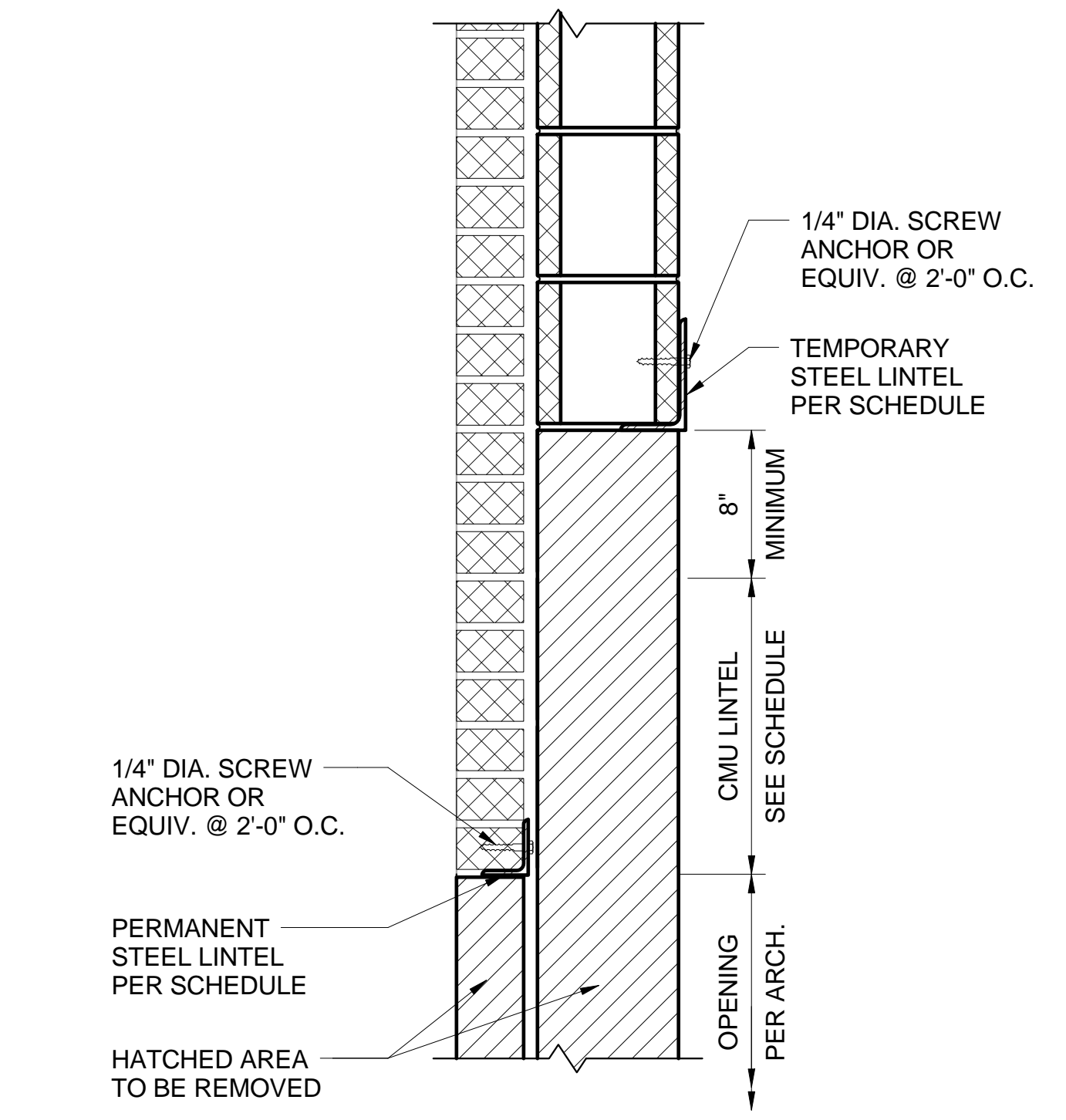
1 CMU Wall Demolition
1/2" = 1'-0"



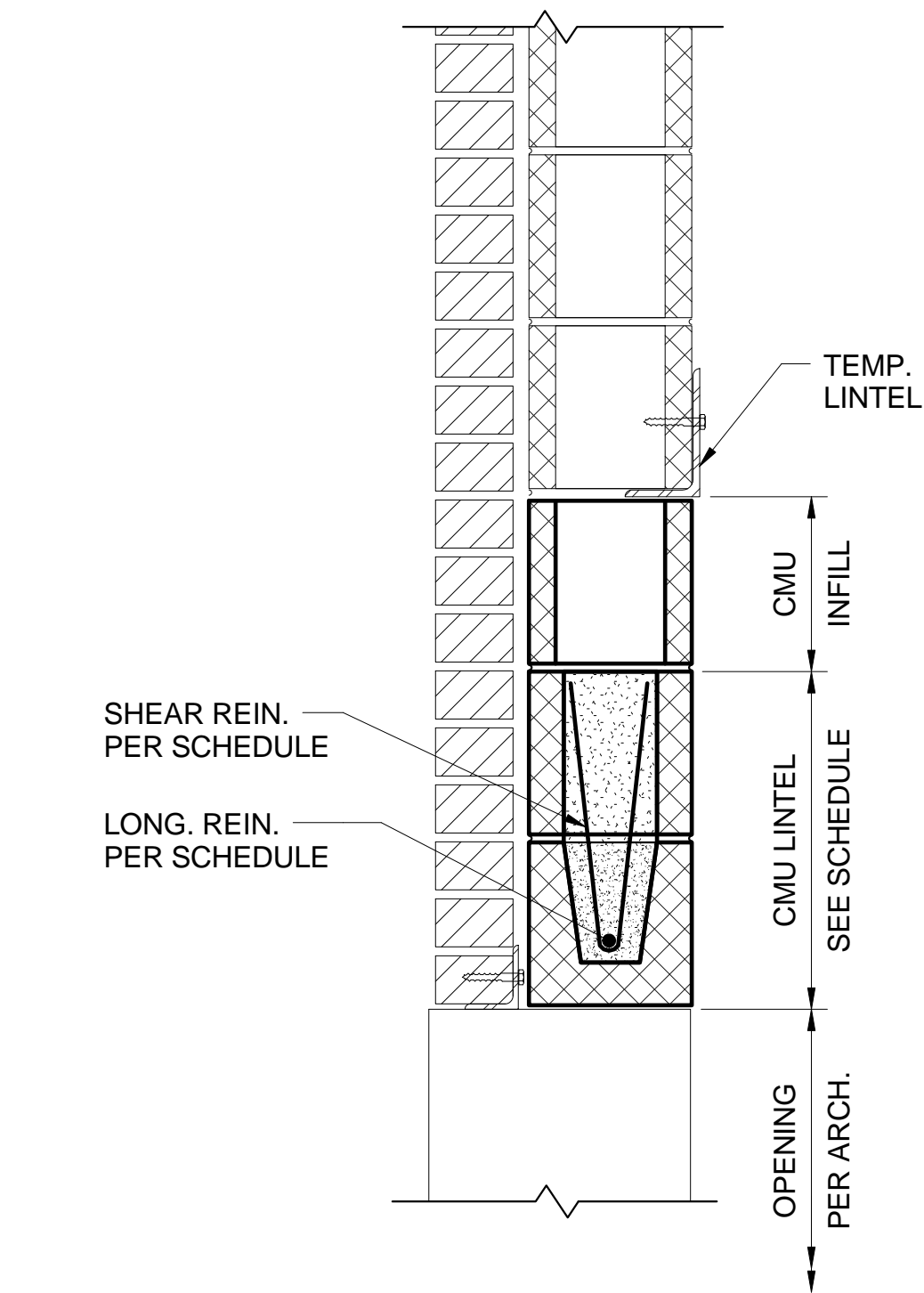
2 Brick Veneer Demolition
1/2" = 1'-0"



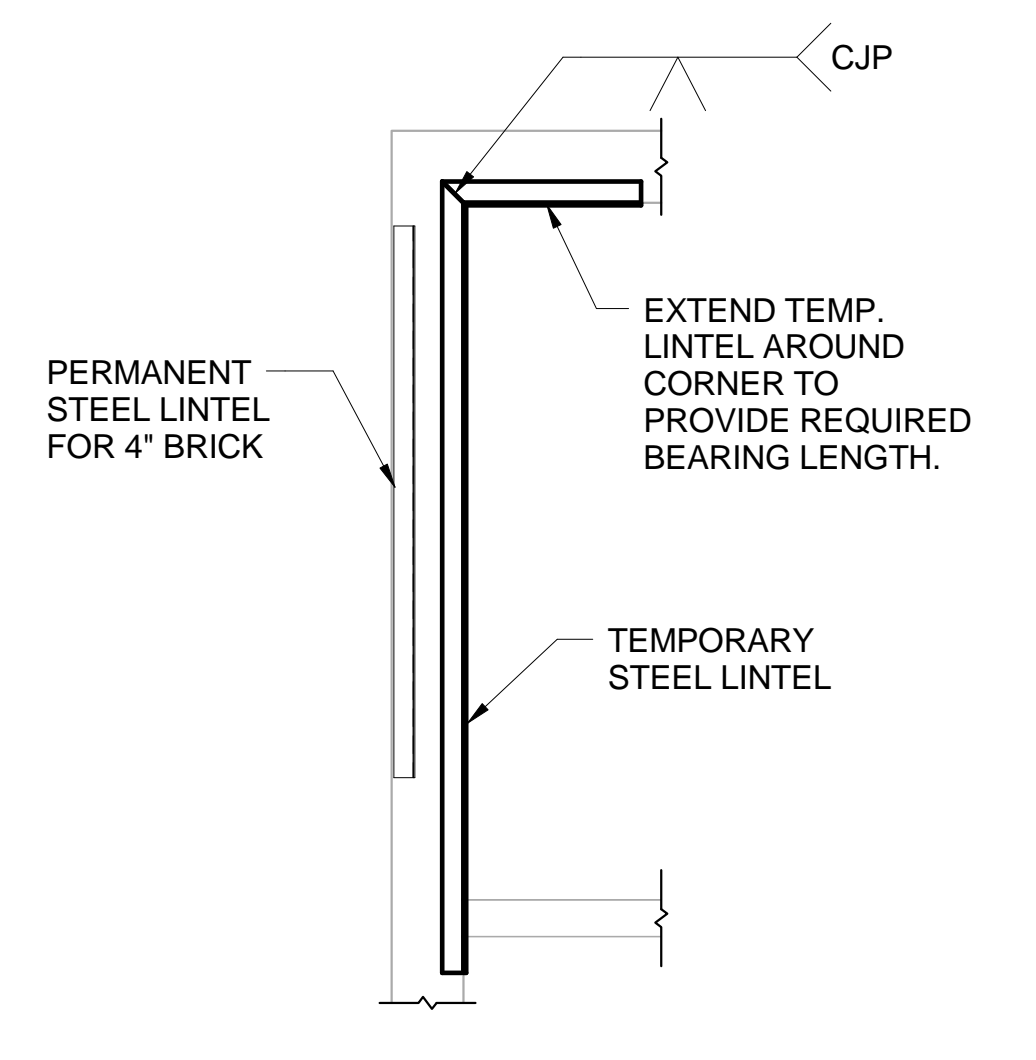
3 CMU Renovation
1/2" = 1'-0"



4 Steel Lintel Section
1 1/2" = 1'-0"



5 Lintel Section
1 1/2" = 1'-0"



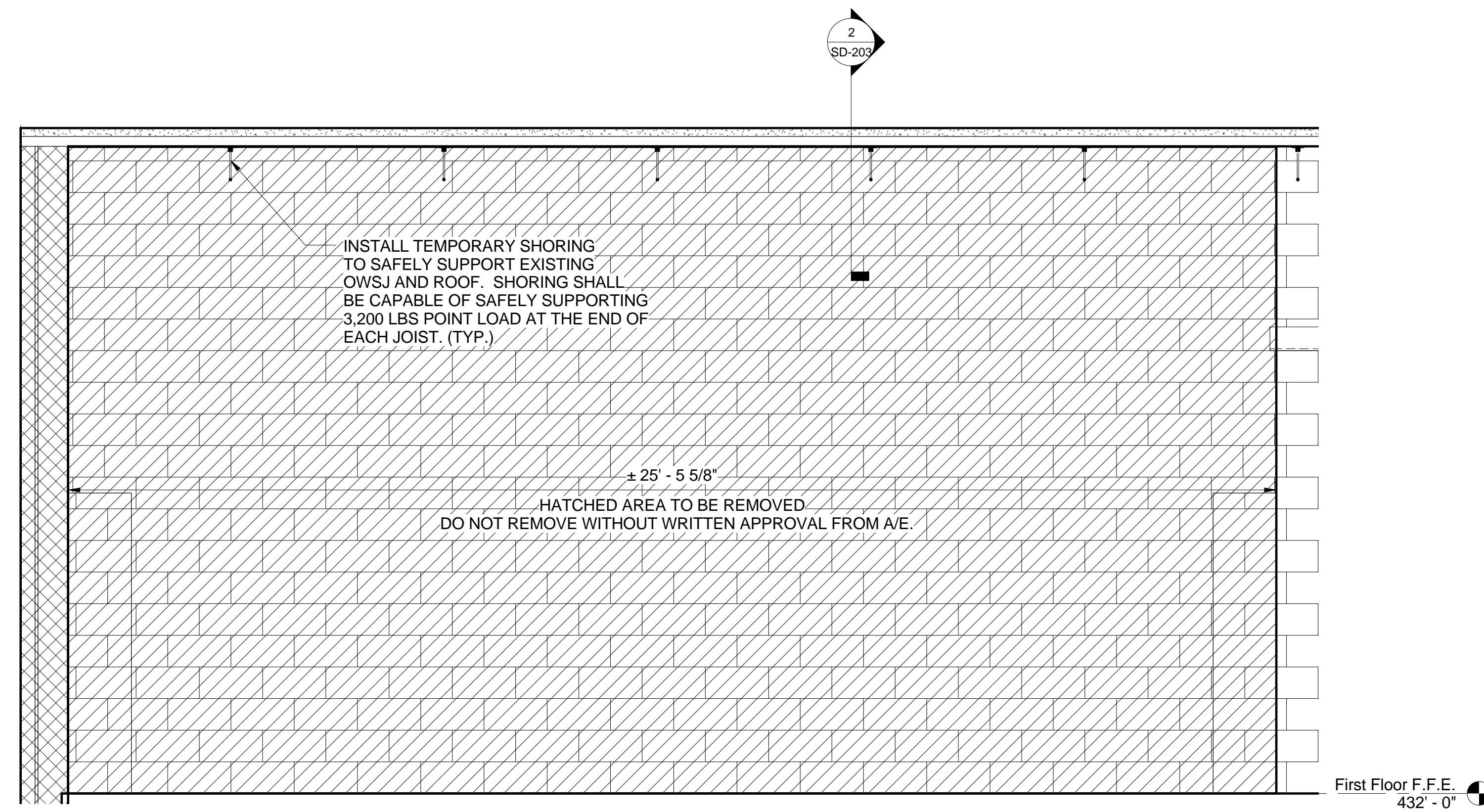
6 Plan Detail for L-105A
3/8" = 1'-0"

MULTI-WYTHE LINTEL SCHEDULE						
Reference Mark	Rough Opening	Wall Type	Lintel	Temporary or Permanent	Longitudinal Reinforcement	Shear Reinforcement
L-102B	7' - 4"	8" CMU	L8X4X7/16	Temporary		
L-102B	3' - 4"	4" Brick	L3X2-1/2X1/4	Permanent		
L-102B	3' - 4"	8" CMU	8 x 16 CMU Lintel	Permanent	1 - #4	-
L-103	7' - 4"	8" CMU	L6X3-1/2X5/16	Temporary		
L-103	3' - 4"	4" Brick	L3X2-1/2X1/4	Permanent		
L-103	3' - 4"	8" CMU	8 x 16 CMU Lintel	Permanent	1 - #4	-
L-104B	7' - 4"	8" CMU	L6X3-1/2X5/16	Temporary		
L-104B	3' - 4"	4" Brick	L3X2-1/2X1/4	Permanent		
L-104B	3' - 4"	8" CMU	8 x 16 CMU Lintel	Permanent	1 - #4	-
L-105A	10' - 4"	8" CMU	L8X4X7/16	Temporary		
L-105A	6' - 4"	4" Brick	L4X3-1/2X1/4	Permanent		
L-105A	6' - 4"	8" CMU	8 x 24 CMU Lintel	Permanent	1 - #4	-
L-112B	14' - 0"	8" CMU	L8X4X7/16	Temporary		
L-112B	10' - 0"	4" Brick	L6X3-1/2X3/8	Permanent		
L-112B	10' - 0"	8" CMU	8 x 16 CMU Lintel	Permanent	1 - #6	-
L-126	7' - 4"	8" CMU	L8X4X7/16	Temporary		
L-126	3' - 4"	4" Brick	L3X2-1/2X1/4	Permanent		
L-126	3' - 4"	8" CMU	8 x 16 CMU Lintel	Permanent	1 - #4	-
L-127	7' - 4"	8" CMU	L8X4X7/16	Temporary		
L-127	3' - 4"	4" Brick	L3X2-1/2X1/4	Permanent		
L-127	3' - 4"	8" CMU	8 x 16 CMU Lintel	Permanent	1 - #4	-
L-128	7' - 4"	8" CMU	L8X4X7/16	Temporary		
L-128	3' - 4"	4" Brick	L3X2-1/2X1/4	Permanent		
L-128	3' - 4"	8" CMU	8 x 16 CMU Lintel	Permanent	1 - #4	-
L-135C	7' - 4"	8" CMU	L8X4X7/16	Temporary		
L-135C	3' - 4"	4" Brick	L3X2-1/2X1/4	Permanent		
L-135C	3' - 4"	8" CMU	8 x 8 CMU Lintel	Permanent	1 - #4	-

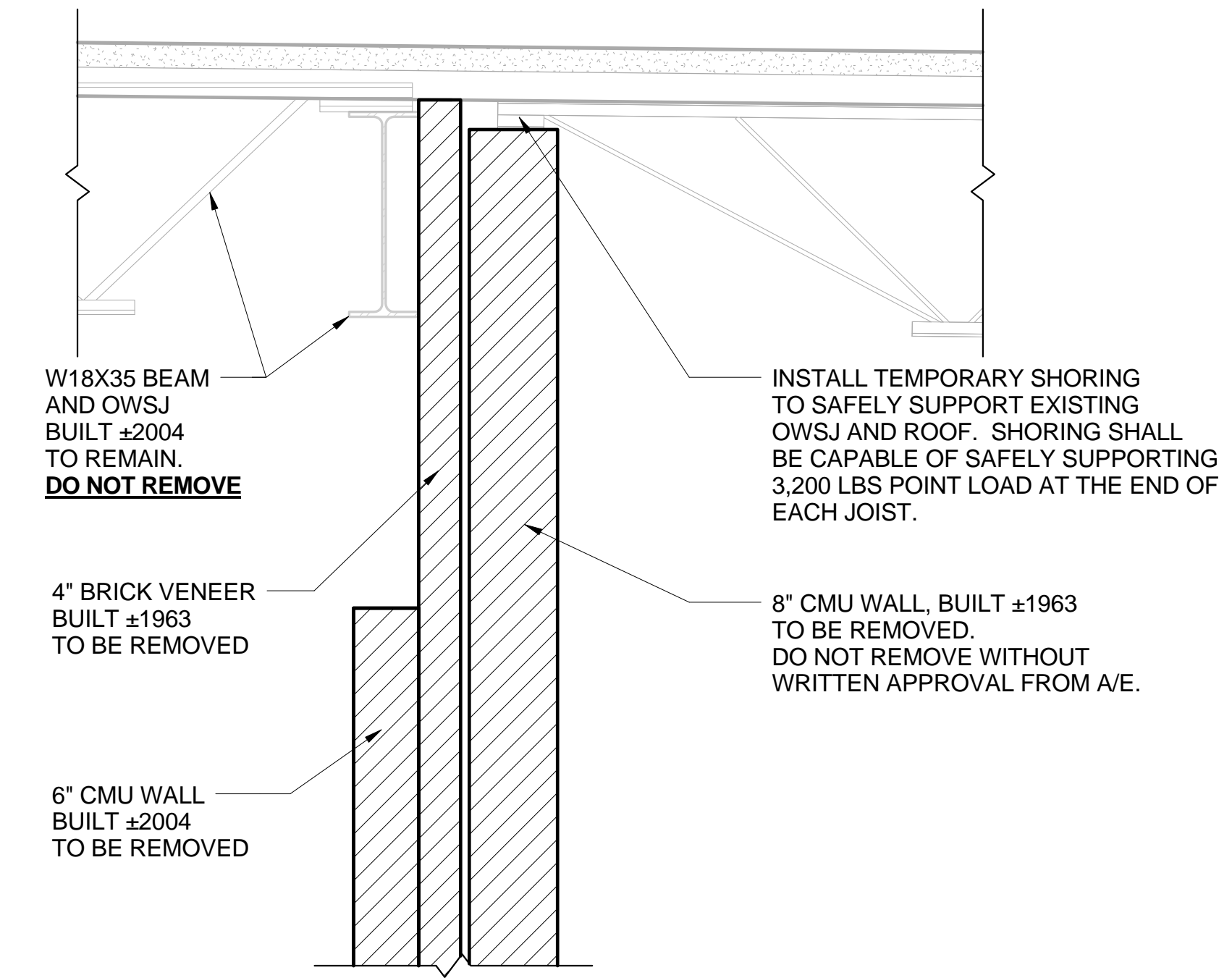
LINTEL SCHEDULE NOTES:

- ALL ANGLES SHALL BE PLACED LONG LEG VERTICAL.
- L-103, L-104B & L-105A MUST BE INSTALLED PRIOR TO CONSTRUCTION OF WEST ADDITION.
- ALL STEEL LINTELS LOCATED IN EXTERIOR WALLS SHALL BE HOT DIP GALVANIZED.
- SEE SHEETS S-001 & S-002 FOR GENERAL STRUCTURAL NOTES.
- SEE SHEETS S-401 & S-402 FOR ADDITIONAL MASONRY INFORMATION NOT SHOWN HERE.

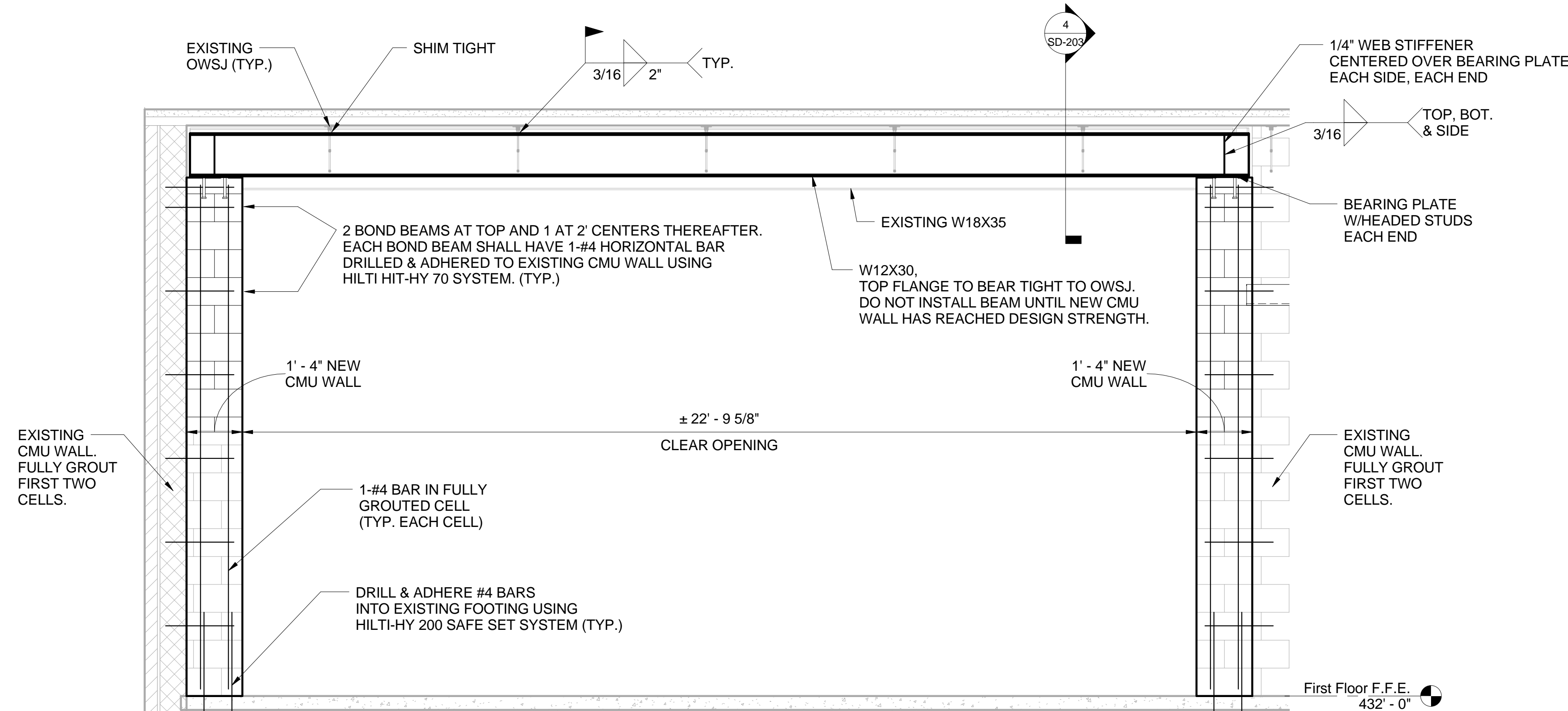
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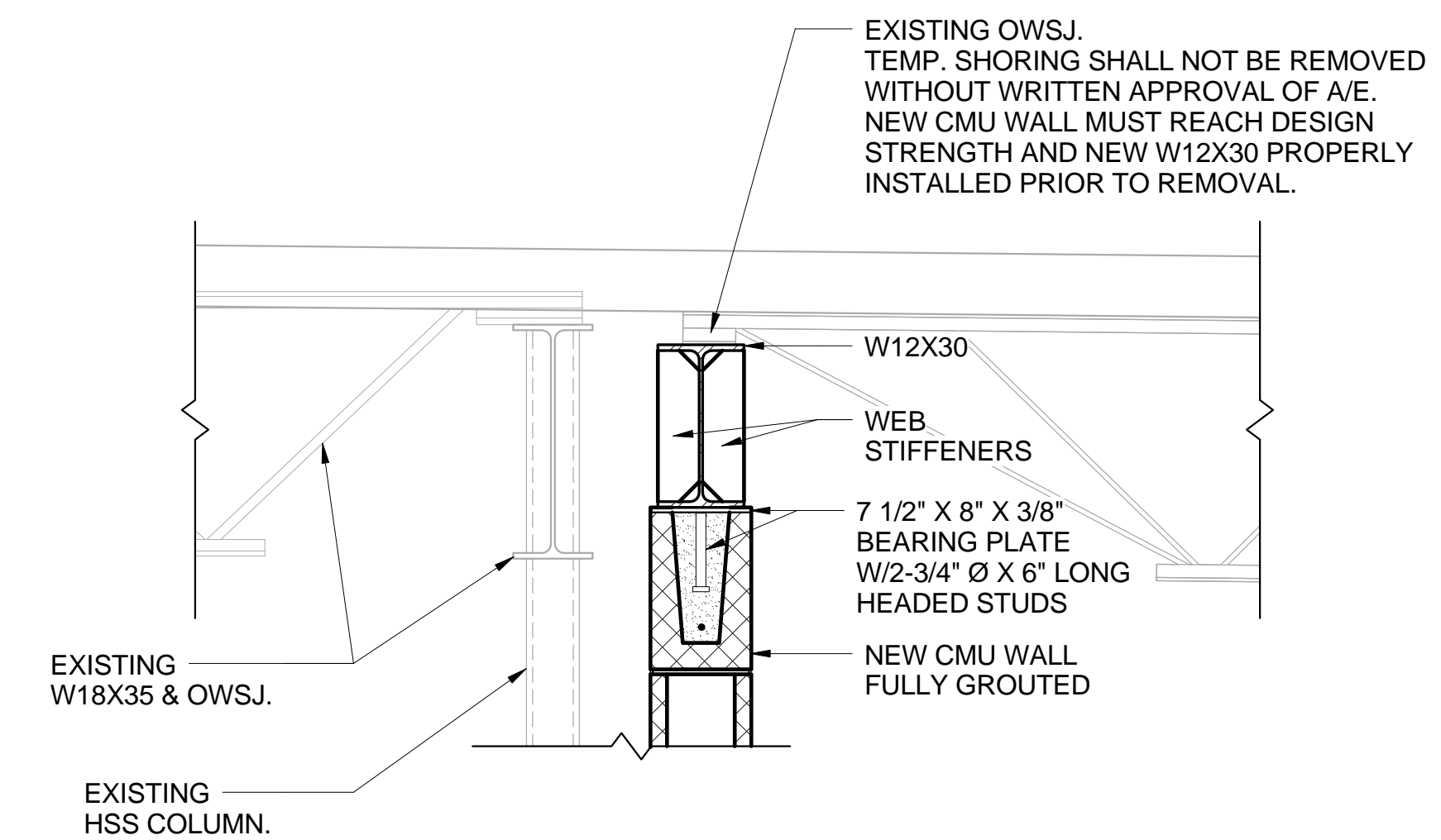
1 Demolition Elevation for L-116 (Looking West)
1/2" = 1'-0"



2 Demolition Section for L-116 (Looking North)
1" = 1'-0"



3 Renovation Elevation for L-116 (Looking West)
1/2" = 1'-0"



4 Renovation Section for L-116 (Looking North)
1" = 1'-0"

NOTES:

- SEE SHEETS S-001 & S-002 FOR GENERAL STRUCTURAL NOTES.
- SEE SHEETS S-401 & S-402 FOR ADDITIONAL MASONRY INFORMATION NOT SHOWN HERE.

Mark	Date	Description

DATE: 03/19/15
PROJECT NO: 360-2632
DESIGN: C.J.C. DRAWN: N.A.L. CHECK: R.V.B.

WALL MODIFICATIONS
L-116